GENERAL NOTES:

- 1. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, SHOP DRAWINGS AND SPECIFICATIONS.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL FINAL DIMENSIONS AND FIT—UP OF THE STRUCTURE, INCLUDING VERIFYING ALL EXISTING CONDITIONS AND DIMENSIONS BEFORE COMMENCING WORK.
- 3. THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING ANY WORK. ANY INTERFERENCE SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER.
- 4. A RECORD SET OF APPROVED SHOP DRAWINGS SHALL BE KEPT IN THE FIELD BY THE GENERAL CONTRACTOR.
- 5. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECT'S DRAWINGS BEFORE STARTING WORK.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN PLACEMENT, MAINTENANCE, ETC. OF ANY AND ALL SHORING, BRACING, TIE BACKS, ETC. NEEDED TO SUPPORT ANY PART OF THE NEW OR EXISTING CONSTRUCTION DURING THE ENTIRE CONSTRUCTION PROCESS TO ENSURE THE SAFETY AND INTEGRITY OF THE STRUCTURE UNTIL THE NECESSARY PERMANENT FLEMENTS ARE IN PLACE
- 7. SEE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR EXACT LOCATION OF ALL DEPRESSIONS, SLOPES, OPENINGS, PENETRATIONS, ETC. PENETRATION THROUGH BEAMS OR OPENINGS IN STRUCTURAL ELEMENTS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER.
- 8. UNLESS NOTED OTHERWISE, DETAILS SHOWN ON ANY DRAWING ARE TO BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.
- . CONTRACTOR SHALL NOTIFY ENGINEER OF RECORDS AND CLIENT BEFORE ANY DEVIATION FROM THE DESIGN DRAWINGS OR GENERAL NOTES.

SHOP DRAWING NOTES:

- 1. THE GENERAL CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTAL FOR REVIEW. SHOP DRAWINGS, REVIEWED BY THE GENERAL CONTRACTOR, FOR REINFORCING, JOIST, DECK, STRUCTURAL MEMBERS, AND STRUCTURAL STEEL SHALL BE SUBMITTED TO THE ARCHITECT AND/OR ENGINEER AND A STAMPED REVIEW SET RECEIVED PRIOR TO FABRICATION. INSTALLATION SHALL BE MADE FROM REVIEWED SHOP DRAWINGS ONLY.
- 2. THE FABRICATOR SHALL HIGHLIGHT CHANGES MADE IN SHOP DRAWINGS WHICH DO NOT COMPLY WITH THE DESIGN DRAWINGS AND RECEIVE CONFIRMATION PRIOR TO COMMENCING WITH FABRICATION OF SAME.
- 3. SHOP DRAWING REVIEW SHALL NOT CONSTITUTE ACCEPTANCE OF FABRICATOR CHANGES TO THE CONTRACT DOCUMENTS, ONLY GENERAL CONFORMANCE TO THE DESIGN INTENT. FABRICATOR CHANGES THAT RESULT IN MODIFICATIONS TO THE CONTRACT SUM MUST BE APPROVED IN ACCORDANCE WITH PROVISIONS CONTAINED IN THE OWNER-CONTRACTOR AGREEMENT OR PROCEDURES OUTLINED IN THE CONTRACT MANUAL.
- 4. ONLY SHOP DRAWINGS MARKED "NO EXCEPTIONS TAKEN" OR "CORRECT AS NOTED" MAY BE RELEASED FOR FABRICATION. SHOP DRAWINGS WITH ANY OTHER MARKINGS MUST BE REVISED AND AN APPROVED COPY RECEIVED BY THE FABRICATOR PRIOR TO FABRICATION OF THE MATERIAL. MATERIAL FABRICATED WITHOUT PROPER APPROVAL IS SUBJECT TO REJECTION.
- 5. REVIEW OF SHOP DRAWINGS IS FOR CONFORMANCE WITH THE DESIGN CONCEPT OF THE PROJECT AND COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE FABRICATION IS RESPONSIBLE FOR DIMENSIONS AND QUANTITIES ASSOCIATED WITH THE FABRICATION OF THEIR RESPECTIVE PARTS AND PORTIONS OF THE PROJECT. MEANS AND METHODS ASSOCIATED WITH THE FABRICATION OF ANY MATERIAL SHALL REMAIN THE RESPONSIBILITY OF THE FABRICATOR AS SHALL THE RESPONSIBILITY FOR THE COORDINATION OF INSTALLATION SEQUENCES AFFECTING OTHER TRADES.

FOUNDATION NOTES:

- 1. PILING SHALL BE AUGERCAST CONCRETE PILES INSTALLED TO A MINIMUM DEPTH AS SHOWN ON DESIGN DRAWINGS. SEE TYPICAL PILE DETAILS TO DETERMINE LENGTH AND REINFORCING REQUIREMENTS. PILE CAPACITY SHALL BE CONFIRMED WITH LOAD TEST.

 DESIGN LOAD = 125 TONS
- 2. PILING SHALL BE PLUMB AND TRUE WITHIN CONSTRUCTION TOLERANCE.
- 3. BEFORE CONCRETE IS PLACED, BOTTOM OF SHAFTS SHALL BE LEVEL AND FREE OF ANY DELETERIOUS.
- 4. PILE CAPS AND GRADE BEAMS SHALL BE CENTERED ON WALLS UNLESS NOTED OTHERWISE.
- 5. PILE CAPS AND GRADE BEAMS MAY BE EARTH FORMED PROVIDED DIMENSIONAL TOLERANCES LISTED IN ACI 117-90 ARE ADHERED TO.
- 6. PROBE PILES ARE SHOWN AN PAN AS (TYP.)
- 7. TEST PILE IS SHOWN ON PLAN AS (TYP.)
- 8. PLACE 15 MIL. STEGO WATERPROOF MEMBRANE BENEATH ALL INTERIOR SLABS AND BEAMS ON GRADE. LAP 12" TO ACCOMMODATE CONCRETE POURING DIRECTION.

CONCRETE NOTES:

- 1. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS WITH A 5" SLUMP.
- 2. CONCRETE COLUMNS AND TOWER CRANE PILE CAP FOUNDATIONS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS WITH A 5" SLUMP UNLESS NOTED OTHERWISE.
- 3. CONCRETE SHALL BE NORMAL WEIGHT (APPROXIMATELY 150 LBS. PER CUBIC FT.) AND SHALL CONFORM TO THE LATEST ACI 301 SPECIFICATION.
- 4. CONCRETE CONSTRUCTION SHALL CONFORM TO THE CURRENT "ACI MANUAL OF CONCRETE PLACEMENT".
- 5. PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I OR II.
- 6. AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL MEET ASTM C33.
- 7. REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ASTM A615 GRADE 60, WELDED WIRE FABRIC (WWF) SHALL BE IN ACCORDANCE WITH ASTM A185 WIRE SHALL CONFORM TO ASTM A82. LAP ALL FABRIC ONE WIRE SPACING PLUS 6 INCHES.
- 8. REINFORCING SHALL BE SPLICED WITH A CLASS "B" LAP SPLICE IN ACCORDANCE WITH THE CURRENT ACI 318 SPECIFICATION.
- 9. REINFORCING OR FABRIC ON GRADE SHALL BE CHAIRED WITH 3000 PSI CONCRETE BRICKETTES SPACED TO ADEQUATELY SUPPORT THE REINFORCING, BUT NOT GREATER THAN 3'-0" O.C. EACH WAY. AT RAISED FLOORS USE METAL CHAIRS.
- 10. PROVIDE A 90 DEGREE HOOK ON ALL TOP REINFORCEMENT IN ALL BEAMS AT DISCONTINUOUS ENDS AND LAP SPLICE 30 BAR DIAS. AT MID—SPAN. CONTINUOUS BOTTOM BARS SHOULD BE LAP SPLICED 6" AT CENTER OF SUPPORT.
- 11. BARS SHALL BE SECURELY SUPPORTED TO PREVENT BOTH VERTICAL AND HORIZONTAL MOVEMENT DURING CONCRETE PLACEMENT.
- 12. TEMPERATURE BARS IN SLAB AND INTERMEDIATE HORIZONTAL BARS IN WALLS AND BEAMS: TENSION LAP SPLICE; SEE TABLE RELOW
- 13. PROVIDE CORNER BARS AT EACH OUTSIDE CORNER FOR EACH HORIZONTAL BAR IN WALLS AND BEAMS; SEE LAP SPLICE TABLE FOR REQ'D LAP LENGTH. HOOK INSIDE BAR IN WALLS AT ENDS.

#4 20 #5 24 #6 30 #7 42 #8 48 #9 55 #10 65

14. COLUMN VERTICAL REINFORCING SHALL HAVE STANDARD HOOKS AT THE TOP OF THE UPPERMOST SECTION OF EACH COLUMN.

CONCRETE NOTES (CONT.):

- 15. PROVIDE 2 #5, 4'-0" LONGER THAN OPENING DIMENSION ON ALL SIDES OF OPENING IN SLAB AND WALLS.
- 16. CONCRETE PROTECTION FOR REINFORCING: 3" AT FOOTINGS AND GRADE BEAMS; 2" AT FORMED SURFACES LATER EXPOSED TO SOIL; 1 1/2" AT BEAMS, COLUMNS, AND WALLS; 1" AT SLABS.
- 17. NO HOLES OR OPENINGS THROUGH FOUNDATION AND/OR FOOTINGS WITHOUT ENGINEER'S APPROVAL.
- 18. EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4" AT 45 DEGREES.
- 19. REMOVE VISQUEEN AND EARTH FILL FROM THE TOPS OF PILES AND WHERE CONCRETE WILL BEAR ON THE TOP OF THE PILE CAP.
- 20. STYROFOAM USED FOR FILLING VOIDS UNDER CONCRETE SHALL BE AMOFORM EXTRUDED POLYSTRENE INSULATION BOARD BY AMOCO FOAM PRODUCTS CO. OR APPROVED EQUAL. MINIMUM COMPRESSIVE STRENGTH SHALL BE 30 PSI.
- 21. WHEN EXISTING CONCRETE AT THE FIRST FLOOR LEVEL IS REMOVED TO INSTALL NEW UTILITIES, ETC., THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF THE LOCATION AND EXTENT OF ANY SUCH REMOVAL PRIOR TO PERFORMING THE WORK. WHERE POSSIBLE, EXISTING REINFORCEMENT SHALL NOT BE CUT, BENT, OR DAMAGED. WHENEVER REINFORCEMENT IS CUT, DAMAGED OR BENT, IT SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER AND REPAIRED OR REPLACED AS

CONCRETE MASONRY UNIT (CMU) NOTES:

- 1. PROVIDE HOLLOW CONCRETE MASONRY UNITS MEETING ASTM C90, LIGHTWEIGHT, TYPE 1, WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI ON THE NET AREA FOR INDIVIDUAL UNITS.
- 2. CMU MORTAR SHALL MEET ASTM C270, TYPE 'M' OR 'S', AND HAVE A COMPRESSIVE CUBE STRENGTH OF 1800 PSI AT 28 DAYS.
- 6. CMU GROUT, POURED OR PUMPED, SHALL MEET ASTM C476, AND HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000
- 4. REINFORCING BARS SHALL MEET ASTM A615, GRADE 60.
- 5. JOINT REINFORCING SHALL MEET ASTM A82.
- 6. REINFORCED MASONRY WALLS SHALL HAVE A MINIMUM F'M = 2000 PSI.
- 7. REINFORCEMENT SHALL BE HELD IN PLACE PRIOR TO GROUTING WITH WIRE POSITIONERS SPACED AT INTERVALS NOT EXCEEDING 192 REINFORCING BAR DIAMETERS FOR 10 FEET. ADDITIONAL POSITIONERS SHALL BE PLACED AT ALL REINFORCING BAR SPLICES.
- 8. PROVIDE HORIZONTAL BOND BEAMS AT THE TOP OF ALL WALLS AND AT ROOF UNLESS SHOWN OTHERWISE, BOND BEAMS SHALL BE REINFORCED AS FOLLOWS: (MINIMUM)

- . CONCRETE MASONRY WALLS SHALL HAVE 2-NO.9 WIRE GALVANIZED HORIZONTAL JOINT REINFORCEMENT EVERY OTHER COURSE, IN ADDITION TO HORIZONTAL BOND BEAMS REINFORCEMENT TERMINATE BOND BEAM AND HORIZONTAL JOINT REINFORCING AT CONTROL JOINTS.
- 10. REINFORCE MASONRY AT BEARING POINTS OF ALL BEAMS, LINTELS, ETC. WITH 1 #6 (CONTINUOUS TO FOUNDATION) IN EACH BLOCK CORE BENEATH BEARING PLATES.
- 11. PROVIDE DOWELS FOR CMU WALL CONNECTION TO CONCRETE BEAMS AND SLABS AND FOOTINGS, SEE DETAILS. LAP DOWELS 2'-0" (MIN) WITH VERTICAL BARS.
- 12. CMU TO BE LAID IN RUNNING BOND PATTERN.

CONDUITS AND PIPES EMBEDDED IN CONCRETE:

- CONDUITS, PIPES, AND SLEEVES OF ANY MATERIAL NOT HARMFUL TO CONCRETE SHALL BE PERMITTED TO BE EMBEDDED IN CONCRETE WITH APPROVAL OF ENGINEER, PROVIDED THAT REGULATIONS ARE FOLLOWED AS OUTLINED IN THE APPLICABLE ACICODES
- 2. CONDUITS, PIPES, AND SLEEVES PASSING THROUGH A SLAB OR BEAM SHALL NOT SIGNIFICANTLY IMPAIR THE STRENGTH OF CONSTRUCTION AS DETERMINED BY THE ENGINEER.
- 3. SINGLE CONDUITS AND PIPES OR INTERSECTING CONDUITS AND PIPES SHALL NOT OCCUPY MORE THAN AN 11/2" OF SLAB THICKNESS AND 1/3 THE OVERALL THICKNESS OF BEAMS IN WHICH THEY ARE EMBEDDED, AND THEY SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER. ANY CONDUIT OR PIPE LARGER SHALL BE LOCATED BELOW THE RESPECTIVE SLAB OR BEAM. THE CONTRACTOR SHALL SUBMIT FOR APPROVAL, A DIAGRAM DEPICTING THE HOME RUNS OD CONDUIT TO ALL PANELS, TYPICAL.
- 4. IT WILL NOT BE PERMITTED TO CUT, BEND, OR DISPLACE THE REINFORCING STEEL FROM ITS PROPER LOCATION.
- 5. COORDINATION MUST BE MADE BY THE CONTRACTOR AT HIS EXPENSE TO FOLLOW THE ABOVE GUIDELINES.

DRILLING HOLES FOR ANCHORS AND CORING HOLES IN EXISTING CONCRETE:

- 1. PRIOR TO DRILLING OR CORING HOLES, THE CONTRACTOR SHALL LOCATE EXISTING REINFORCING STEEL, POST—TENSIONING, CONDUIT, PIPING, ETC. IN THE AREA WHERE NEW HOLES ARE TO BE INSTALLED THROUGH NON—DESTRUCTIVE TESTING SUCH AS WITH AN X—RAY, RADAR, OR WITH OTHER NON—DESTRUCTIVE DEVICES.
- 2. MARK THE LOCATION OF ALL REINFORCING STEEL, POST—TENSIONING, CONDUIT, PIPING, AND OTHER EXISTING INTERFERENCES ON THE SURFACE OF THE SLAB.
- 3. IF NEW HOLE LOCATIONS CONFLICT WITH EXISTING REINFORCING, POST—TENSIONING, CONDUIT, PIPING, ETC., THE CONTRACTOR SHALL NOTIFY THE ENGINEER BEFORE INSTALLING THE NEW HOLES.
- 4. VERIFY NO CONFLICTS EXIST AT NEW HOLE LOCATIONS BY SMALL DRILLED PILOT HOLES. IF NO CONFLICT EXIST, COMPLETE THE INSTALLATION. IN THE CASE OF STEEL TO BE FASTENED TO THE CONCRETE WITH MULTIPLE ANCHORS, FABRICATE, FROM A FIELD TEMPLATE, THE STEEL TO BE FASTENED TO THE CONCRETE BY THE ANCHORS AND COMPLETE THE INSTALLATION.
- 5. WHEN INSTALLING NEW HOLES, CARE SHALL BE EXERCISED SO AS NOT TO NICK OR CUT EXISTING REINFORCING STEEL, POST—TENSIONING, CONDUIT, PIPING, ETC.

MECHANICAL EQUIPMENT:

- 1. PRIOR TO THE DETAILING OF ANY STRUCTURAL MATERIALS INVOLVED IN THE SUPPORT OF MECHANICAL EQUIPMENT, THE CONTRACTOR SHALL FURNISH TO THE ARCHITECT ALL INFORMATION RELATIVE TO LOADS, LOAD POINTS, DIMENSIONS, ETC. OF THE ACTUAL EQUIPMENT WHICH IS TO BE FURNISHED.
- 2. ALL DETAILS AND MEMBER SIZES SHOWN ON THE STRUCTURAL DRAWINGS ARE TENTATIVE UNTIL SUCH TIME AS THIS INFORMATION IS REVIEWED BY THE ARCHITECT.
- 3. LOCATION OF SUPPORT BEAMS MUST BE COORDINATED WITH MECHANICAL EQUIPMENT LOCATIONS.
- 4. COORDINATION MUST BE MADE BY THE CONTRACTOR AT HIS EXPENSE TO FOLLOW THE ABOVE GUIDELINES.

METAL FLOOR DECK NOTES:

- 1. METAL FLOOR DECK SHALL BE 3", 16 GAUGE GALVANIZED STEEL COMPOSITE TYPE DECK, CONTINUOUS OVER 3 OR MORE SPANS AND FIELD WELDED TO SUPPORTS WITH 5/8" PUDDLE WELDS AT 12" O.C. WIRE BRUSH ALL WELDS AND TOUCH UP WITH GALVANIZED REPAIR PAINT BEFORE PLACING CONCRETE FLOOR SLAB.
- 2. SHEAR STUDS TO BE INSTALLED USING NELSON STUD GUN. ALL FERRULES SHALL BE REMOVED FROM THE BASE OF THE STUDS BEFORE PLACING THE CONCRETE SLAB. ALL SHEAR STUDS TO MEET ASTM A108 Fy = 60 KSI.
- 3. PROVIDE 5 1/2" x 4 1/2" x 1/2" BENT PLATE WELDED TO TOP OF SUPPORT BEAMS ON ALL SIDES OF OPENING THROUGH
- 4. UNLESS INDICATED OTHERWISE, PROVIDE POUR STOPS OF LENGTH, DEPTH AND GAGE APPROPRIATE FOR OVERHANG AND SLAB
- 5. FLOOR DECK OPENINGS LARGER THAN 12 INCHES, WHICH ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS, SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER.
- 6. FLOOR DECK SHALL BE SUPPORTED AROUND ALL OPENINGS AND COLUMNS.

STFFL NOTES

- 1. STRUCTURAL STEEL FABRICATOR SHALL PARTICIPATE IN THE AISC FABRICATOR CERTIFICATION PROGRAM AND BE CERTIFIED BY AISC FOR "STANDARD BUILDING STRUCTURES (STD)".
- 2. STRUCTURAL STEEL SHALL MEET THE LATEST AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- 3. UNLESS NOTED OTHERWISE ALL W, C, OR S STRUCTURAL STEEL SHAPES SHALL CONFORM TO ASTM A992 GRADE 50. ALL OTHER SHAPES AND PLATES SHALL BE IN ACCORDANCE WITH ASTM A36.
- 4. TUBE SECTIONS SHALL BE ASTM A-500 GRADE B (46 KSI YIELD).
- 5. SPLICES IN STRUCTURAL STEEL NOT SHOWN ON THE STRUCTURAL DRAWINGS WILL NOT BE ACCEPTED WITHOUT SPECIFIC APPROVAL OF THE STRUCTURAL ENGINEER.
- 6. CONNECTION BOLTS SHALL MEET ASTM A325 (3/4" MIN. DIA.), WITH WASHERS AS REQUIRED. EXCEPT AS NOTED, ALL BEAM CONNECTIONS SHALL BE AISC STANDARD FRAMED CONNECTIONS, BOLTED OR WELDED. CONNECTIONS TO BE DESIGNED FOR 3/4 UNIFORM LOAD BEAM CAPACITY FOR PROPER BEAM SPAN UNLESS OTHERWISE INDICATED. CONNECTIONS NOT DESIGNED NOT DETAILED ON THE DRAWINGS, INCLUDING AISC STANDARD FRAMED CONNECTIONS, SHALL BE DESIGNED BY A LOUISIANA REGISTERED CIVIL ENGINEER COMMISSIONED BY THE CONTRACTOR AND SHOP DRAWINGS MUST BEAR THAT ENGINEER'S SEAL.
- 7. UNLESS NOTED OTHERWISE ALL ANCHOR BOLTS SHALL BE 3/4" DIAMETER ASTM A307. ALL ANCHOR BOLTS SHALL BE HOOKED OR HEADED AT THE UNTHREADED END AND NOT MODIFIED WITHOUT SPECIFIC APPROVAL OF THE STRUCTURAL ENGINEER.
- 8. UNLESS NOTED OTHERWISE EVERY WELD SHALL DEVELOP THE FULL STRENGTH OF THE LESSER OF THE MEMBERS IT JOINS. ALL BUTT, GROOVE, OR BEVEL WELDS SHALL BE COMPLETE, FULL PENETRATION. WELDING SHALL CONFORM TO THE STANDARDS SET FORTH IN THE AWS PUBLICATION "WELDING IN BUILDING CONSTRUCTION".
- 9. NOTED SHOP CONNECTIONS TO HAVE 3/16" FILLET WELDS (MIN), UNLESS AS BOLTED CONNECTION.
- 10. FIELD WELDS TO BE WITH E70XX ELECTRODES. ALL ERECTION DRAWINGS SHALL SHOW ALL FIELD WELDS REQUIRED.
- 11. WHERE POSSIBLE, ALL BOLT HOLES IN STRUCTURAL STEEL SHALL BE DRILLED OR PUNCHED IN THE SHOP. ANY HOLES REQUIRED TO BE MADE AT THE PROJECT SITE SHALL BE MECHANICALLY DRILLED OR PUNCHED. NO BURNING OF HOLES SHALL BE
- 12. UNLESS SHOWN OTHERWISE ALL CAP AND BASE PLATES SHALL BE WELDED TO THE COLUMNS CONTINUOUSLY ALL AROUND WITH A 1/4" FILLET WELD.
- 13. STRUCTURAL STEEL SHALL HAVE THE FOLLOWING SURFACE PREPARATION IN ACCORDANCE WITH THE STRUCTURAL STEEL PAINTING COUNCIL REQUIREMENTS FOR THE FOLLOWING GRADE: SSPC—SP3 POWER TOOL.
- 14. STRUCTURAL STEEL SHALL BE SHIPPED WITH ONE COAT OF SHOP PRIMER EXCEPT THOSE MEMBERS THAT ARE GALVANIZED OR IN AREAS SCHEDULED TO RECEIVE FIRE PROOFING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING AREAS TO BE
- 16. NO OPENINGS TO BE PLACED IN BEAM WEBS OR FLANGES WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.
- 17. ALL BEAMS AND DIAGONAL BRACING SHALL NOT BE RELEASED FROM THE HOIST CABLE UNTIL MEMBER IS SECURED BY A MINIMUM OF TWO BOLTS PER END.
- 18. THE STEEL FRAME IS "NON-SELF SUPPORTING". ADEQUATE TEMPORARY SUPPORT MUST BE PROVIDED BY THE CONTRACTOR UNTIL REQUIRED CONNECTIONS OR ELEMENTS ARE IN PLACE.

METAL ROOF DECK NOTES:

- 1. METAL ROOF DECK SHALL BE 3", 16 GAUGE GALVANIZED STEEL ROOF DECK, INTERMEDIATE RIB, CONTINUOUS OVER 3 OR MORE SPANS AND FIELD WELDED TO SUPPORTS.
- 2. ROOF DECK AND ITS FASTENINGS TO ITS SUPPORTED FRAMING SHALL BE DESIGNED (AS A MINIMUM) FOR A NET UPLIFT PRESSURE APPLIED TO THE HORIZONTAL PROJECTED ROOF AREA AS INDICATED IN THE COMPONENTS AND CLADDING DESIGN PRESSURES (LISTED IN THE DESIGN CRITERIA SECTION). THIS PRESSURE IS UNFACTORED AND CONSEQUENTLY THE 1/3 INCREASE IN ALLOWABLE STRESS CAN BE APPLIED. THE MANUFACTURER SHALL SUBMIT A NOTARIZED AFFIDAVIT STATING FULL DESIGN COMPLIANCE WITH THE UPLIFT CRITERION.
- 3. ROOF DECK OPENINGS LARGER THAN 12 INCHES, WHICH ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS, SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER.
- 4. ROOF DECK SHALL BE SUPPORTED AROUND ALL OPENINGS, PENETRATIONS, HIPS, AND VALLEYS.
- 5. PROVIDE HSS 3 1/2x2 1/2x3/16" AT CENTERLINE OF BEAM TOP FLANGE BETWEEN JOISTS FOR SUPPORT OF DECK WHERE DECK CHANGES DIRECTION.



THESE PLANS HAVE BEEN PREPARED BY ME OR UNDER MY CLOSE PERSONAL SUPERVISION AND TO THE BEST OF MY KNOWLEDGE AND BELIEF, THEY COMPLY WITH ALL LOCAL, REGIONAL AND NATIONAL REQUIREMENTS. I AM OBSERVING THE WORK.

ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

LICENSE NUMBER:

51.0A

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CERTIFIED CORRECT

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PROFESSIONAL ENGINEER

GENERAL NOTES

PROJECT #: 1601

PHASE: CD
DRAFTER: JRN
CHECKER: JBH
SCALE: AS NOTED

S1 04

31593

ISSUED: 04/20/2018

COLD FORMED METAL FRAMING NOTES:

- 1. DETAILING, FABRICATION AND ERECTION OF COLD FORMED METAL FRAMING SHALL CONFORM TO AISI SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS AND AWS D1.3 STRUCTURAL WELDING CODE.
- 2. COLD FORMED METAL JOISTS, STUDS AND TRACKS FORMED WITH 20 OR 18 GAGE MATERIAL SHALL CONFORM TO ASTM A446 GRADE A, Fy = 33 KSI. JOISTS, STUDS AND TRACKS FORMED WITH 16, 14 AND 12 GAGE MATERIAL SHALL CONFORM TO ASTM A446 GRADÉ D, Fy = 50 KSI.
- 3. COLD FORMED METAL MEMBERS ARE CALLED OUT WITH THE STANDARD DESIGNATION USED BY CLARK WESTERN. FOR EXAMPLE 600 S 162-43 IS A 6" DEEP, 1 5/8" FLANGE, 18 GAGE "C" - SHAPED MEMBER. CLARK WESTERN BRAND MAY BE SUBSTITUTED WITH AN APPROVED EQUAL.
- 4. UNLESS NOTED OTHERWISE ALL COLD FORMED ELEMENTS SHALL BE CONNECTED WITH #12 STEEL SCREWS. SCREWS USED SHALL HAVE THE FOLLOWING MINIMUM DIAMETERS MEASURED OUT TO OUT OF THREADS: #8 - 0.164", #10 - 0.190", #12 - 0.216".
- 5. OPENINGS IN EXTERIOR WALLS OR INTERIOR BEARING WALLS SHALL HAVE HEADERS AS SHOWN ON THE PLANS OR AS FOLLOWS:
 - OPENINGS LESS THAN 4'-0"....2 600 S 162-43 OPENINGS 4'-0" TO 6'-0"......2 - 1000 S 162-43
 - GREATER THAN 6'-0".....SEE PLAN OR CONSULT STRUCTURAL ENGINEER
- ALL HEADER MEMBERS SHALL BE CONSTRUCTED WITH NON-PUNCHED MEMBERS. 6. OPENINGS IN EXTERIOR WALLS SHALL BE FRAMED AS SHOWN IN THE TYPICAL DETAILS ON THESE DRAWINGS.

BUILDING CRITERIA - 2012 INTERNATIONAL BUILDING CODE

DESIGN GRAVITY LOADS:

FLOOR LIVE LOADS:

- 1. RESIDENTIAL AREAS: 50 PSF
- 2. BALCONY AREAS: 100 PSF 3. DINING AREAS: 100 PSF
- 4. RETAIL AREAS: 100 PSF
- 5. CORRIDORS (1ST FLOOR): 100 PSF.
- 6. CORRIDORS (UPPER FLOORS): 80 PSF. 7. PLACES OF ASSEMBLY: 100 PSF.
- 8. RESTROOMS: 60 PSF.
- 9. PARKING AREAS (PASSENGER CARS ONLY): 40 PSF.
- 10. MECHANICAL/ELECTRICAL ROOMS: 125 PSF (MINIMUM). 11. STORAGE: 200 PSF.

FLOOR SUPERIMPOSED DEAD LOADS (EXCLUDES STRUCTURE'S SELF WEIGHT, WEIGHT OF SLAB & BEAMS):

1. CEILING + PARTITIONS: 20 PSF.

ROOF LOADS (UNOCCUPIED AREAS):

1. LIVE LOAD: 20 PSF 2. SUPERIMPOSED DEAD LOAD: 20 PSF

ROOF SNOW LOADS:

1. GROUND SNOW LOAD (Pg): 0.0 PSF

WIND LOADS (ASCE 7-10):

MAIN WIND FORCE RESISTING SYSTEM

PARAMETER	VALUE	ASCE 7-10
RISK CATEGORY BASIC WIND SPEED DIRECTIONALITY EXPOSURE CATEGORY TOPOGRAPHIC FACTOR GUST EFFECT FACTOR ENCLOSURE CLASSIFICATION INTERNAL PRESSURE COEFFICIENT VELOCITY	I V = 143 MPH Kd = 0.85 B Kzt = 1.0 0.85 ENCLOSED CGpi = +/-0.18 qh = 31.15 PSF	FIGURE 26.6-1 SECTION 26.7 FIGURE 26.8-1

	COMPONENTS AND CLADDING DESIGN PRESSURES (PSF)										
ZONE EWA (FT ²)		1	:	2	į	3		4	,	5	
≤10	N/A	-87.1	N/A	-136.7	N/A	-186.3	59.5	-59.5	59.5	-109.1	
<u>20</u>	N/A	-82.2	N/A	-129.9	N/A	-177.5	59.5	-59.5	59.5	-109.1	
<u>50</u>	N/A	-75.7	N/A	-120.8	N/A	-165.9	54.8	-56.4	54.8	-96.6	
<u>100</u>	N/A	-70.9	N/A	-114.0	N/A	-157.1	51.3	-54.0	51.3	-87.1	
200	N/A	-66.0	N/A	-107.1	N/A	-148.3	47.7	-51.6	47.7	-77.6	
500	N/A	-59.5	N/A	-98.1	N/A	-136.7	43.0	-48.5	43.0	-65.0	
1000	N/A	-59.5	N/A	-98.1	N/A	-136.7	43.0	-48.5	43.0	-65.0	
NOTEC:											

NOTES:

1. EWA IS THE EFFECTIVE WIND AREA OF A STRUCTURAL COMPONENT.

FOR ZONE DEFINITIONS, SEE ASCE 7-10 TABLE 30.7-2. PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE SURFACES, RESPECTIVELY.

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GENERAL NOTES

PROJECT #: 1601 PHASE: CD CIVIL / STRUCTURAL ENGINEERING 3500 N. CAUSEWAY BLVD., #1100, METAIRIE, LA 70002 DRAFTER: JRN

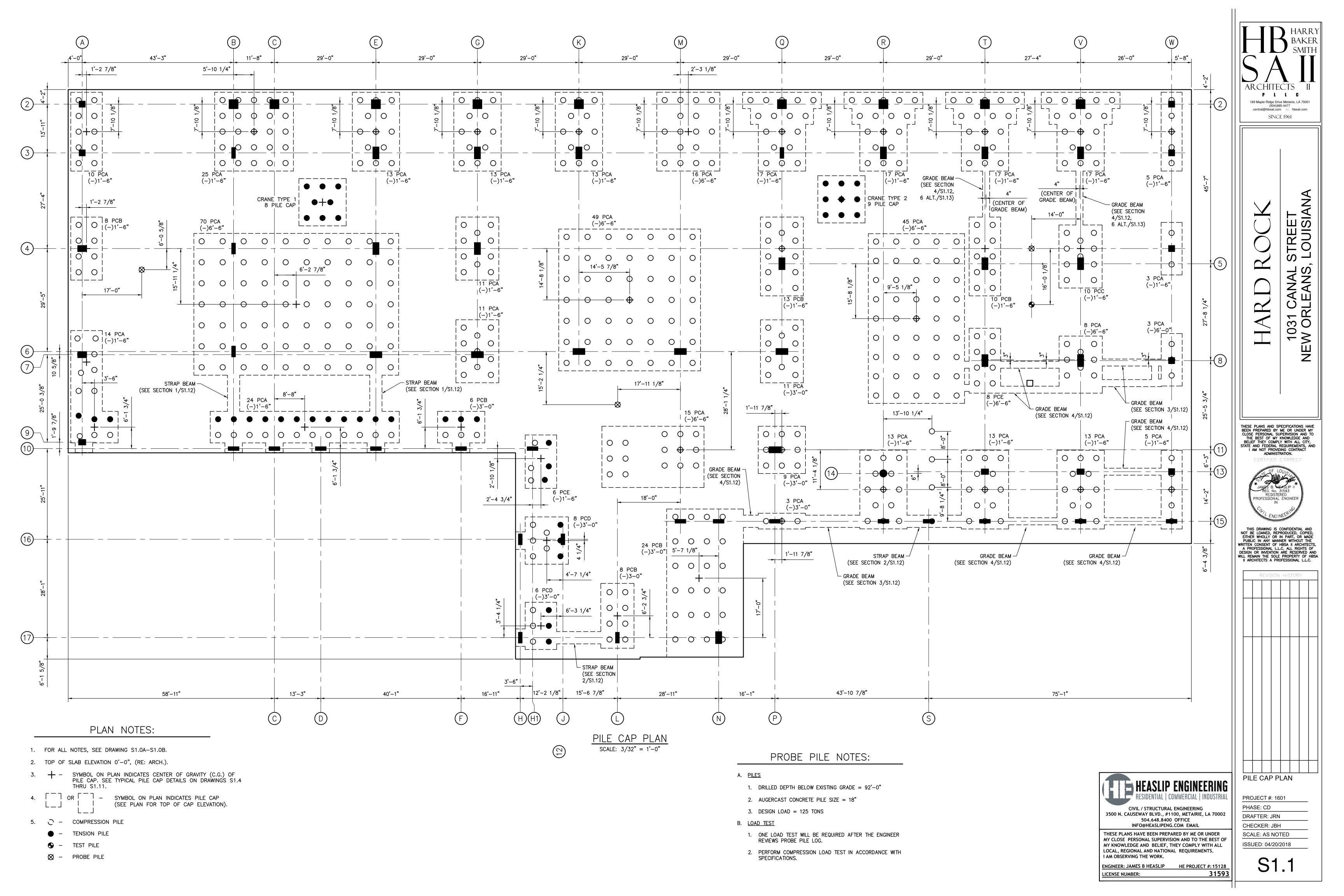
504.648.8400 OFFICE INFO@HEASLIPENG.COM EMAIL THESE PLANS HAVE BEEN PREPARED BY ME OR UNDER MY CLOSE PERSONAL SUPERVISION AND TO THE BEST OF MY KNOWLEDGE AND BELIEF, THEY COMPLY WITH ALL

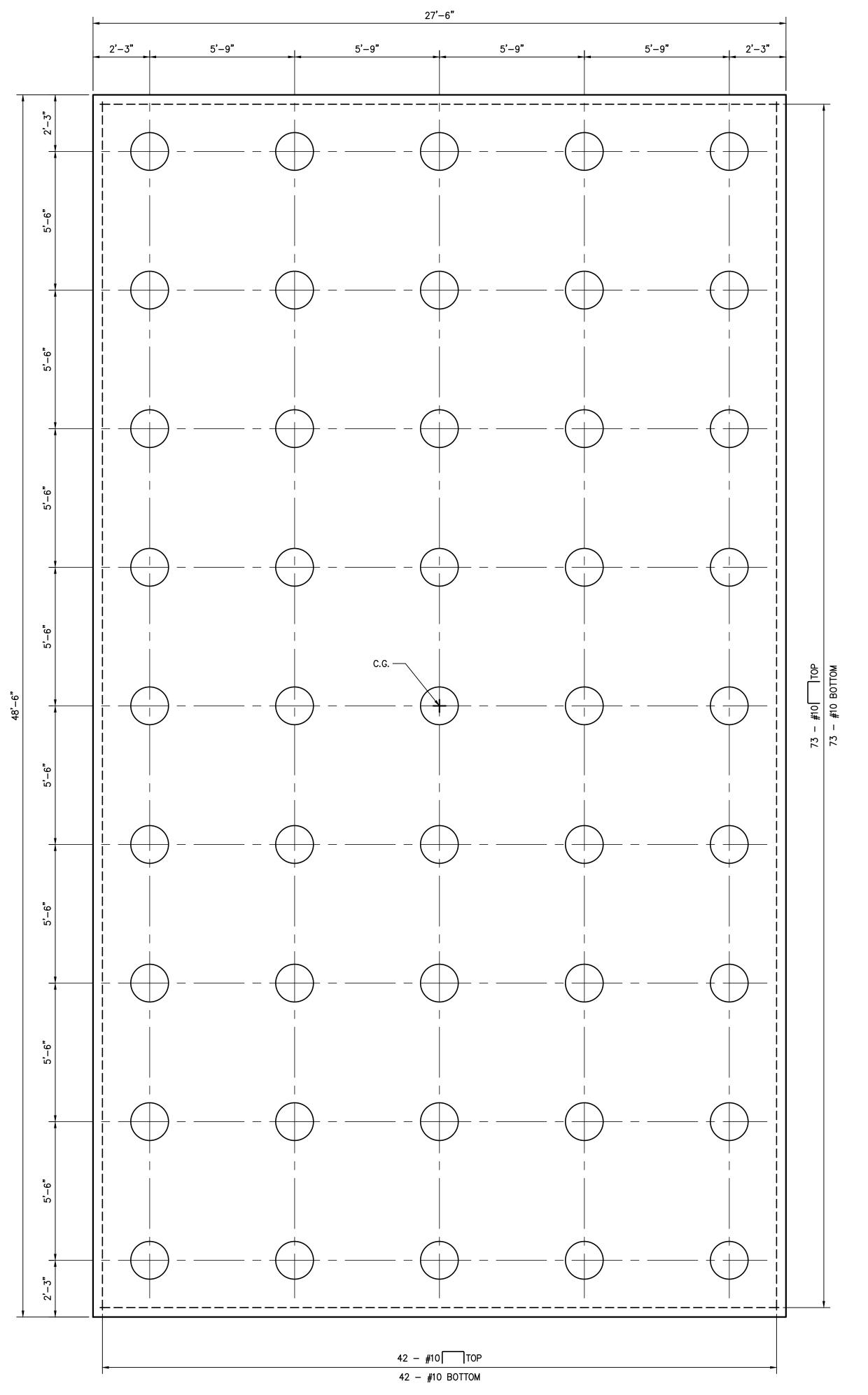
LOCAL, REGIONAL AND NATIONAL REQUIREMENTS. I AM OBSERVING THE WORK. ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593

CHECKER: JBH

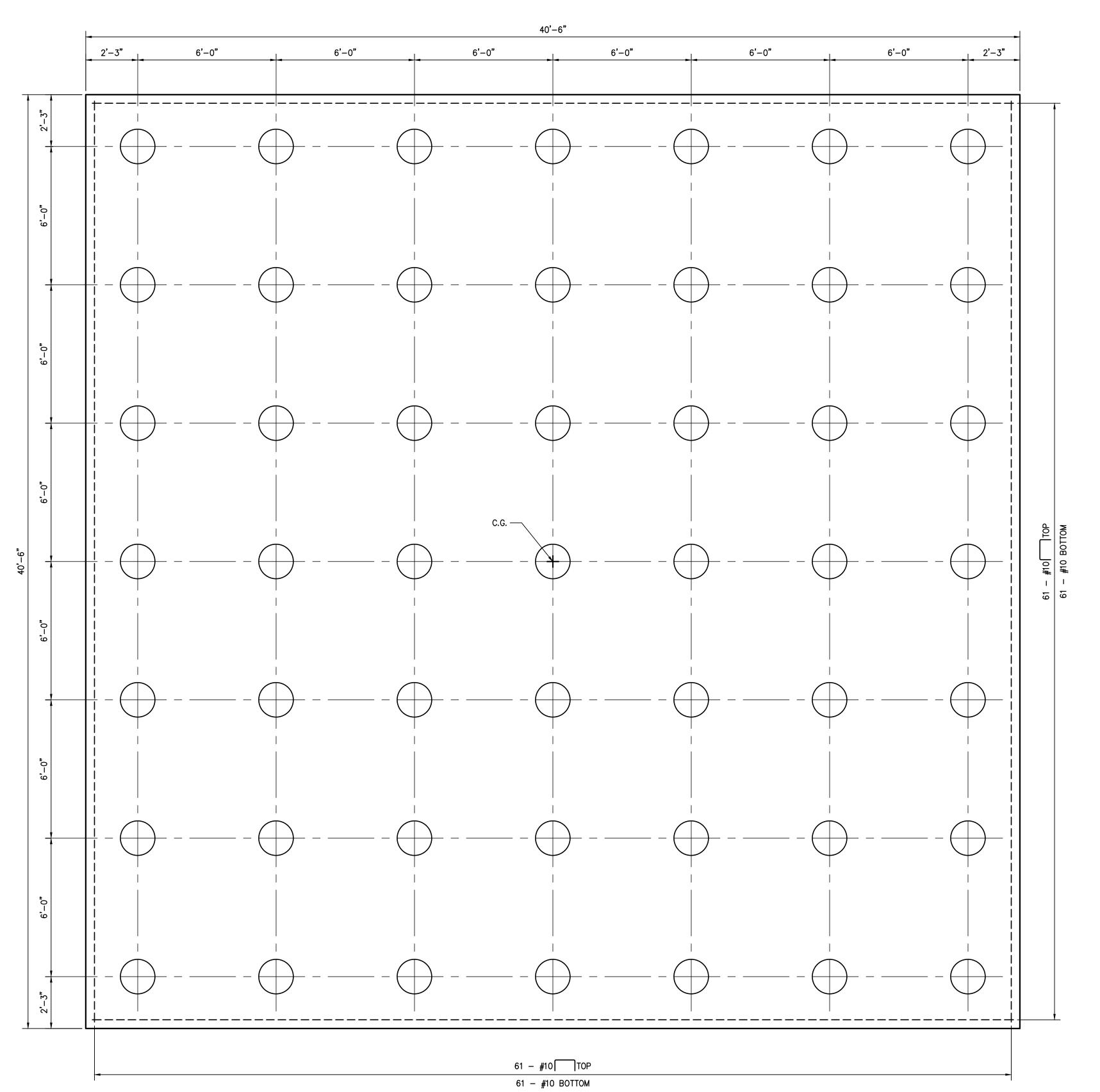
SCALE: AS NOTED

ISSUED: 04/20/2018





45 PCA PILE CAP DETAIL (DEPTH = 5'-0") SCALE: 3/8" = 1'-0"



49 PCA PILE CAP DETAIL (DEPTH = 5'-0") SCALE: 3/8" = 1'-0"

PLAN NOTES: 1. FOR ALL NOTES, SEE DRAWING S1.0A-S1.0B. COMPRESSION PILE



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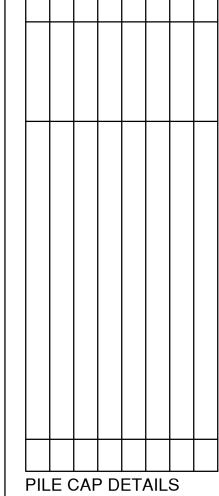
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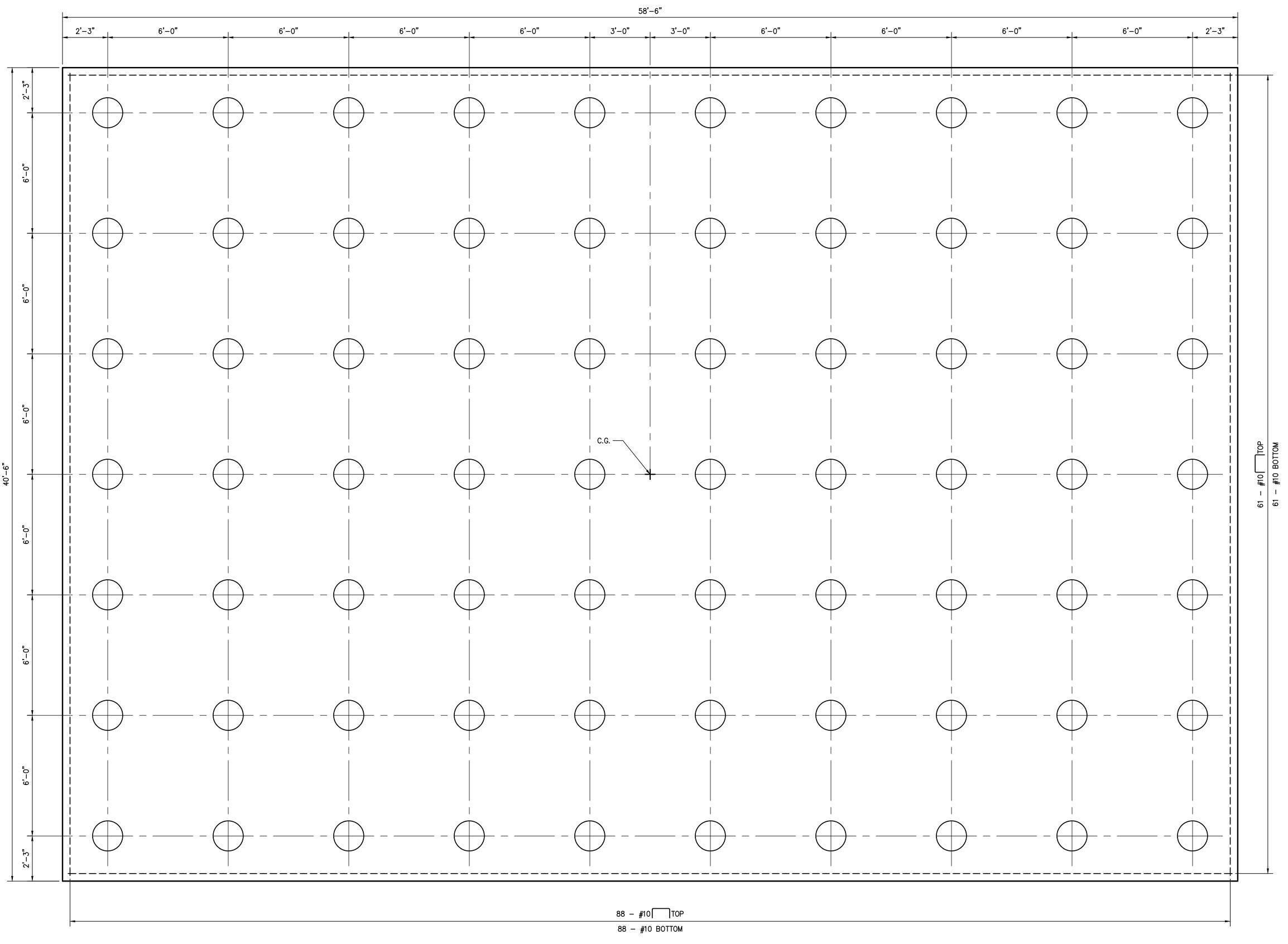


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PROJECT #: 1601 PHASE: CD DRAFTER: JRN

CHECKER: JBH SCALE: AS NOTED ISSUED: 04/20/2018



70 PCA PILE CAP DETAIL (DEPTH = 5'-0")

SCALE: 3/8" = 1'-0"

PLAN NOTES:

1. FOR ALL NOTES, SEE DRAWING \$1.0A-\$1.0B.

2. COMPRESSION PILE



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LICENSE NUMBER: 31593

S 1 31593

HARRY
BAKER
SMITH
SMITH
SMITH
ARCHITECTS II
P L C

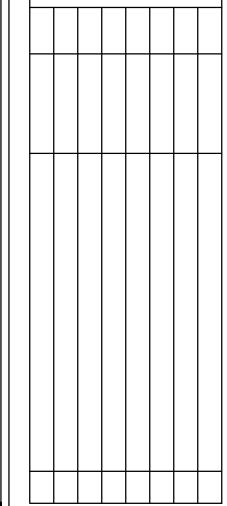
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HARD ROCK

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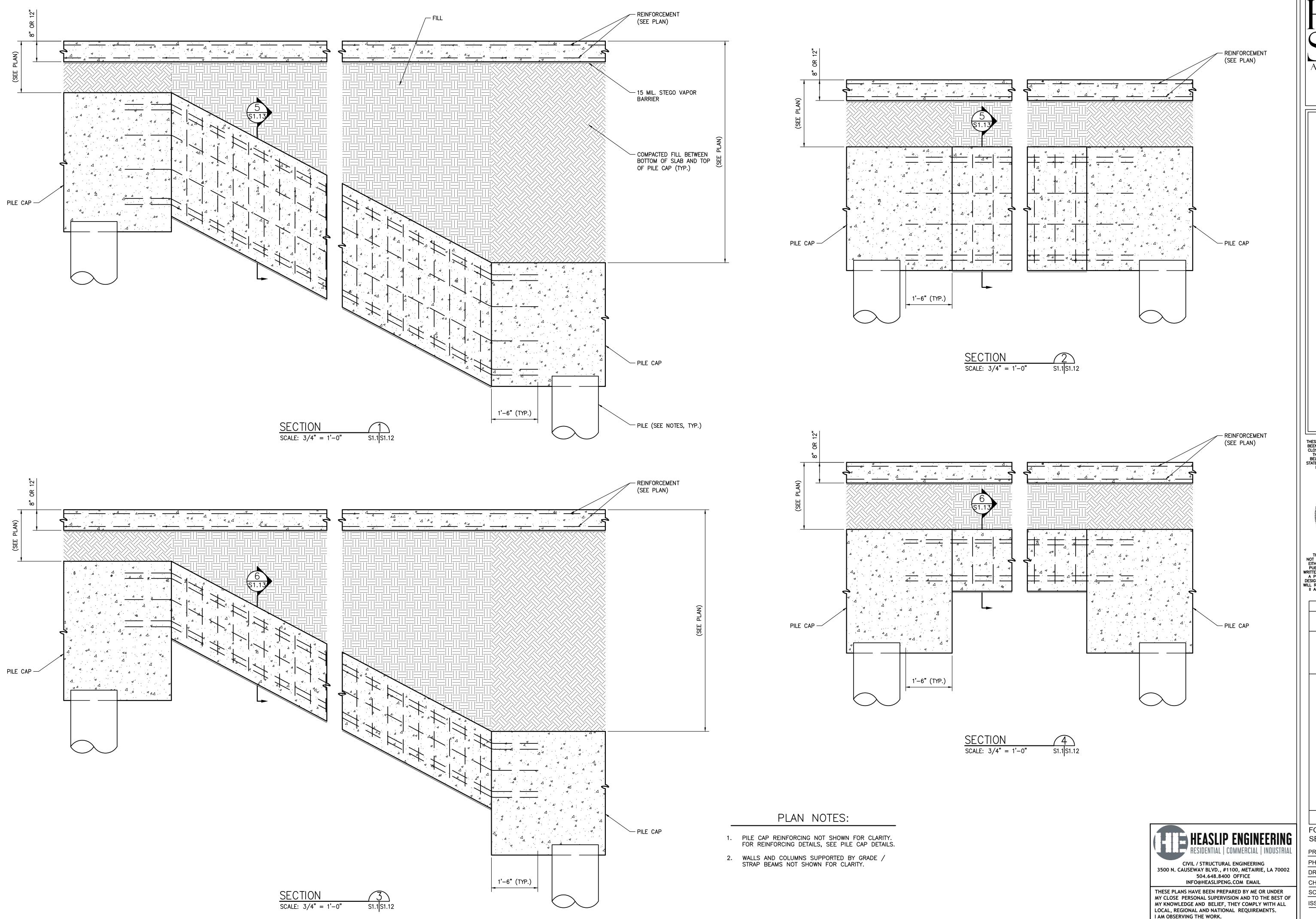
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PILE CAP DETAIL

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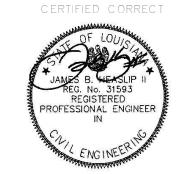
CHECKER: JBH
SCALE: AS NOTED
ISSUED: 04/20/2018



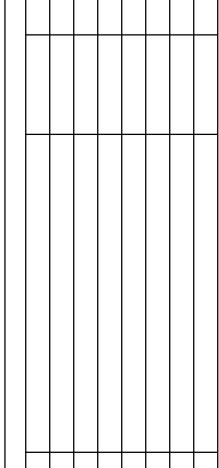
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STREET, LOUISIANA 1031 CANAL 8 NEW ORLEANS,

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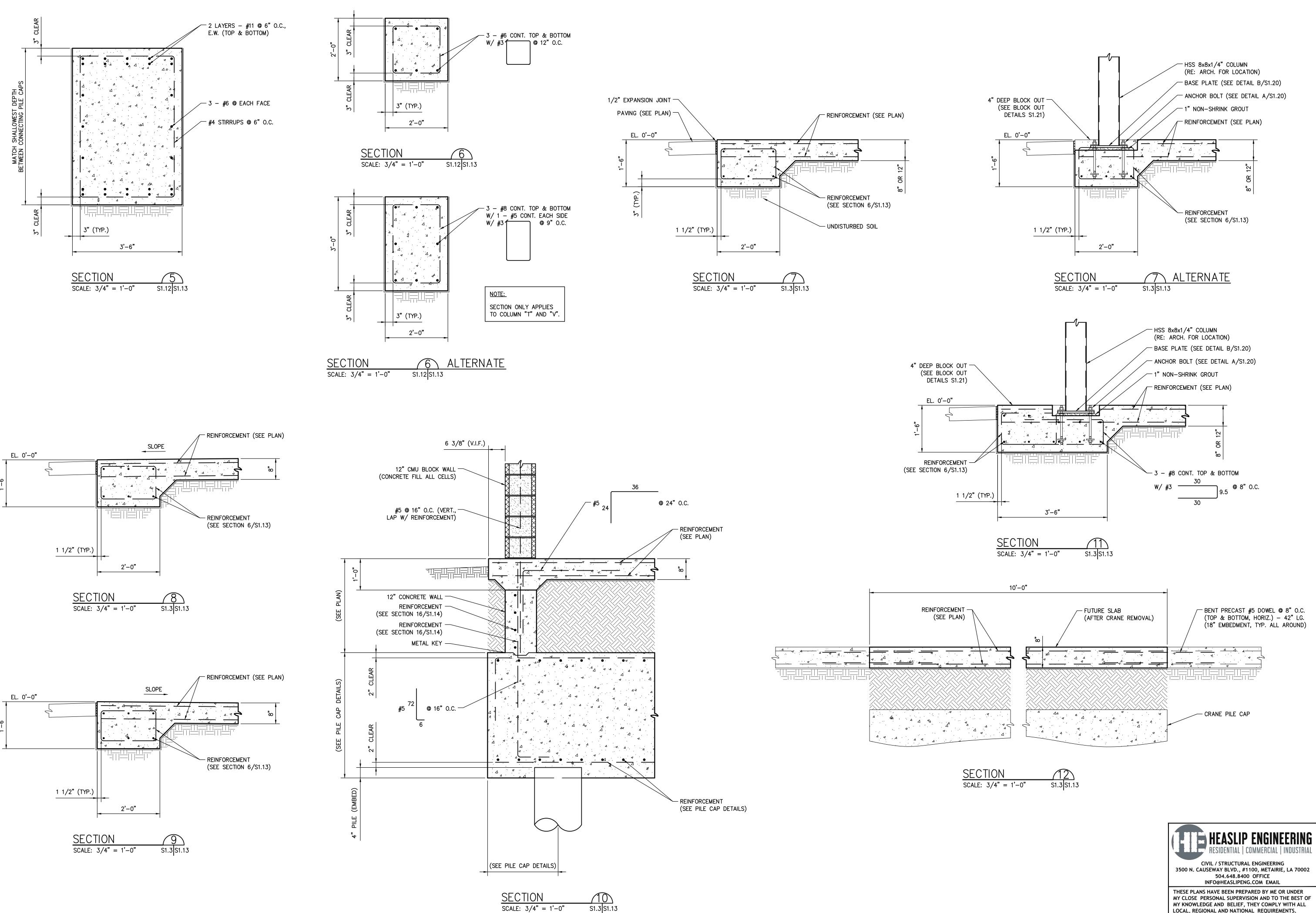
PROJECT #: 1601 PHASE: CD

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SCALE: AS NOTED ISSUED: 04/20/2018

ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

31593



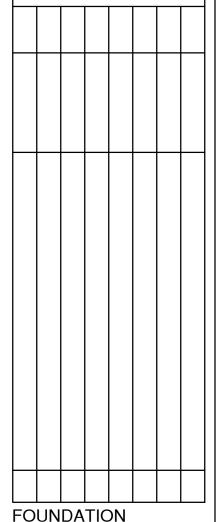
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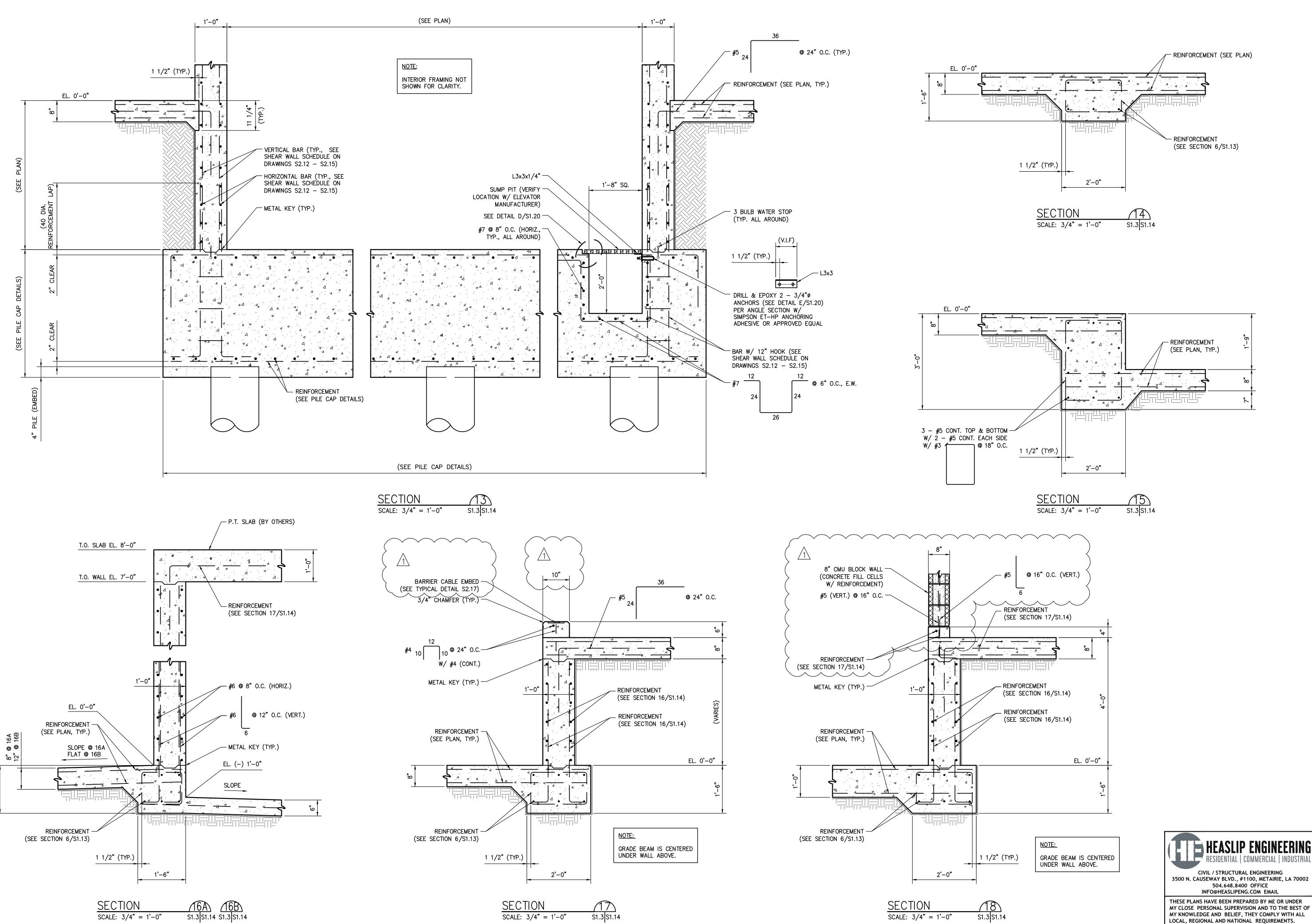
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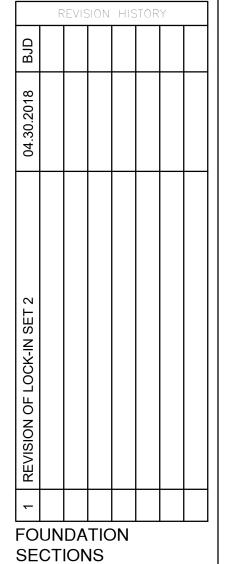
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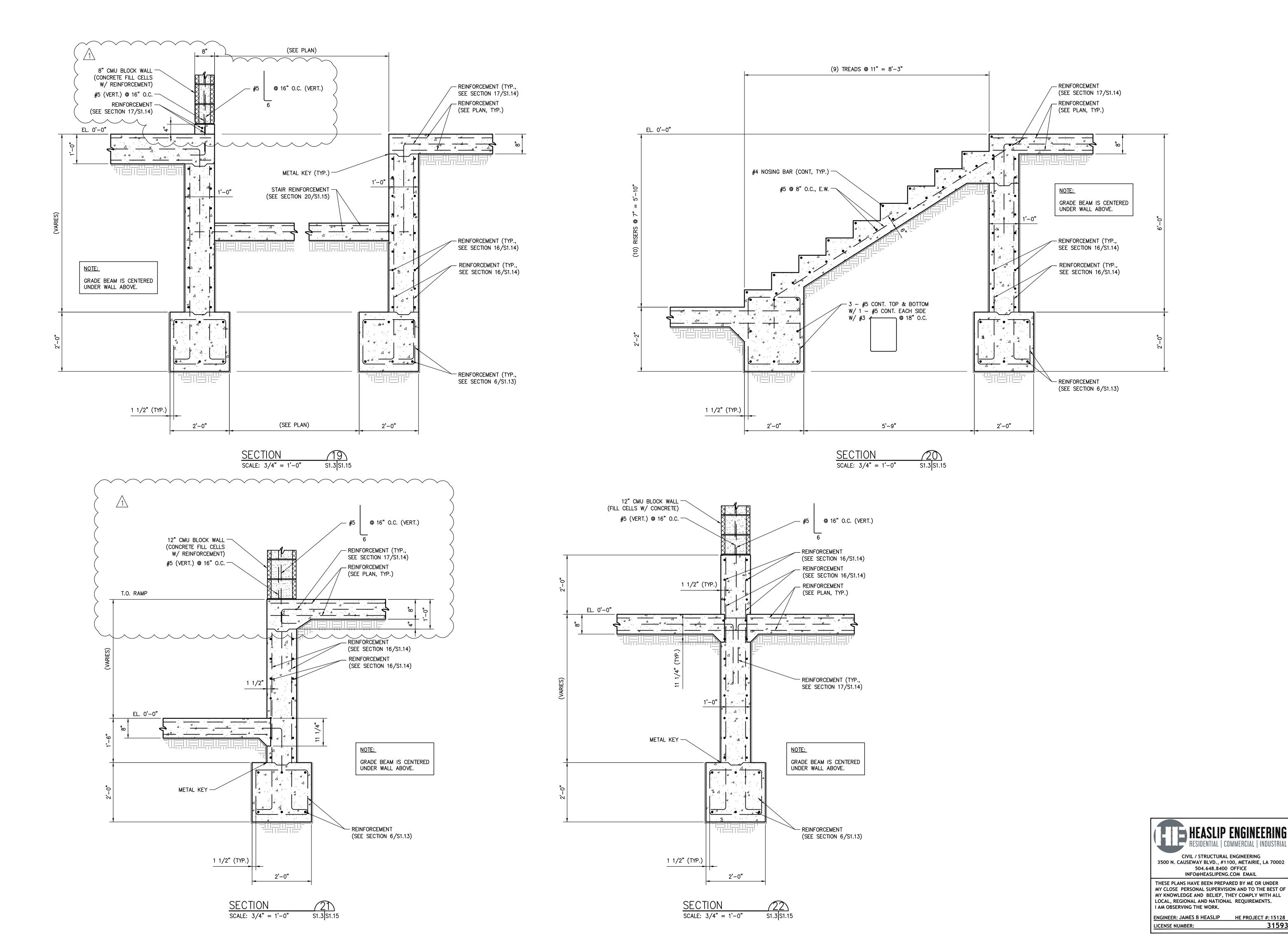
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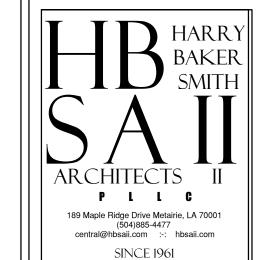
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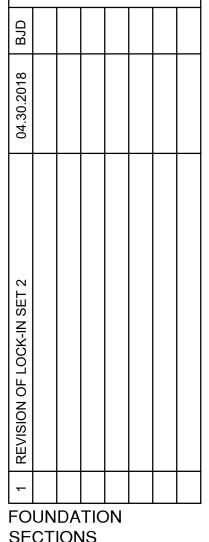


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SECTIONS

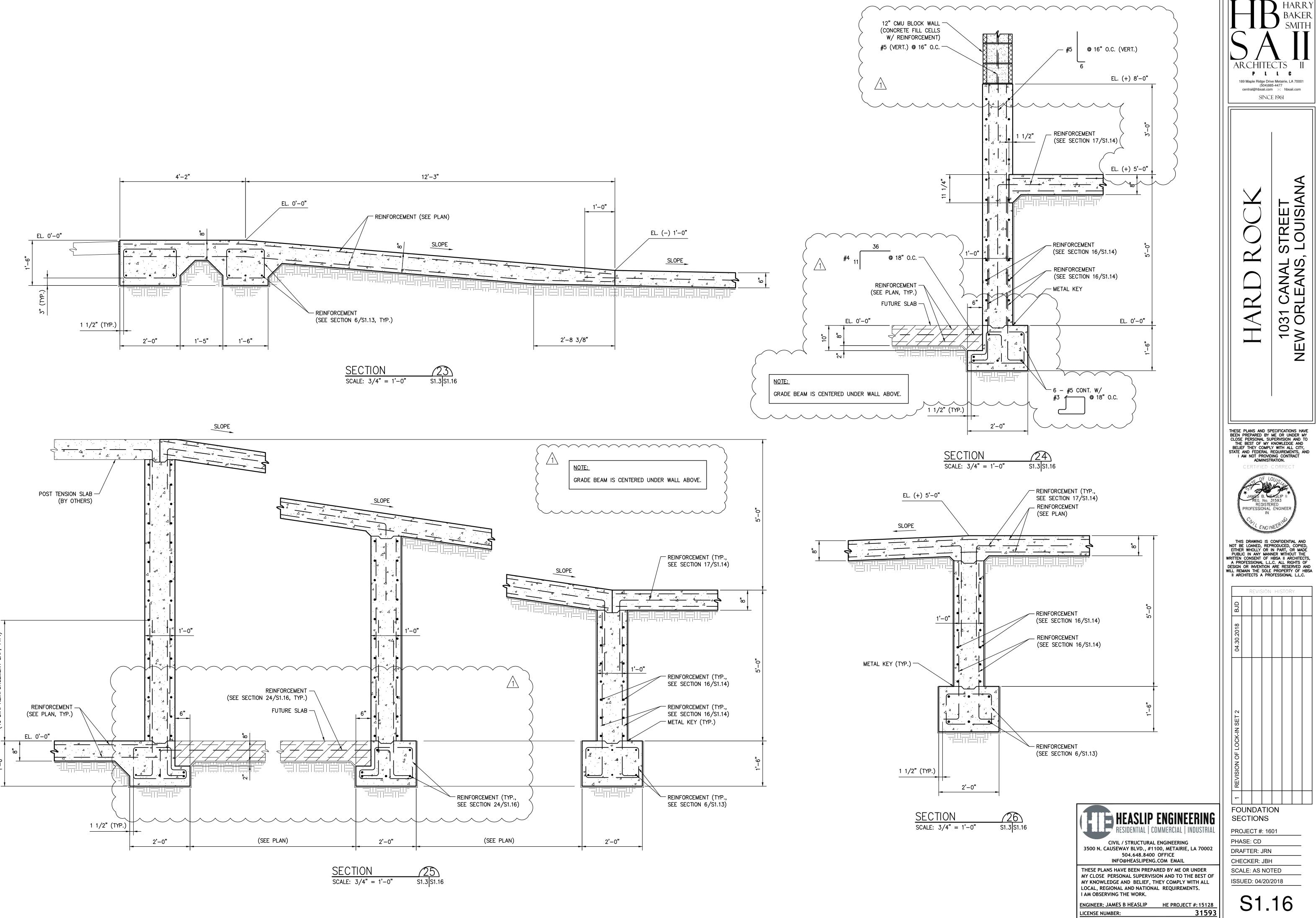
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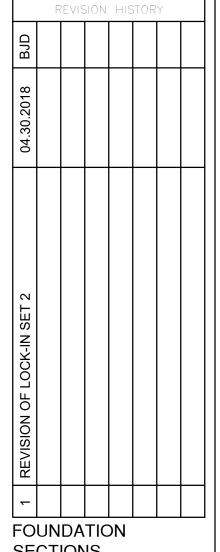
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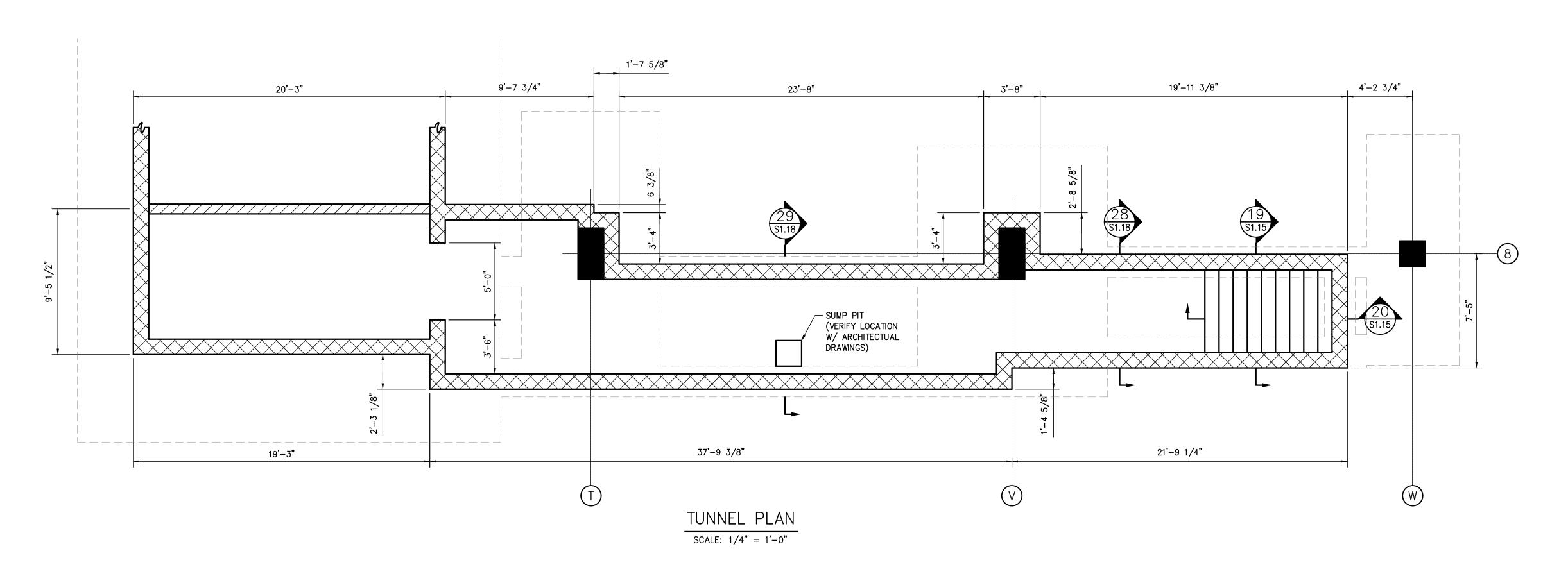
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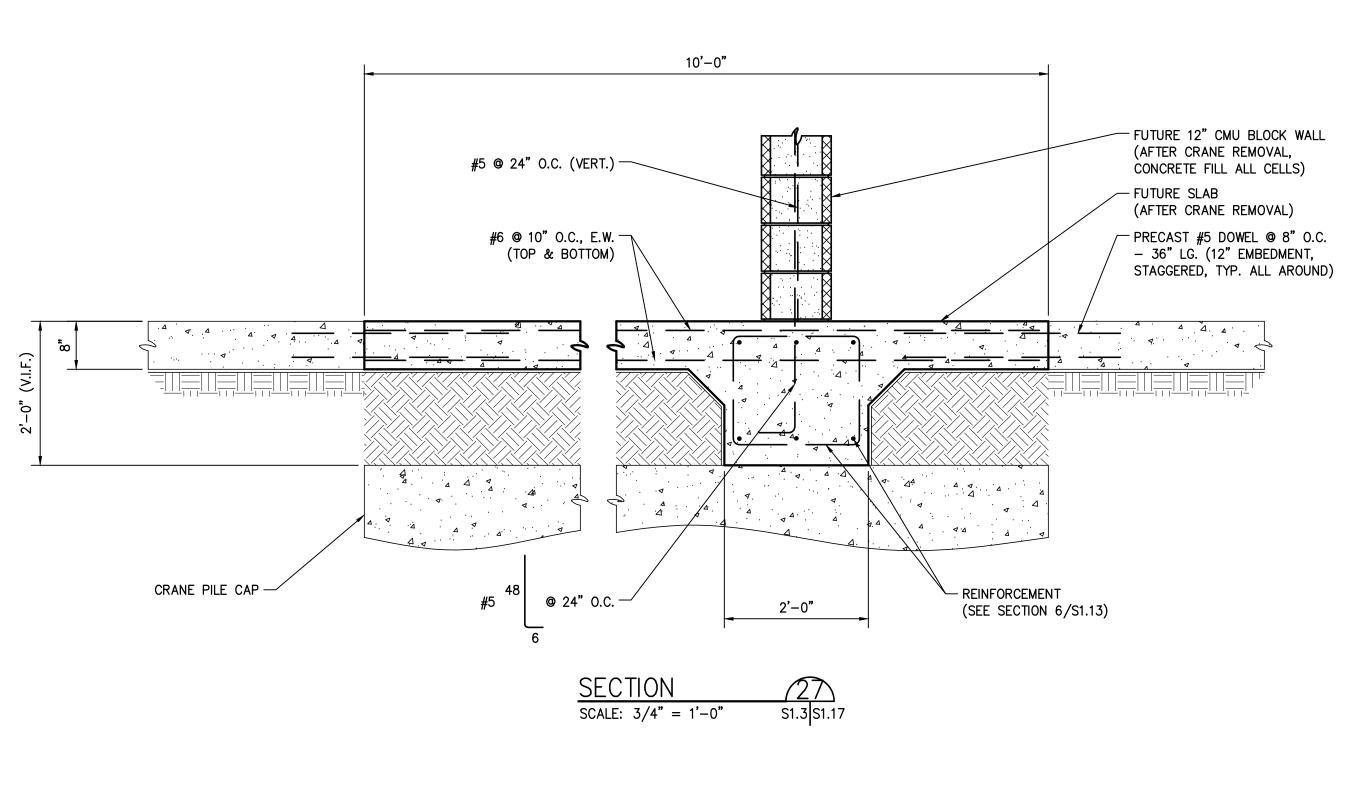
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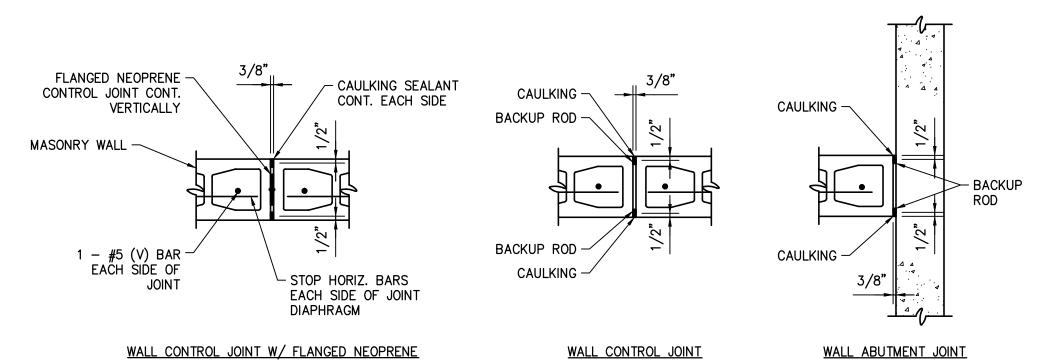
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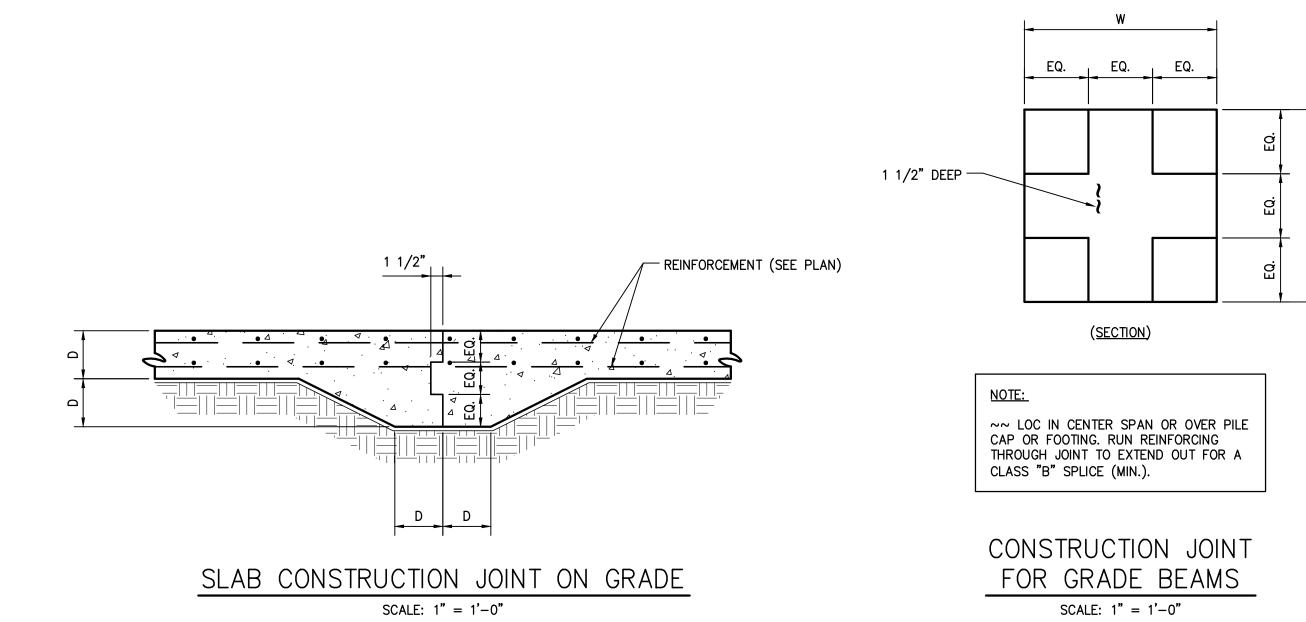


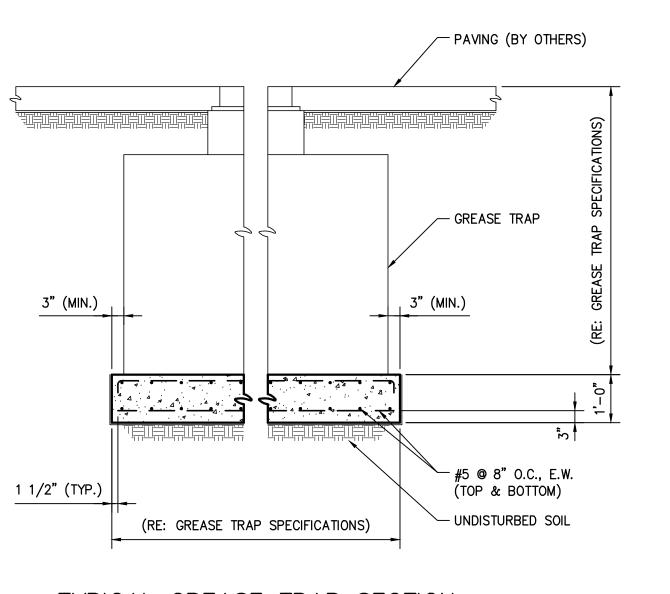




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MASONRY CONTROL JOINT DETAILS





TYPICAL GREASE TRAP SECTION

SCALE: 1/2" = 1'-0"

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128
LICENSE NUMBER: 31593

HARRY
BAKER
SMITH
SMITH
ARCHITECTS II
PLLC

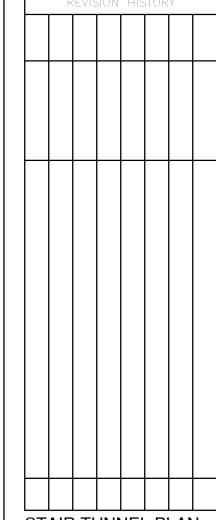
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HARD ROCK
1031 CANAL STREET
NEW ORLEANS, LOUISIANA

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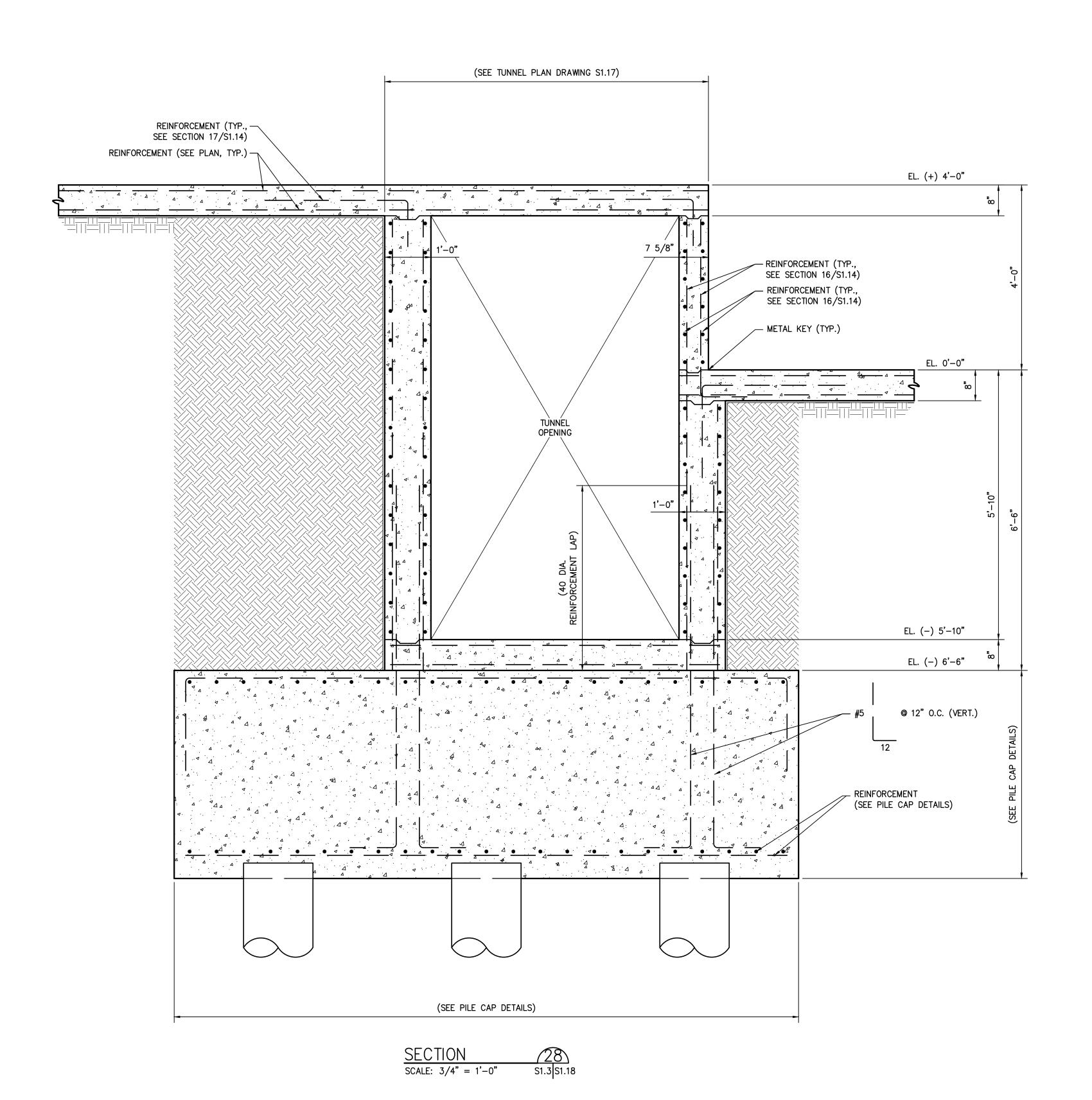


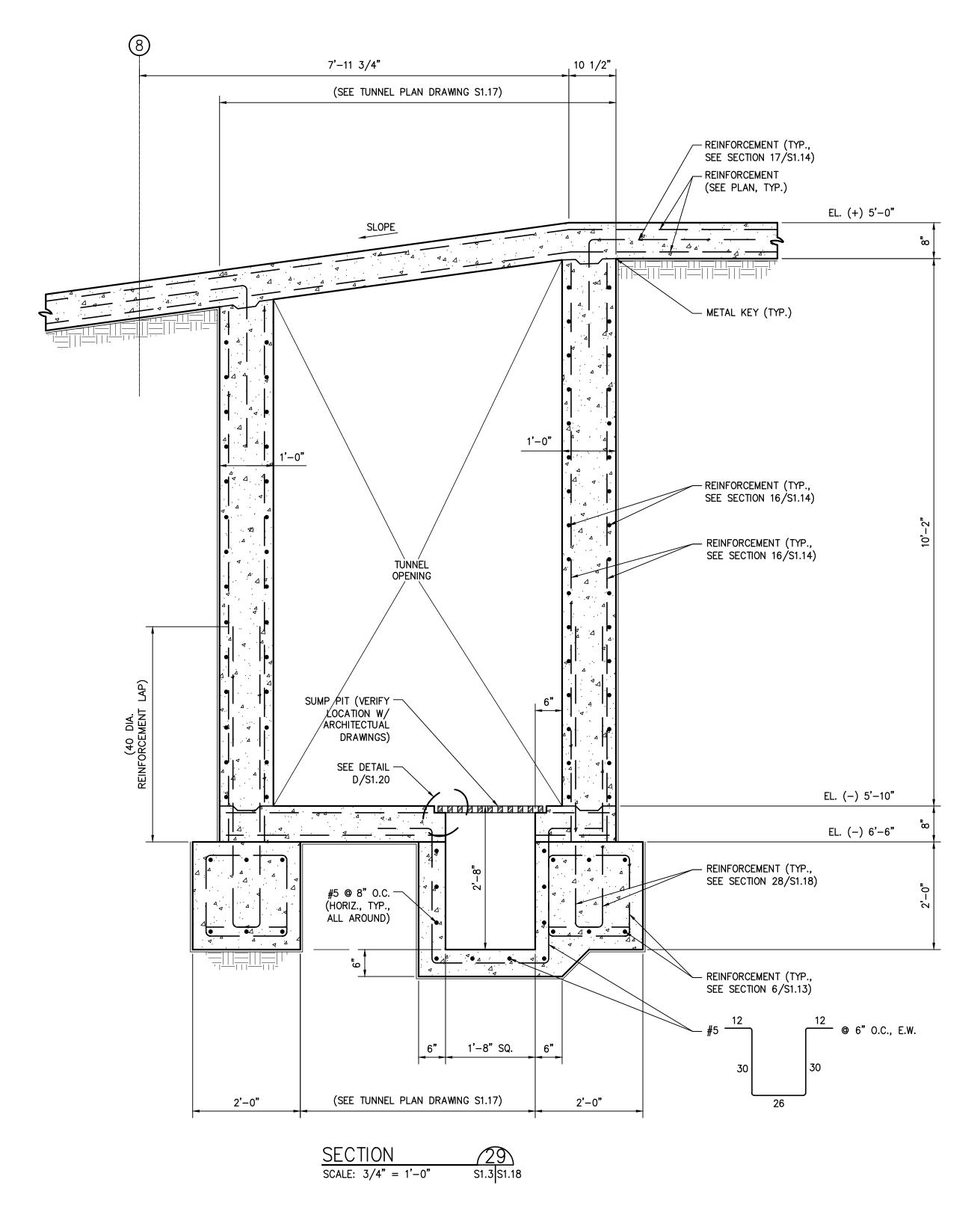
STAIR TUNNEL PLAN, SECTIONS, & DETAILS PROJECT #: 1601

PROJECT #: 1601
PHASE: CD
DRAFTER: JRN
CHECKER: JBH

SCALE: AS NOTED

ISSUED: 04/20/2018







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FOUNDATION
SECTIONS
PROJECT #: 1601
PHASE: CD
DRAFTER: JRN

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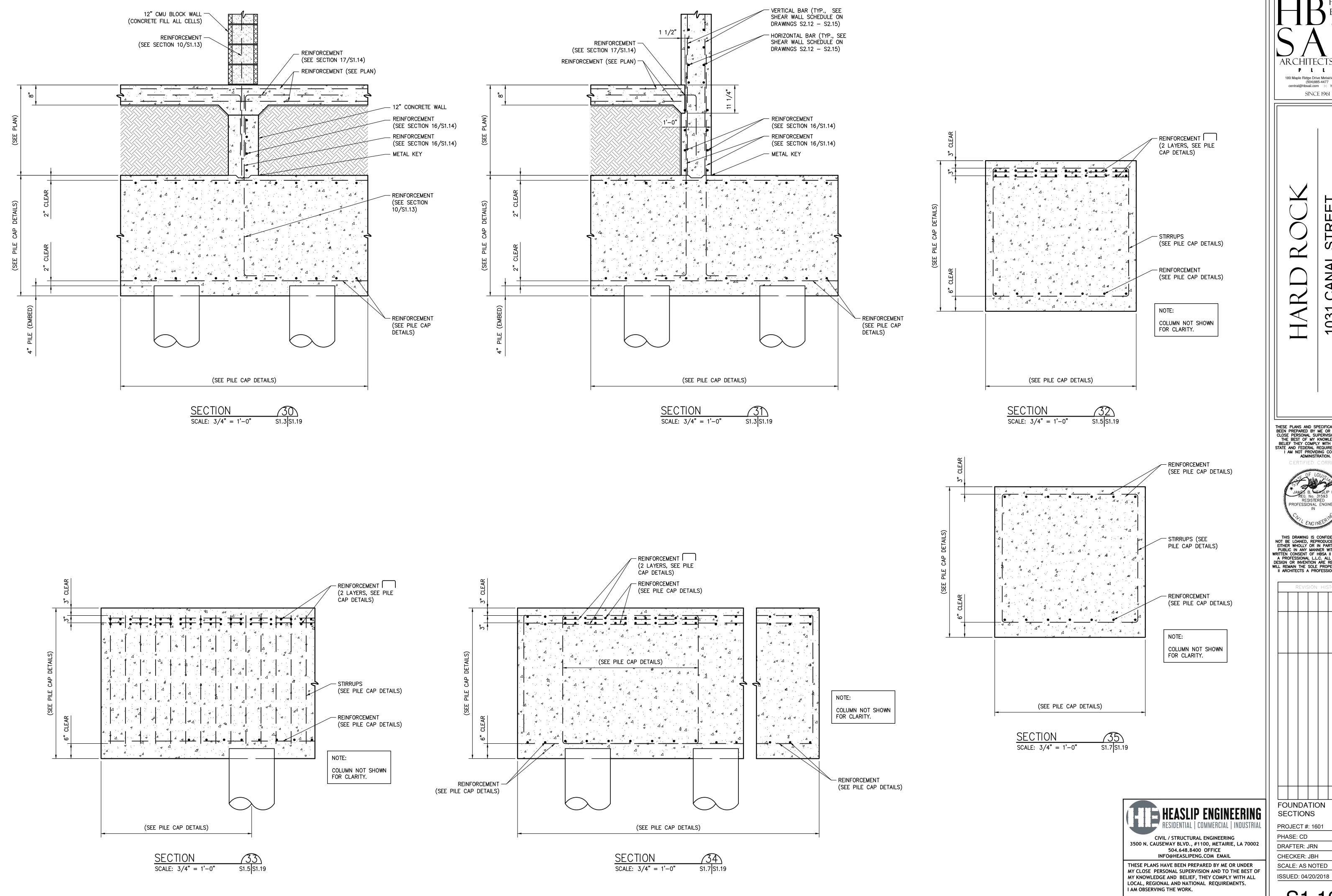
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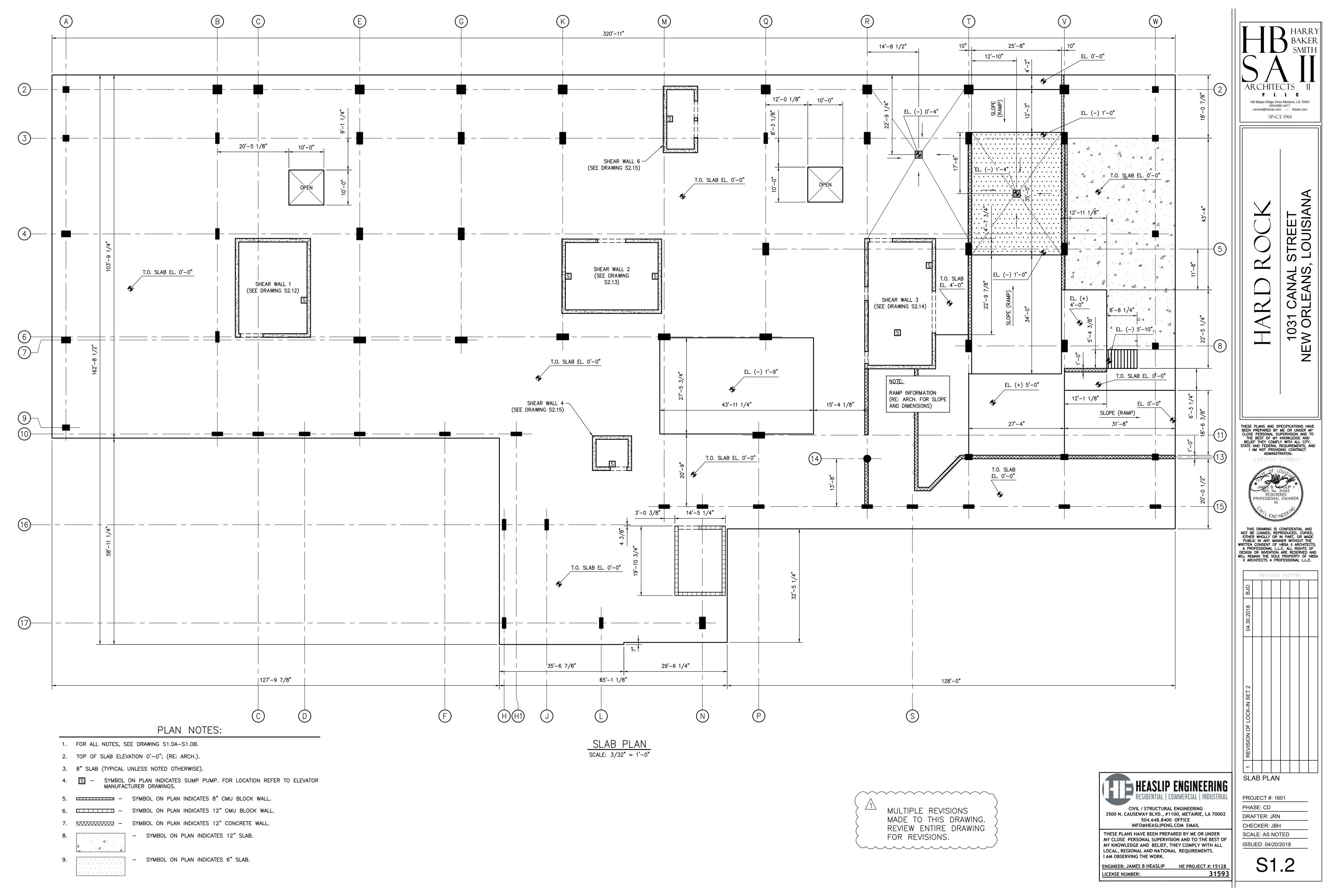
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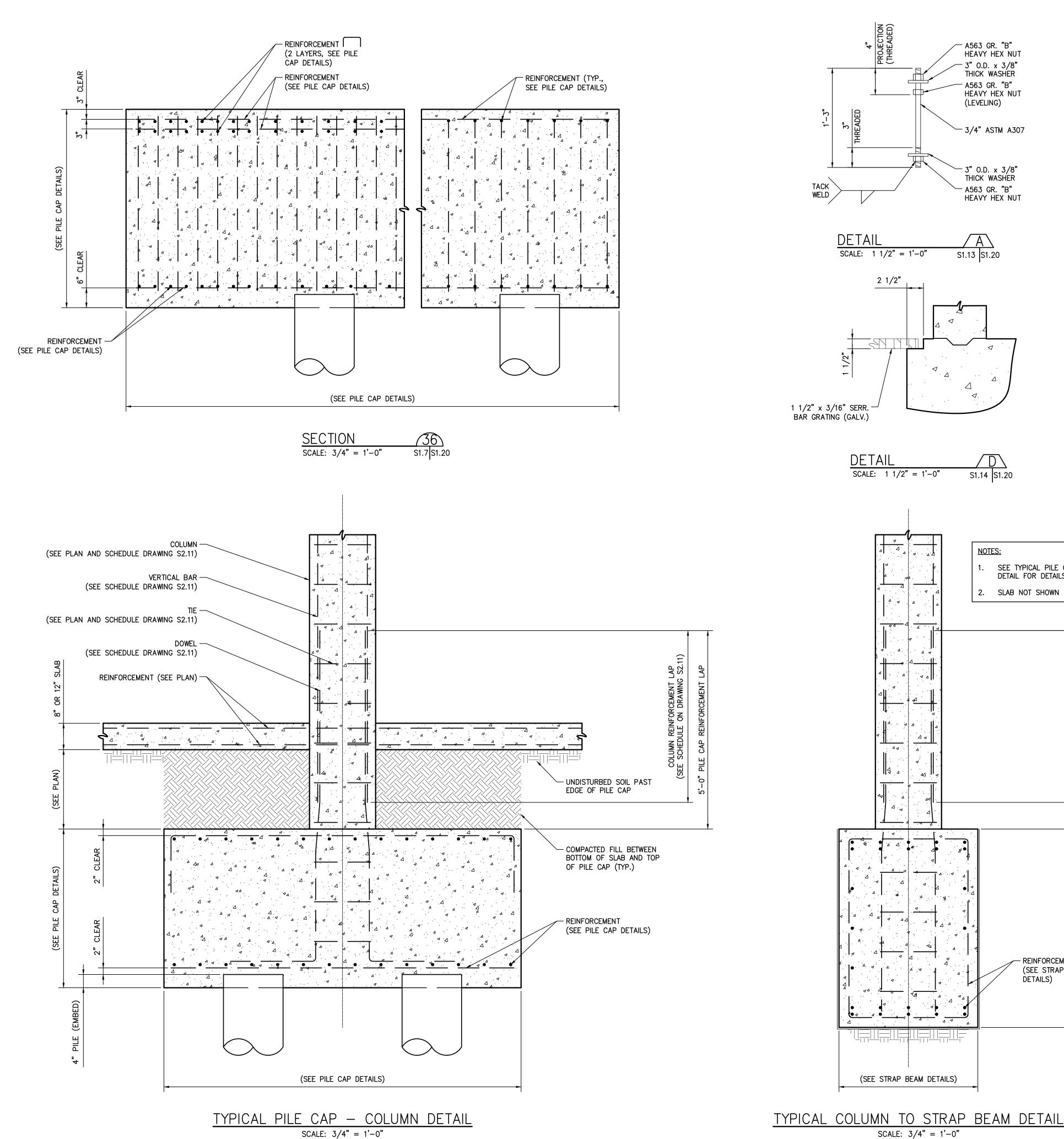
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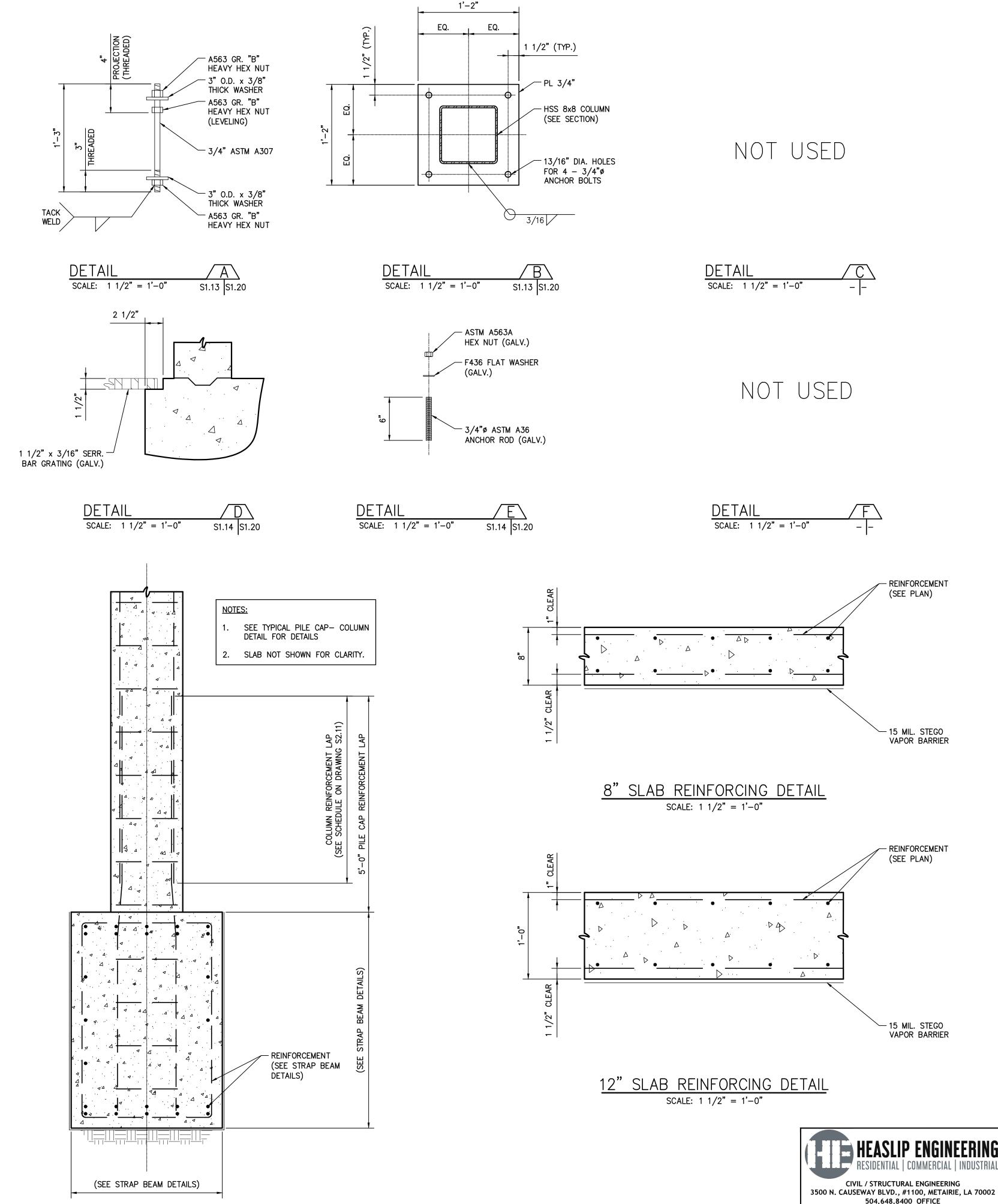
ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

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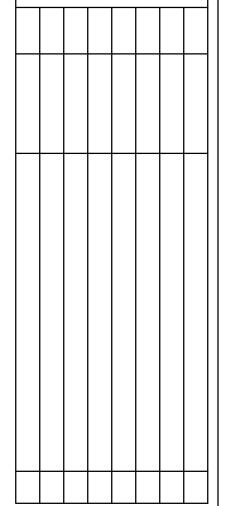


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FOUNDATION SECTION & DETAILS

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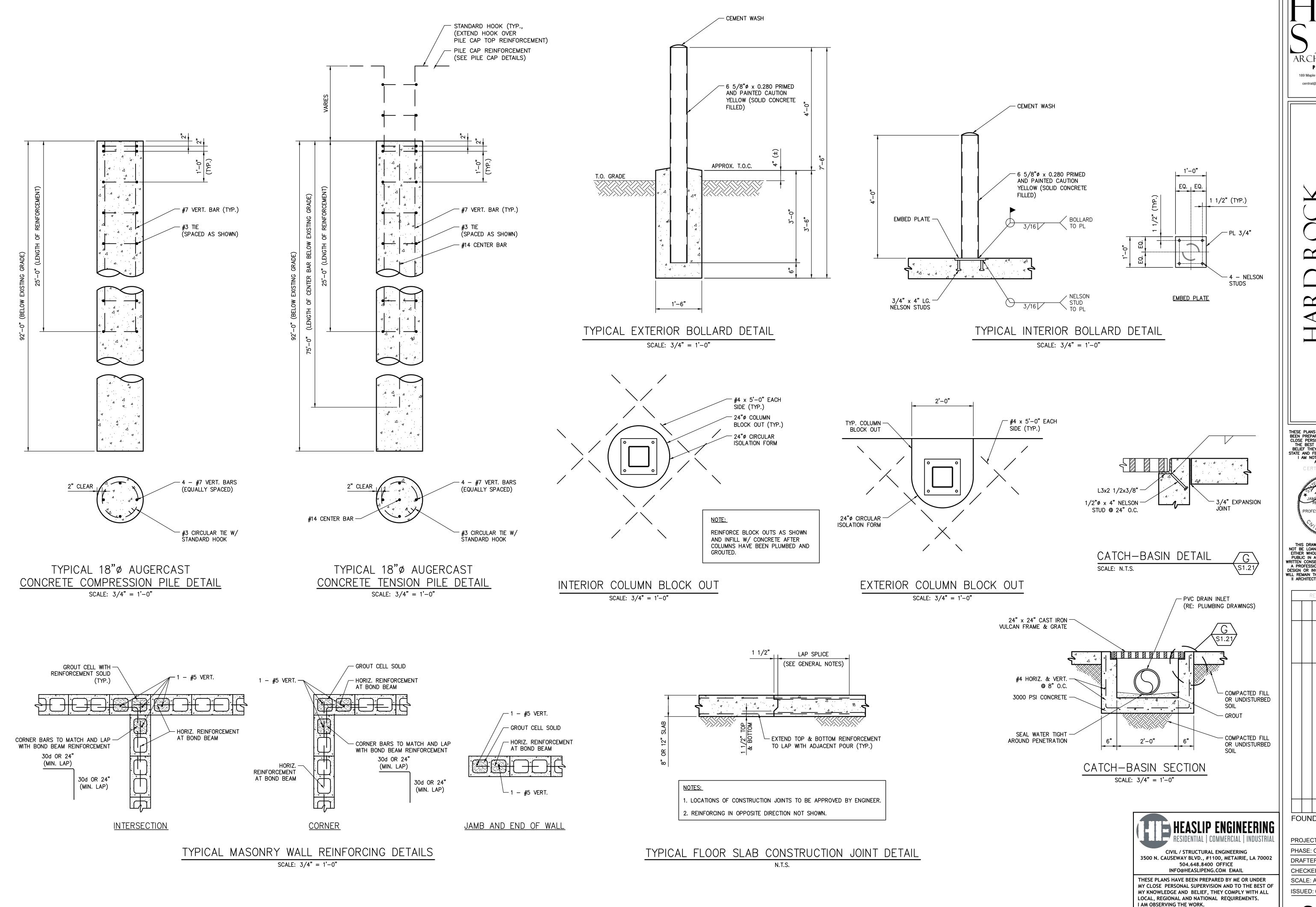
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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

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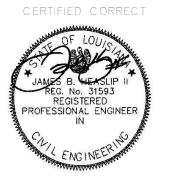
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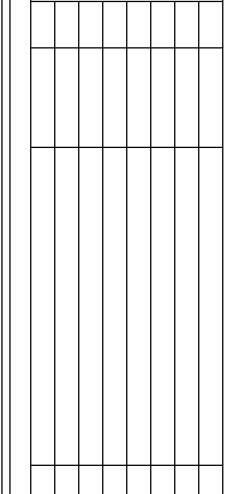
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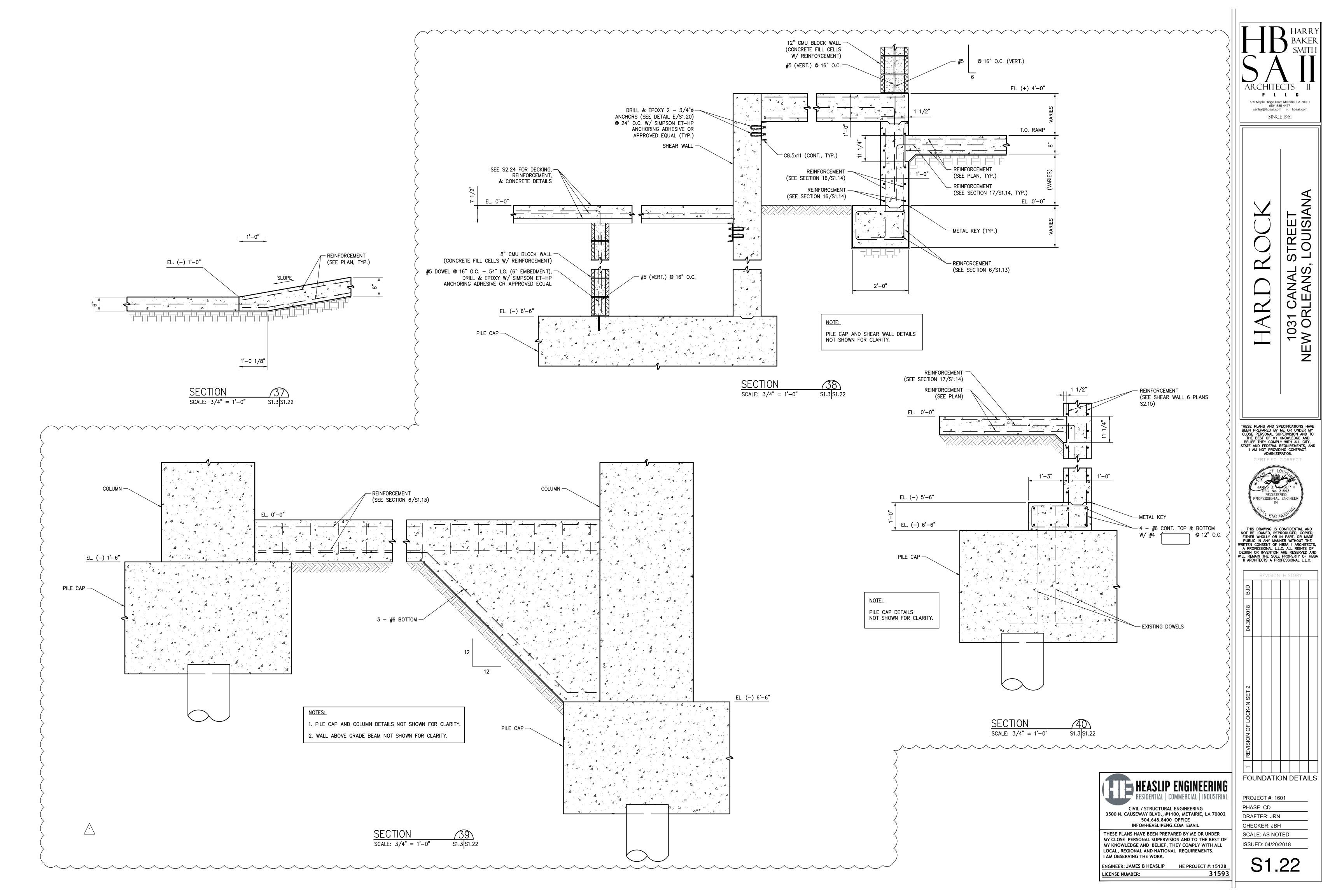
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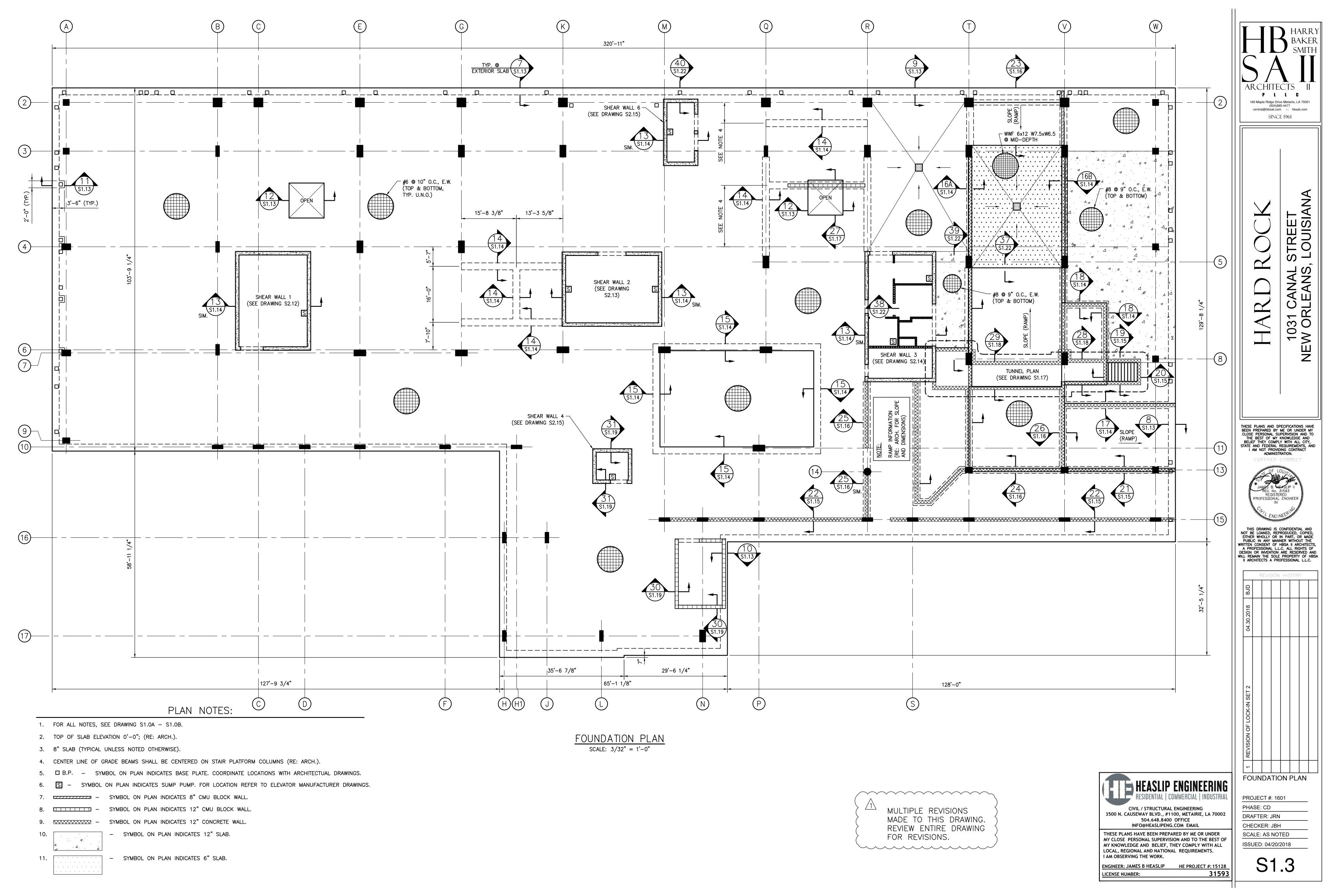
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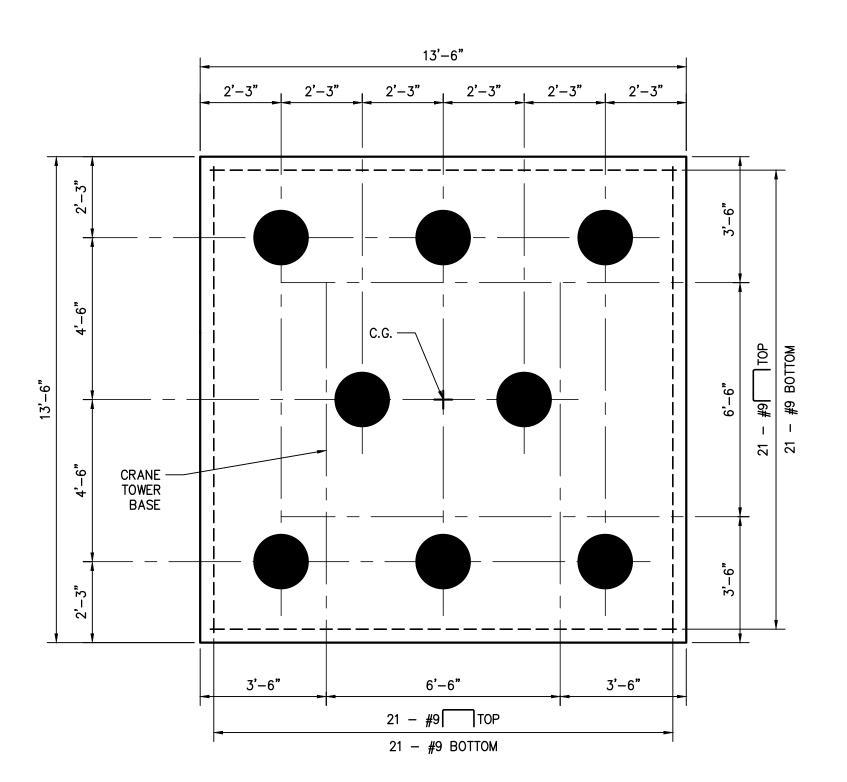
S1.21

ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

31593







CRANE TYPE 1 FOUNDATION LIEBHERR 316 EC-H 8 PILE CAP DETAIL SCALE: 3/8" = 1'-0"

	CRANE TYPE 1 LOAD TABLE										
MODE M H V Md											
IN OPERATION	3,083,010	ft-lbs	8,318	lbs	207,723	lbs	163,739	ft-lbs			
OUT OF OPERATION STORM FROM REAR	1,852,019	ft-lbs	25,628	lbs	185,018	lbs	0	ft-lbs			
OUT OF OPERATION STORM FROM FRONT	3,749,028	ft-lbs	20,458	lbs	186,142	lbs	0	ft-lbs			
IN ERECTION	1,783,425	ft-lbs	6,519	lbs	128,816	lbs	0	ft-lbs			

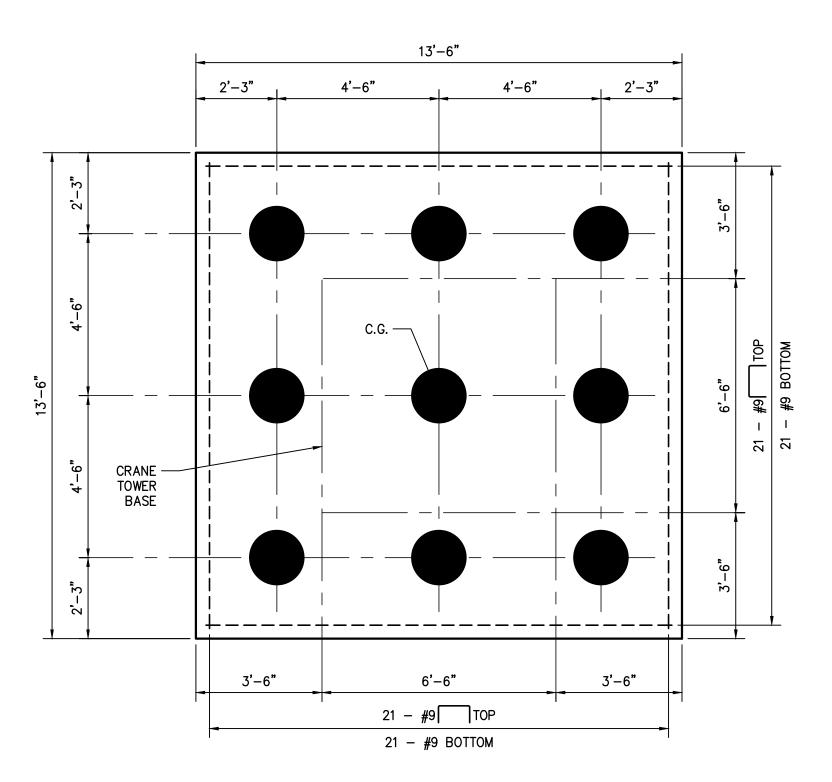
- M: OVERTURNING MOMENT
- H: HORIZONTAL LOAD AT THE TOP OF CONCRETE
- V: VERTICAL LOAD Md: SLEWING TORQUE MOMENT ABOUT THE
- CENTERLINE OF THE TOWER



TENSION PILE

ALL LOADS ARE REPORTED AT SERVICE LEVEL

ENGINEER OF RECORD SHALL BE NOTIFIED OF ANY CHANGES IN THE LOADS LISTED IN THIS TABLE.



CRANE TYPE 2 FOUNDATION LIEBHERR 316 EC-H 9 PILE CAP DETAIL SCALE: 3/8" = 1'-0"

CRANE TYPE 2 LOAD TABLE											
MODE M H V Md											
IN OPERATION	3,203,232	ft-lbs	10,341	lbs	230,204	lbs	211,680	ft-lbs			
OUT OF OPERATION STORM FROM REAR	3,567,588	ft-lbs	30,799	lbs	216,266	lbs	0	ft-lbs			
OUT OF OPERATION STORM FROM FROMT	5,017,635	ft-lbs	24,729	lbs	216,266	lbs	0	ft-lbs			
IN ERECTION	2,188,347	ft-lbs	7,868	lbs	147,025	lbs	0	ft-lbs			

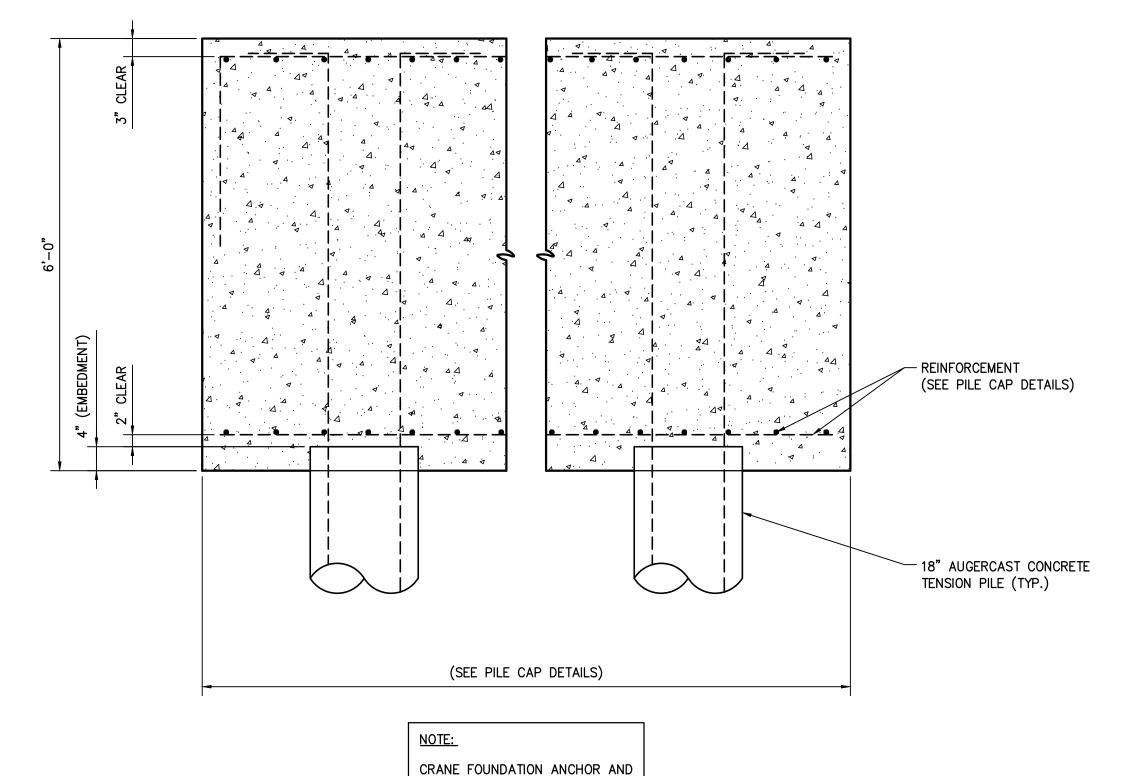
- M: OVERTURNING MOMENT
- H: HORIZONTAL LOAD AT THE TOP OF CONCRETE
- V: VERTICAL LOAD Md: SLEWING TORQUE MOMENT ABOUT THE CENTERLINE OF THE TOWER

TENSION PILE

ALL LOADS ARE REPORTED AT SERVICE LEVEL

NOTE:

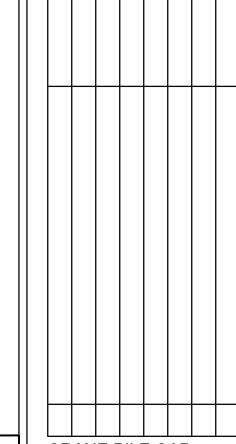
ENGINEER OF RECORD SHALL BE NOTIFIED OF ANY CHANGES IN THE LOADS LISTED IN THIS TABLE.



CRANE PILE CAP SECTION SCALE: 3/4" = 1'-0"

CONCRETE LEVELING PADS NOT

SHOWN FOR CLARITY.



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CRANE PILE CAP **DETAILS**

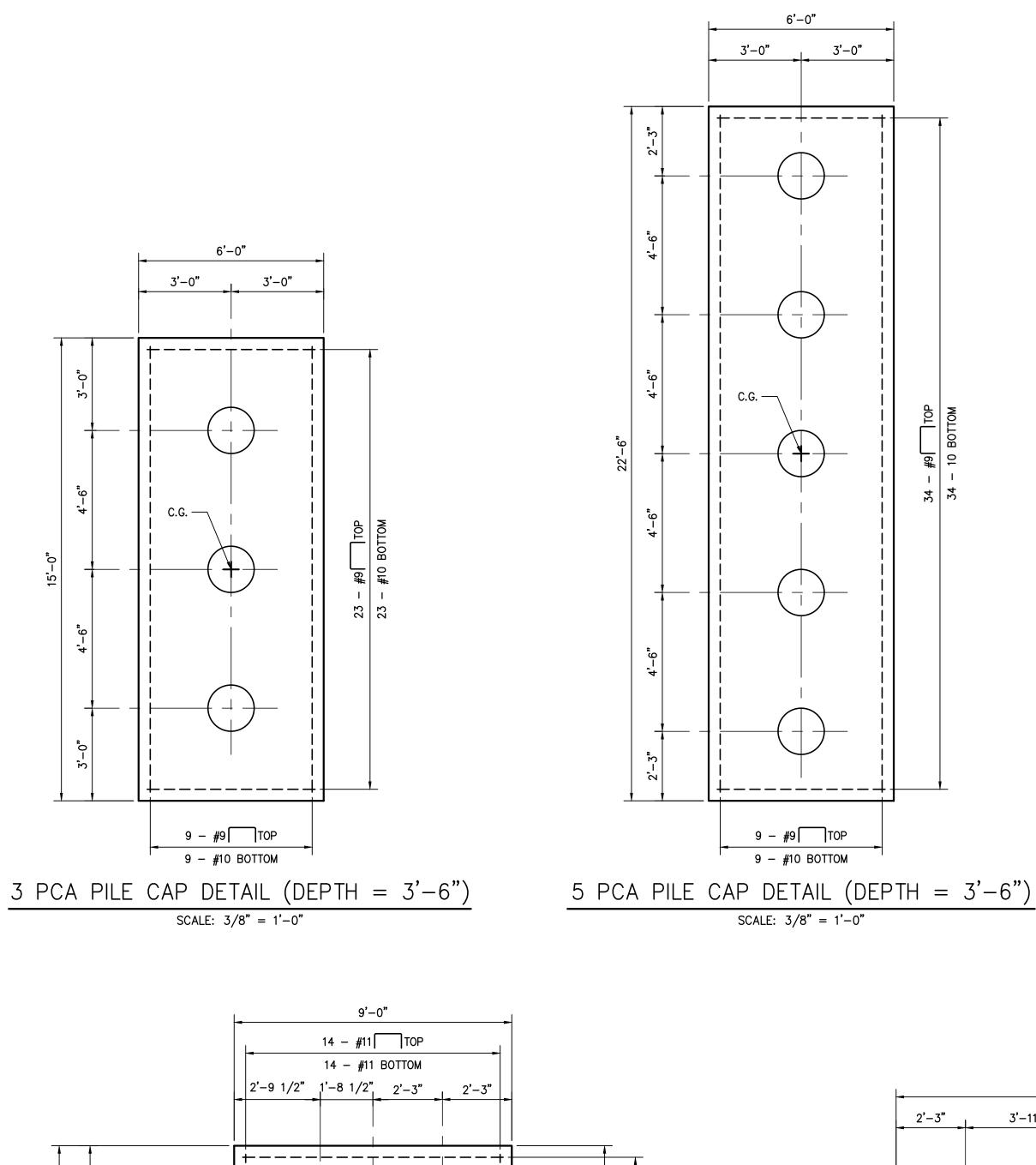
PROJECT #: 1601 PHASE: CD DRAFTER: JRN

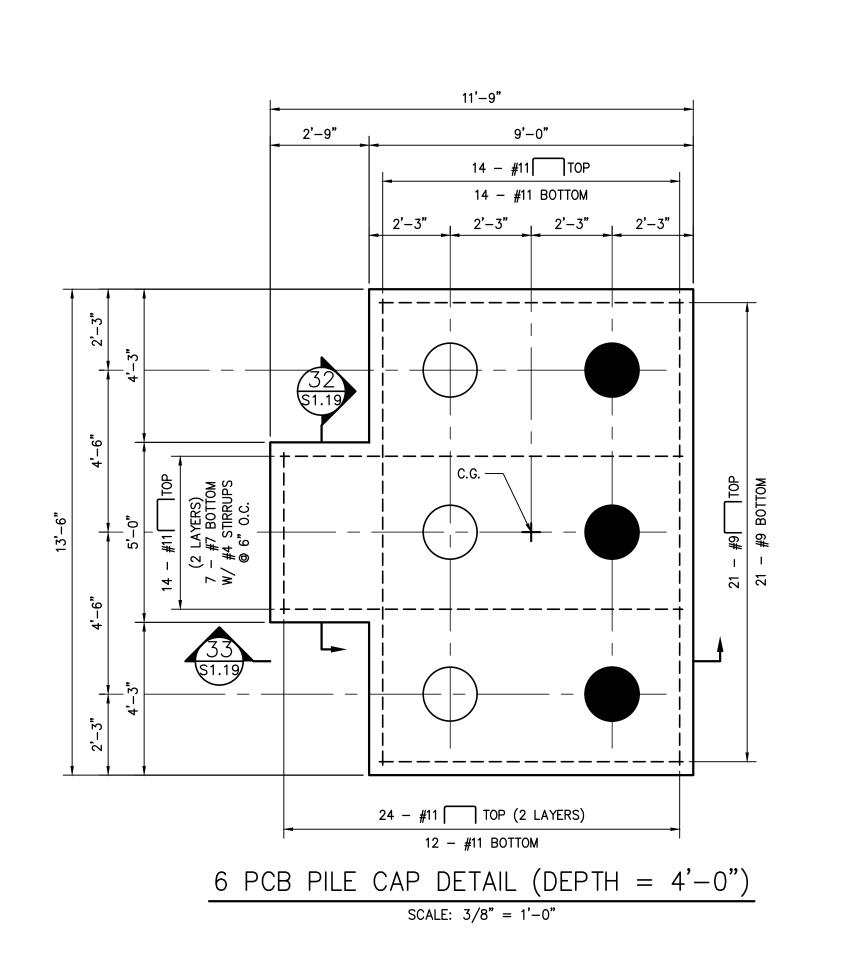
504.648.8400 OFFICE INFO@HEASLIPENG.COM EMAIL CHECKER: JBH SCALE: AS NOTED ISSUED: 04/20/2018

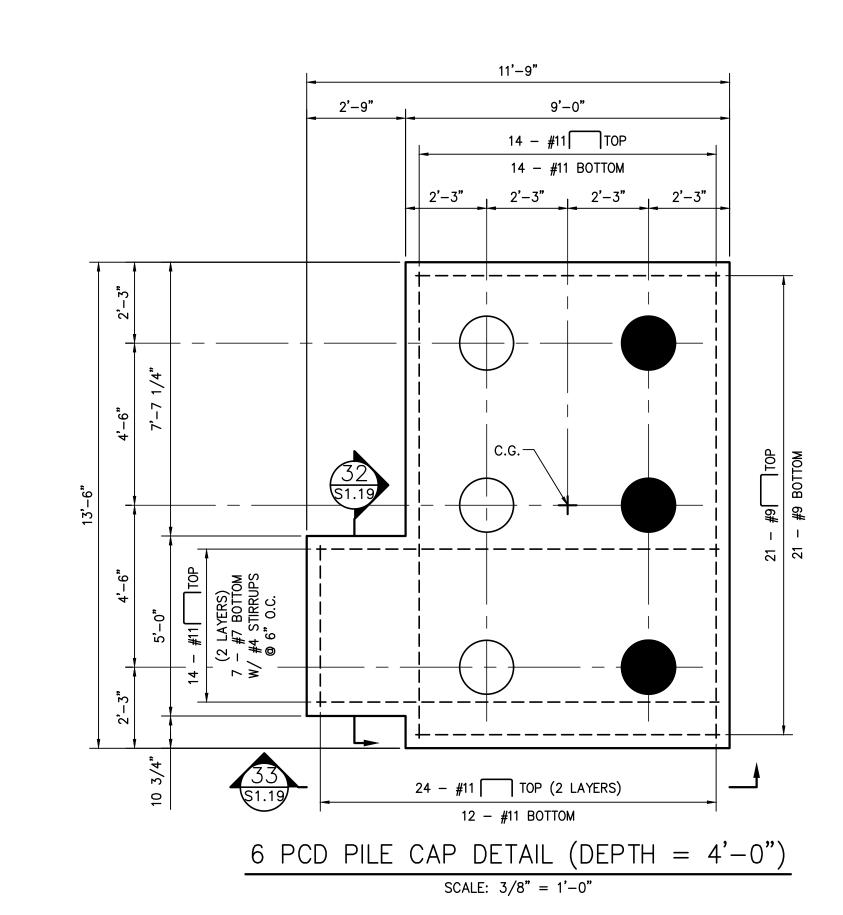
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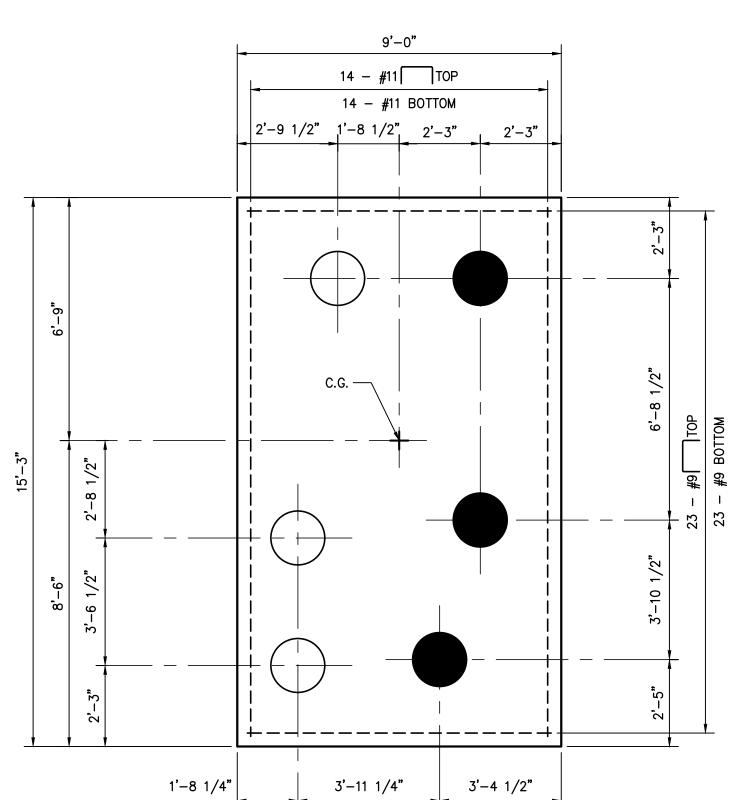
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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593 LICENSE NUMBER:

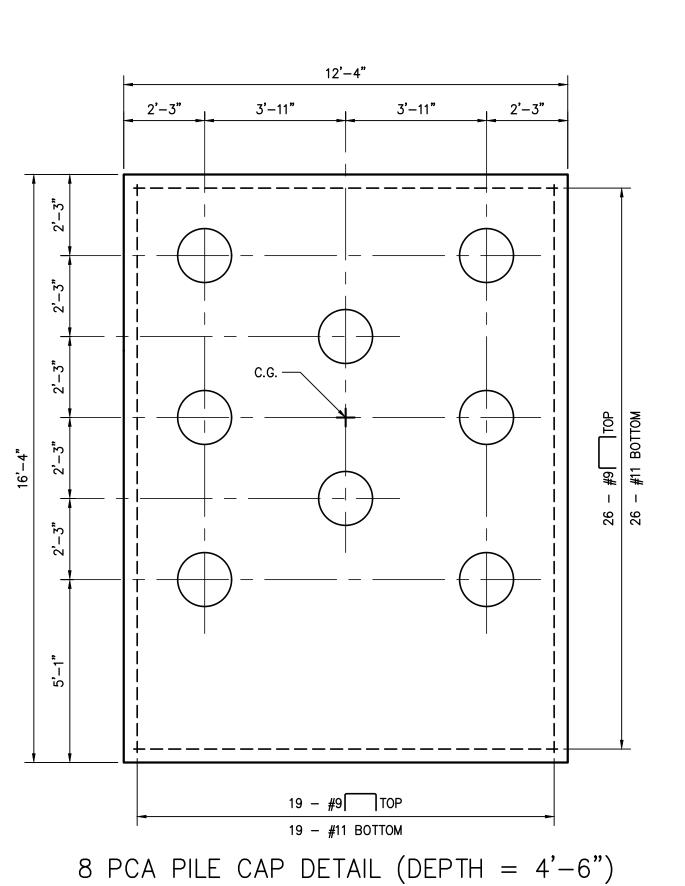




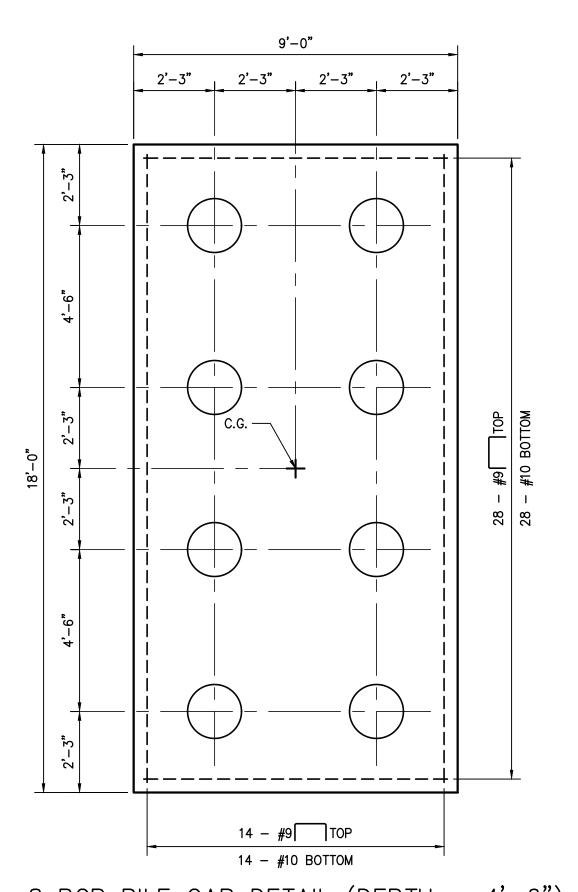






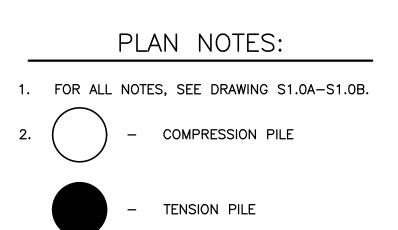


SCALE: 3/8" = 1'-0"



8 PCB PILE CAP DETAIL (DEPTH = 4'-6")

SCALE: 3/8" = 1'-0"





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LICENSE NUMBER: 31593

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REVISION HISTORY

PILE CAP DETAILS

SCALE: AS NOTED

ISSUED: 04/20/2018

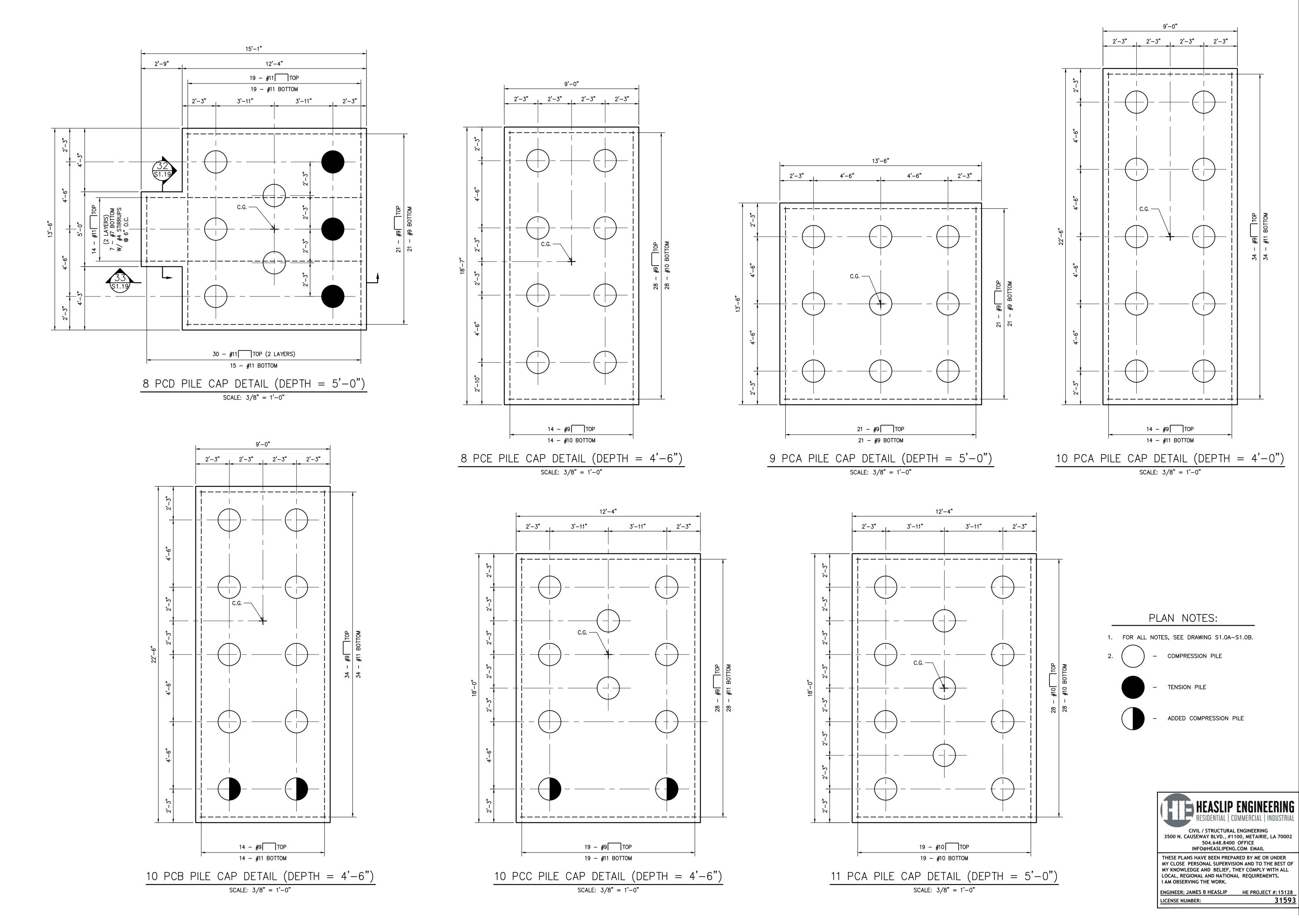
\$1.5

PROJECT #: 1601

PHASE: CD

DRAFTER: JRN

CHECKER: JBH

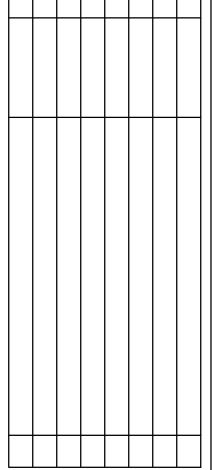


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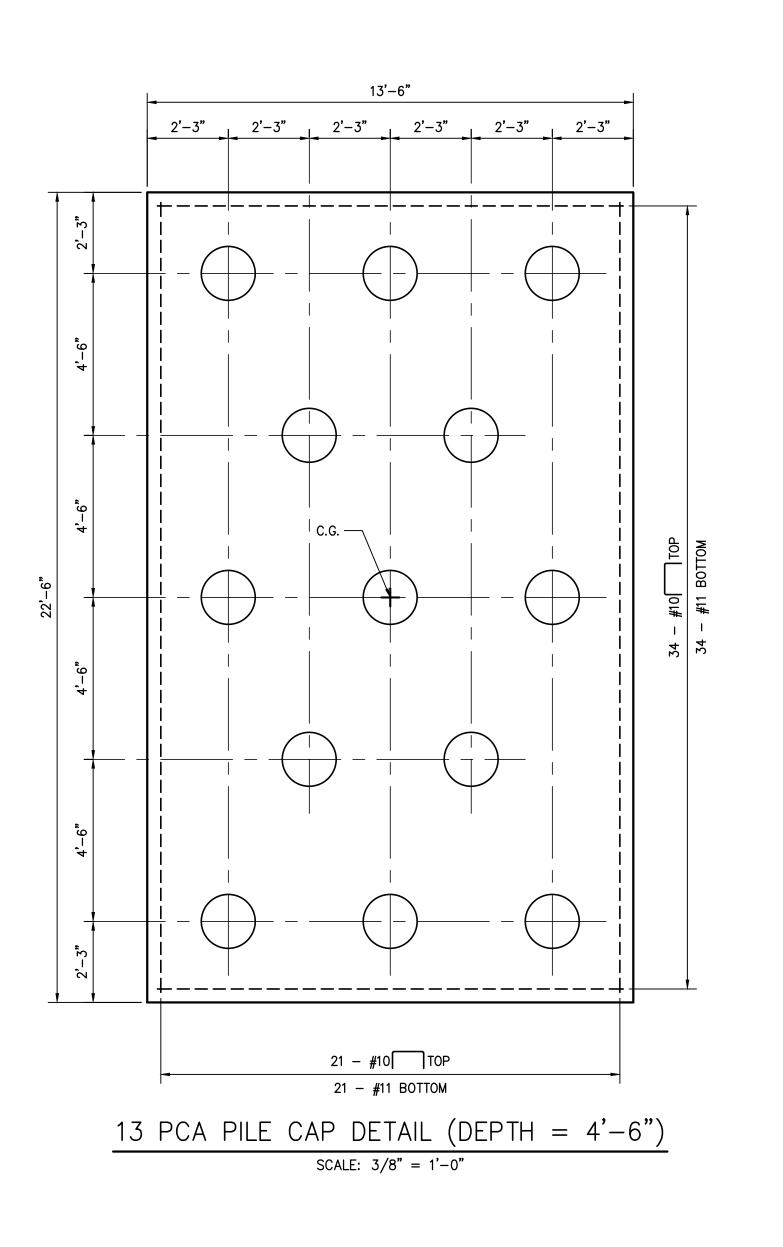


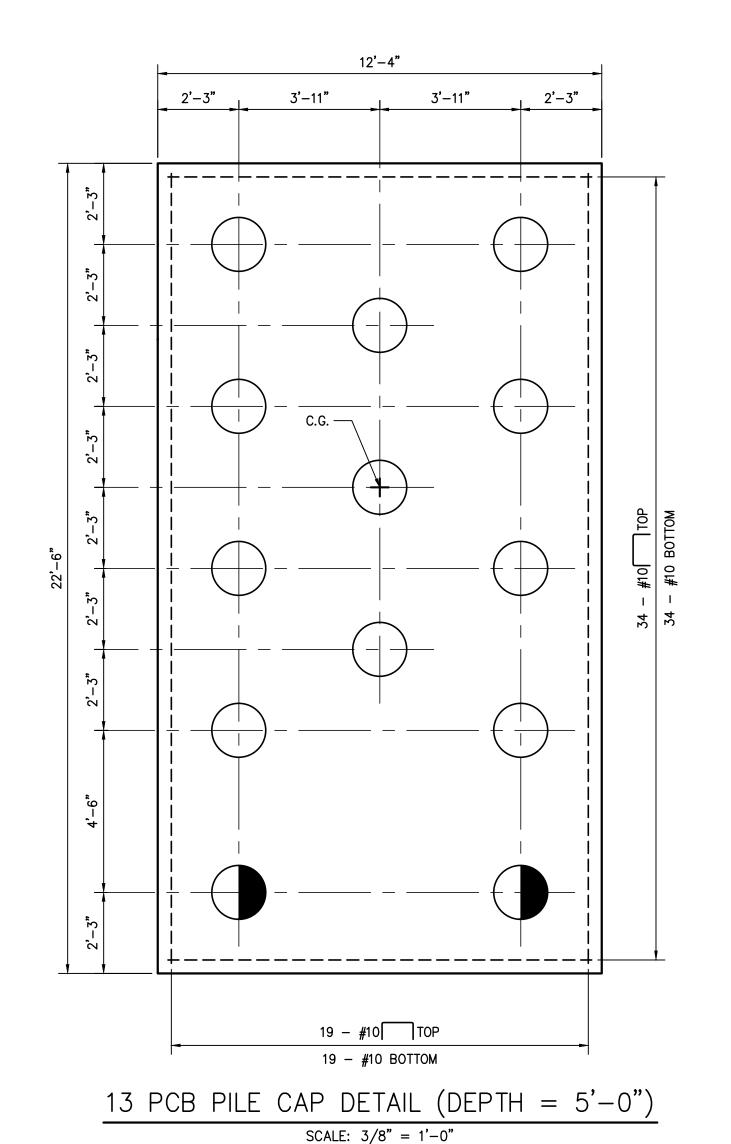
PILE CAP DETAILS

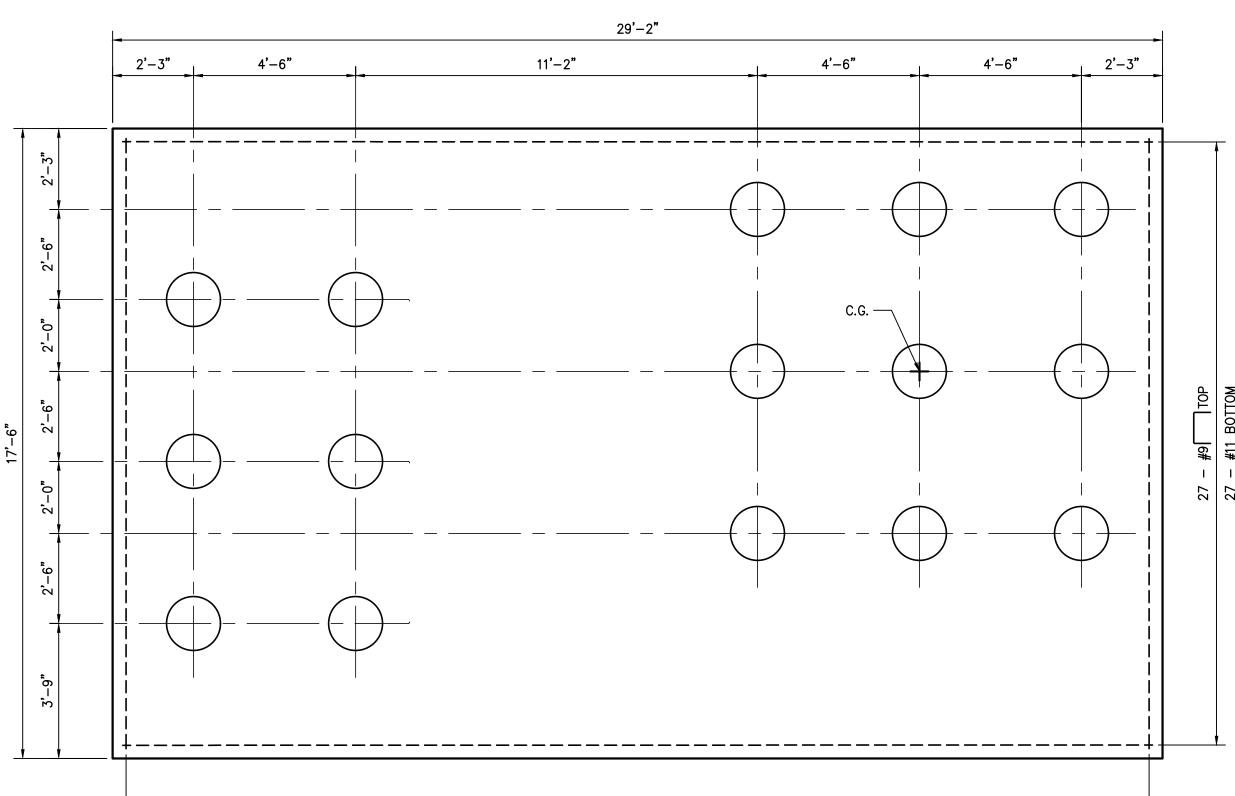
PROJECT #: 1601 PHASE: CD DRAFTER: JRN

CHECKER: JBH SCALE: AS NOTED ISSUED: 04/20/2018

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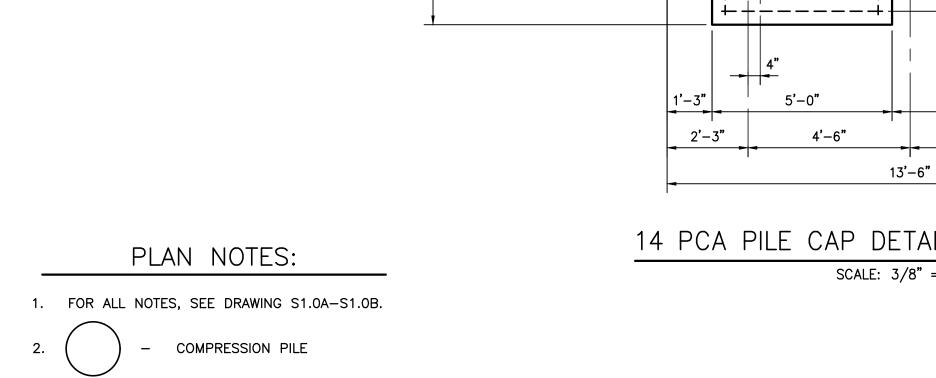


46 - #9 TOP

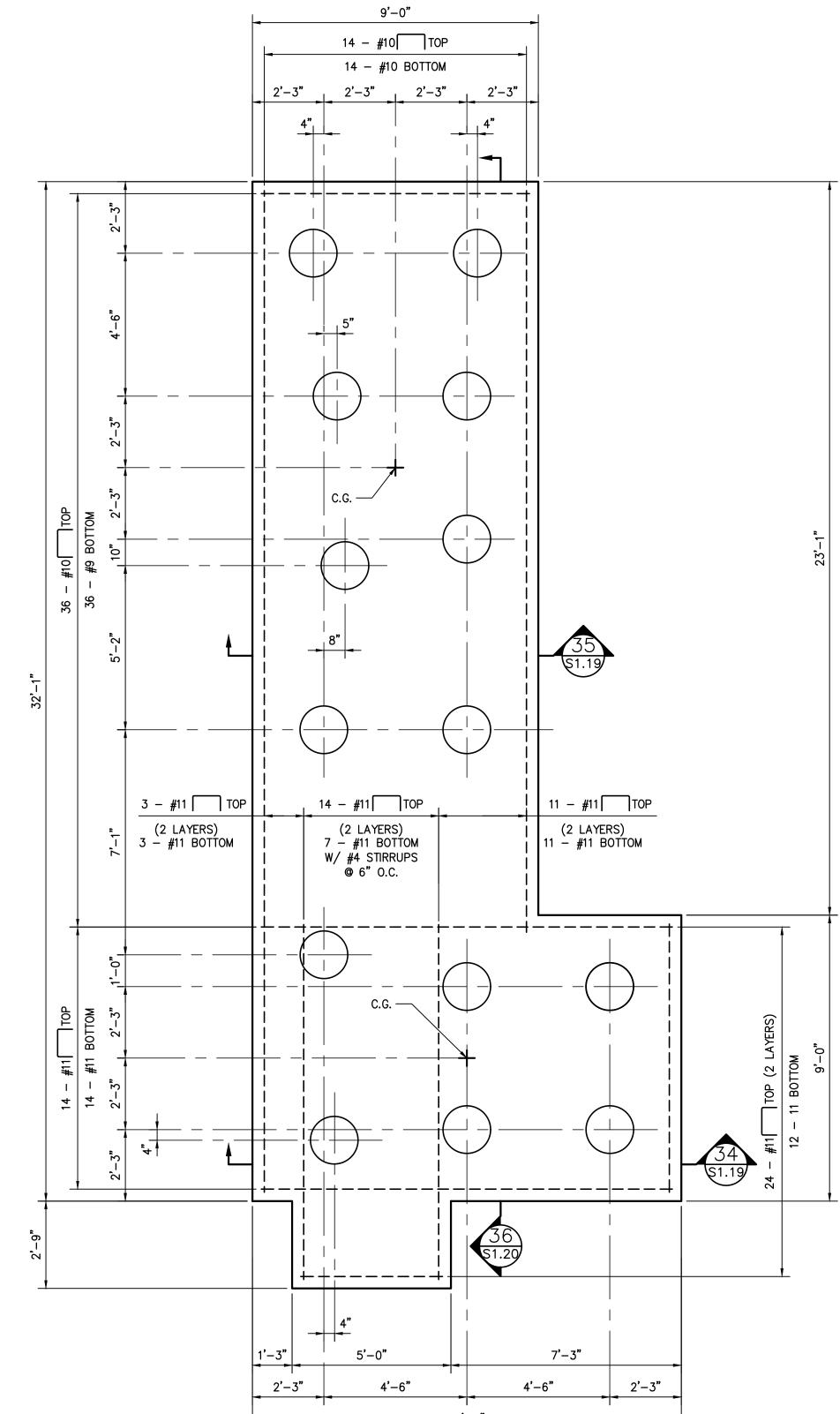
46 - #11 BOTTOM

15 PCA PILE CAP DETAIL (DEPTH = 4'-0")

SCALE: 3/8" = 1'-0"



ADDED COMPRESSION PILE



14 PCA PILE CAP DETAIL (DEPTH = 5'-0") SCALE: 3/8" = 1'-0"



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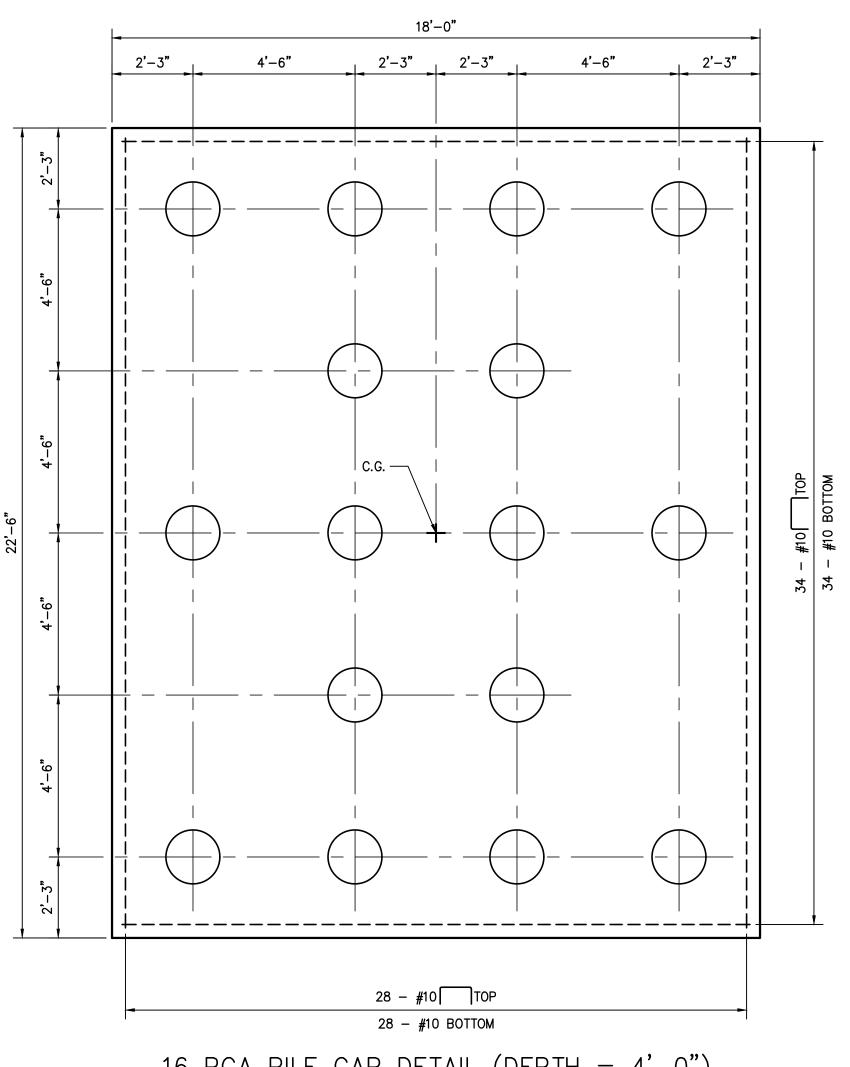
S1.7

PILE CAP DETAILS

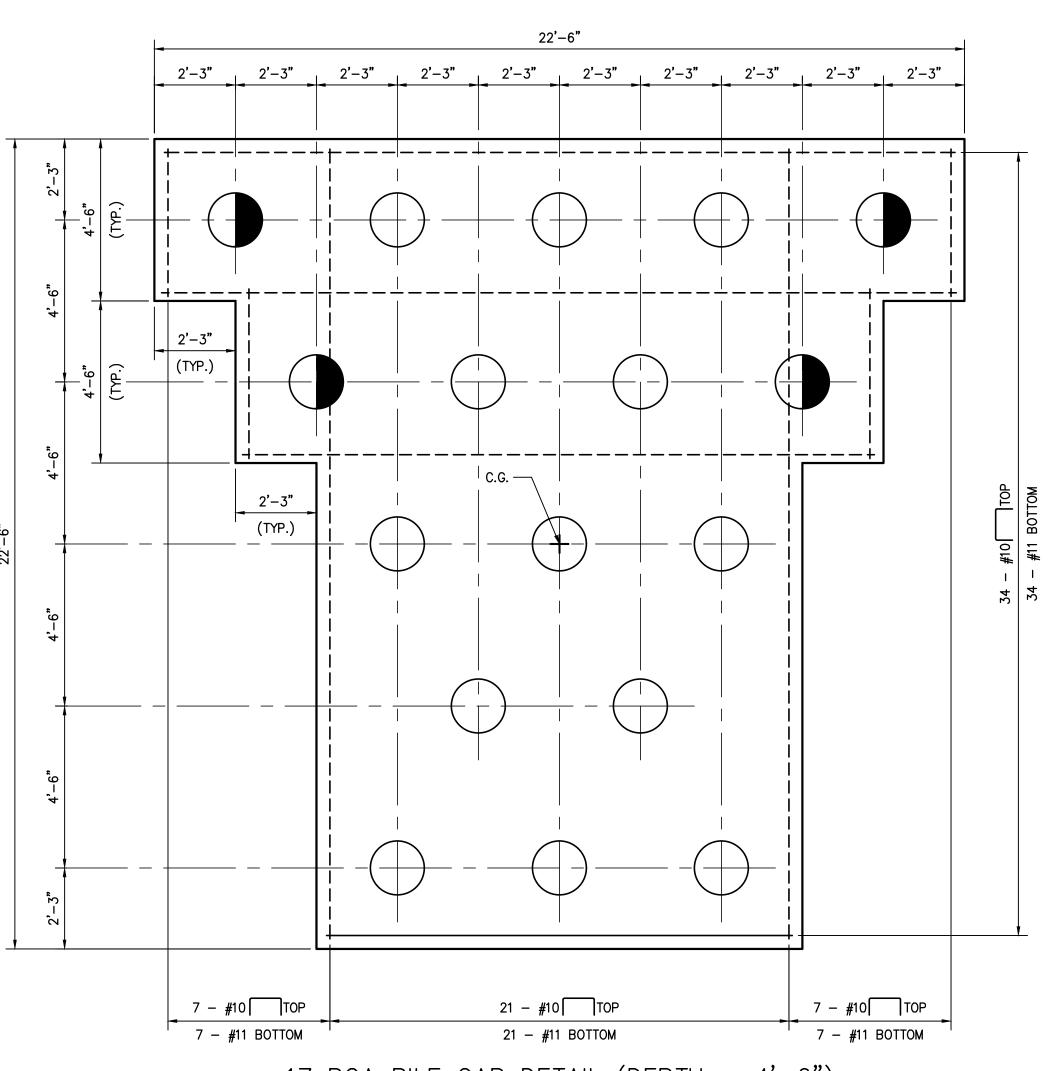
PROJECT #: 1601 PHASE: CD DRAFTER: JRN CHECKER: JBH SCALE: AS NOTED ISSUED: 04/20/2018

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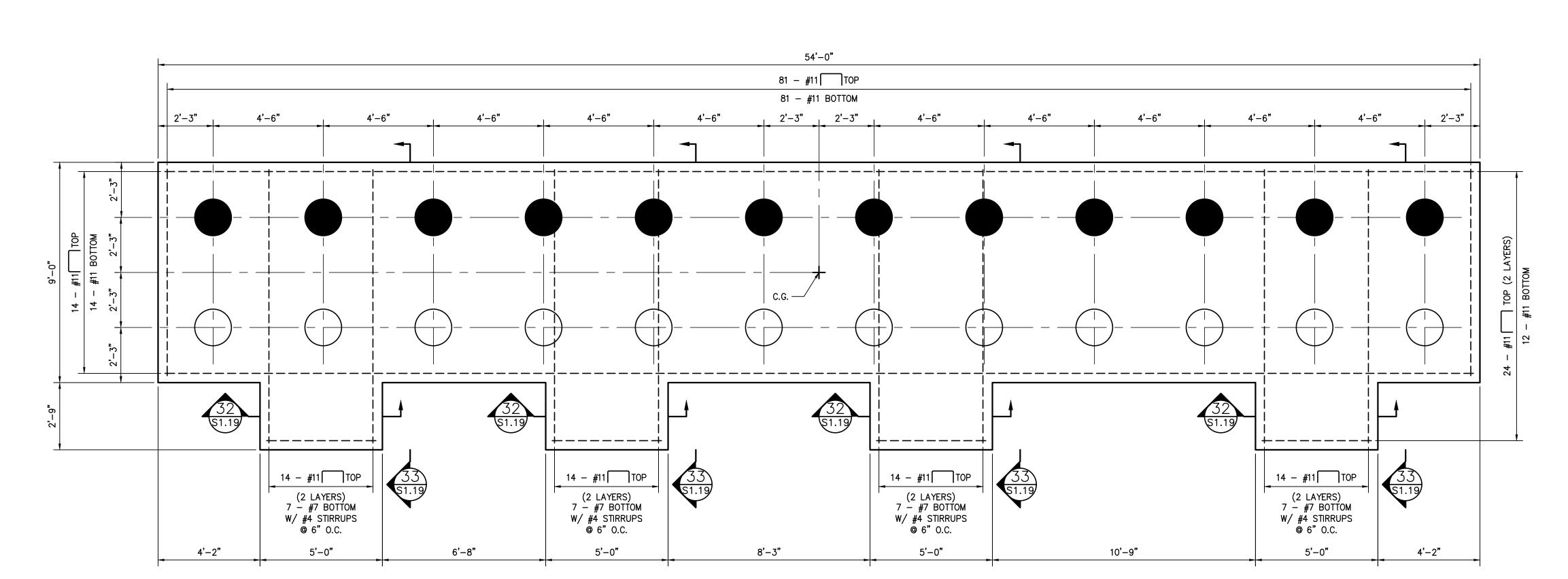
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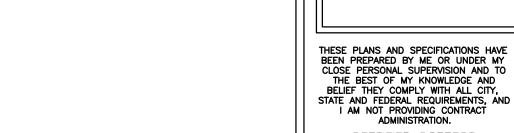
16 PCA PILE CAP DETAIL (DEPTH = 4'-0") SCALE: 3/8" = 1'-0"



17 PCA PILE CAP DETAIL (DEPTH = 4'-6") SCALE: 3/8" = 1'-0"



24 PCA PILE CAP DETAIL (DEPTH = 4'-6") SCALE: 3/8" = 1'-0"





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PLAN NOTES:

1. FOR ALL NOTES, SEE DRAWING S1.0A-S1.0B.

COMPRESSION PILE

ADDED COMPRESSION PILE

TENSION PILE

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593

PILE CAP DETAILS HEASLIP ENGINEERING PROJECT #: 1601 DRAFTER: JRN

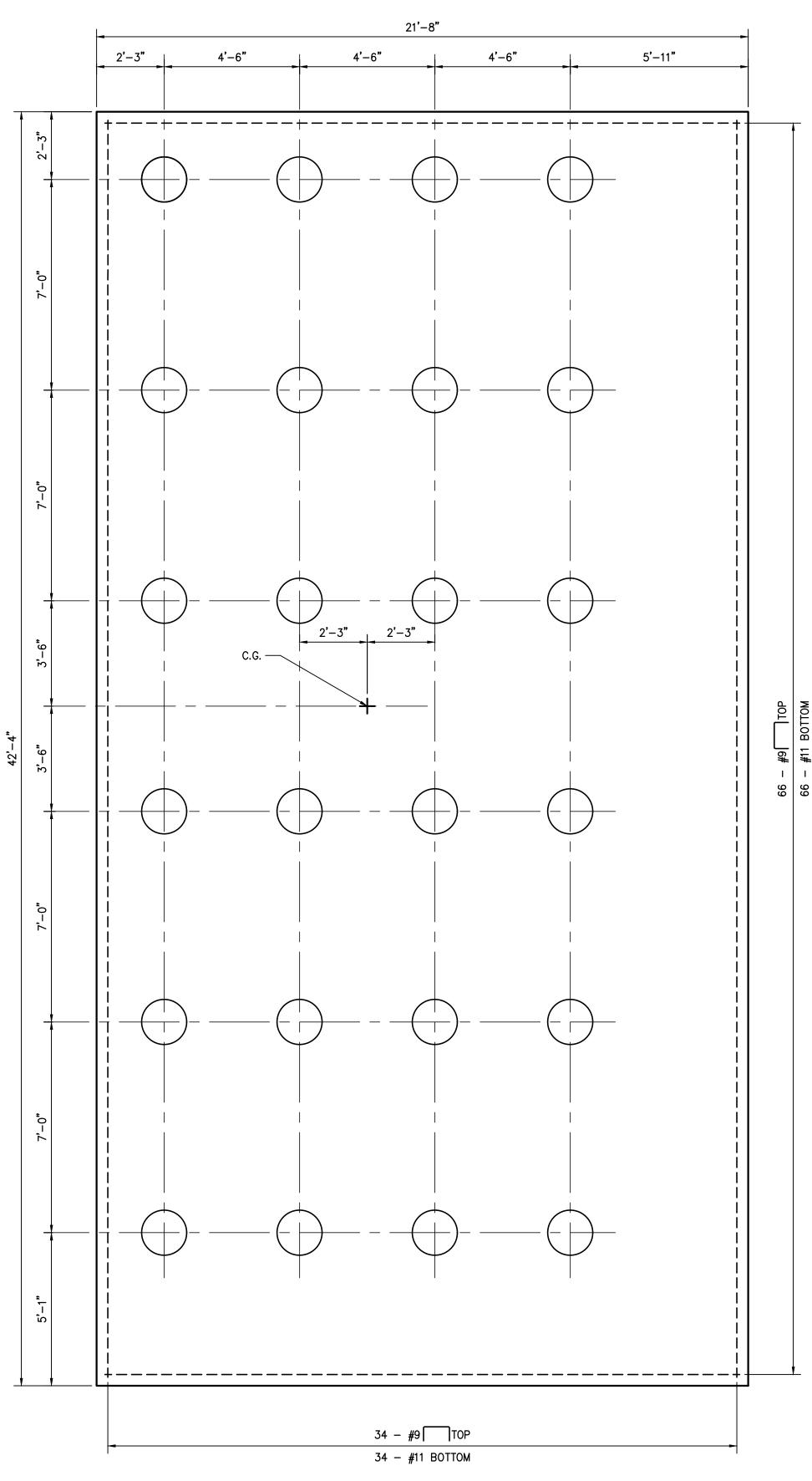
S1.8

PHASE: CD

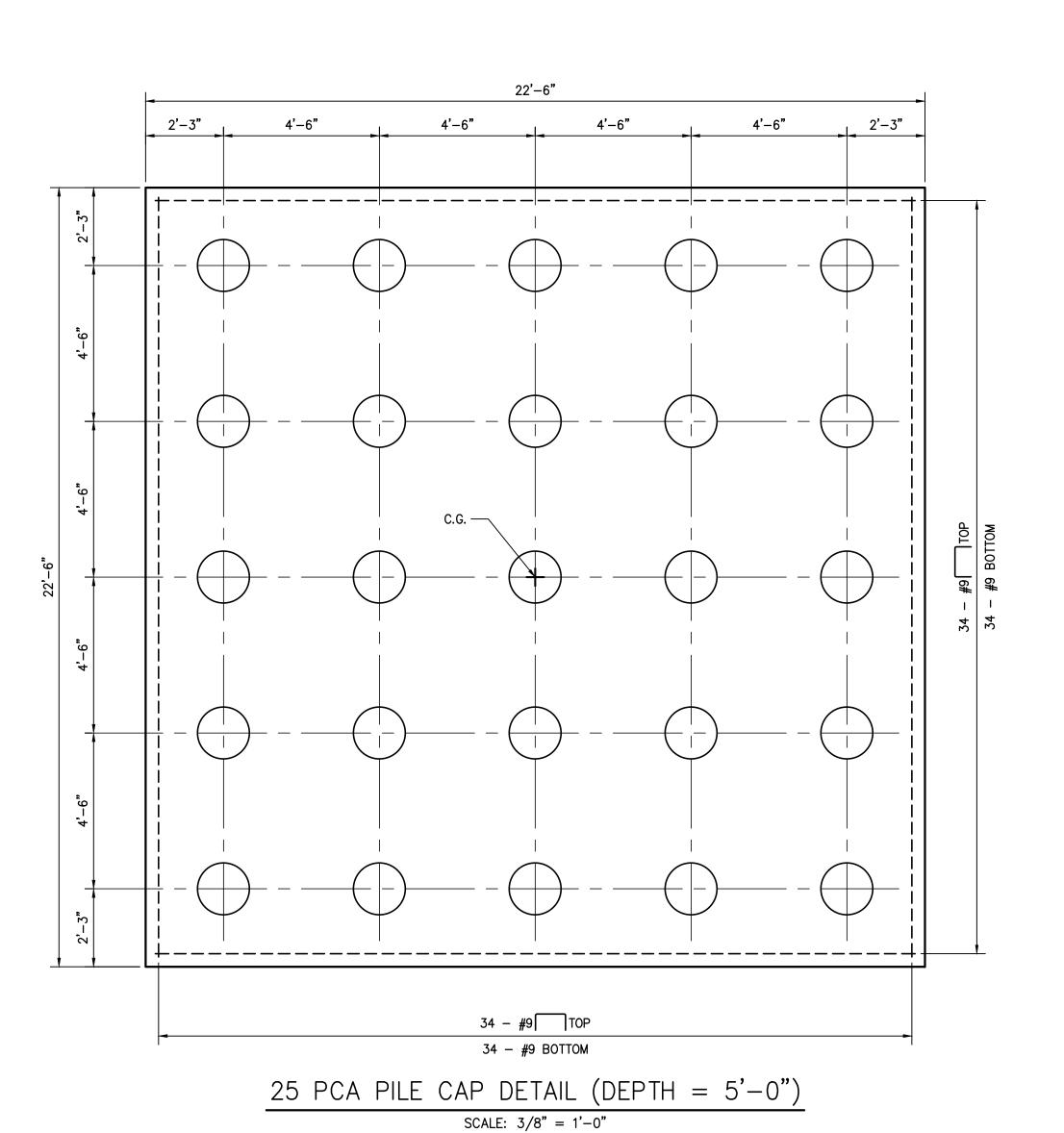
CHECKER: JBH

SCALE: AS NOTED

ISSUED: 04/20/2018



24 PCB PILE CAP DETAIL (DEPTH = 4'-6") SCALE: 3/8" = 1'-0"



PLAN NOTES: 1. FOR ALL NOTES, SEE DRAWING S1.0A-S1.0B. COMPRESSION PILE



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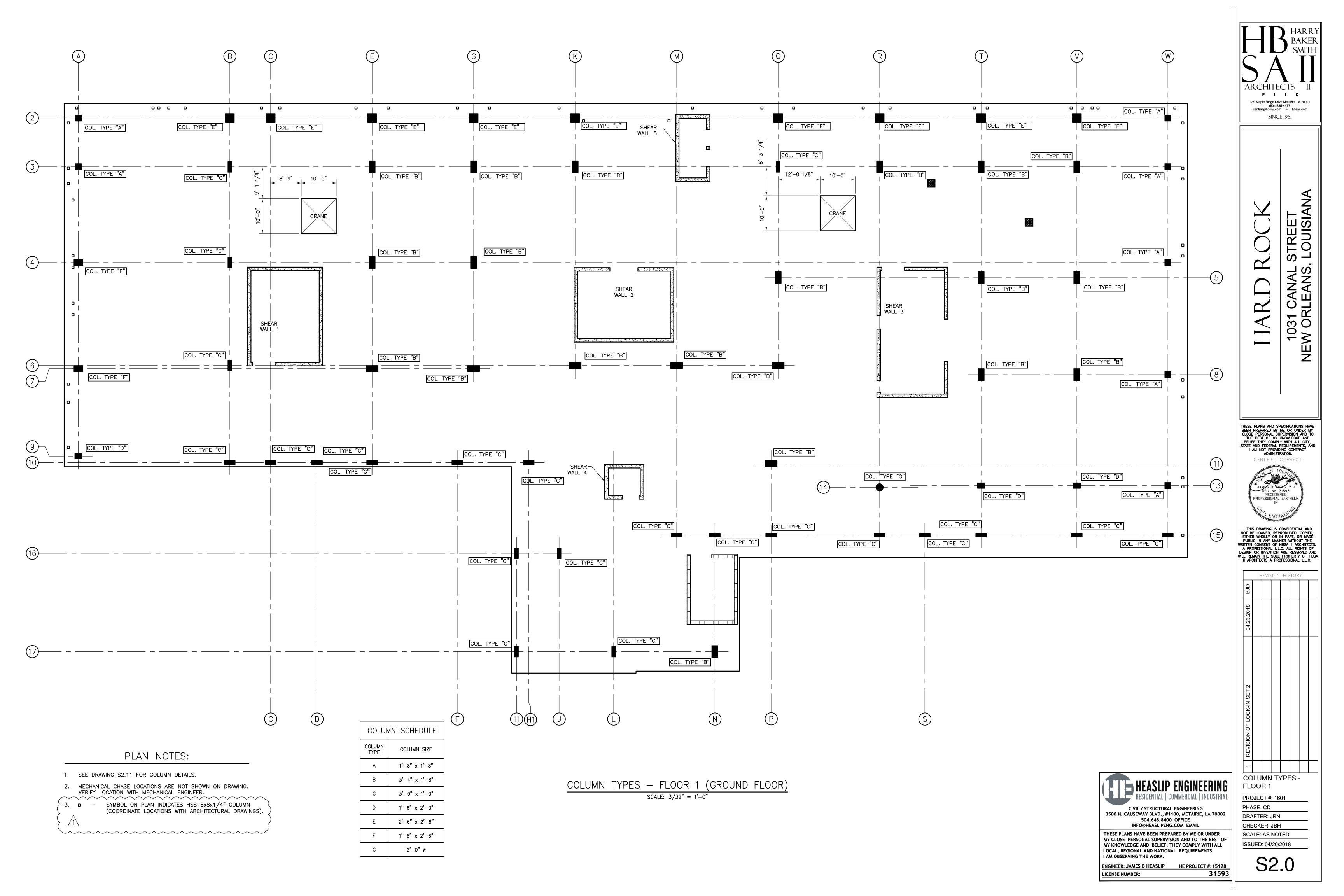
R	EVIS	SION	HIS	TOR	Υ	

PILE CAP DETAILS

PROJECT #: 1601 PHASE: CD

DRAFTER: JRN CHECKER: JBH SCALE: AS NOTED

ISSUED: 04/20/2018 S1.9



1.

SYMBOL ON PLAN INDICATES HSS 8x8x1/4" COLUMN (COORDINATE LOCATIONS WITH ARCHITECTURAL DRAWINGS).

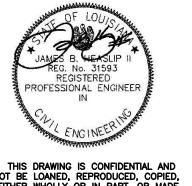
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BAKER
SMITH
SMITH
SALI
ARCHITECTS II
P L C

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BJD	ara						
04.23.2018	04.30.2018						
REVISION OF LOCK-IN SET 2	GENERAL REVISIONS						
۔ 1S	2						
1S	ΤL	EVI	ĒL				

1ST LEVEL
STOREFRONT
FRAMING PLAN
PROJECT #: 1601

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

31593

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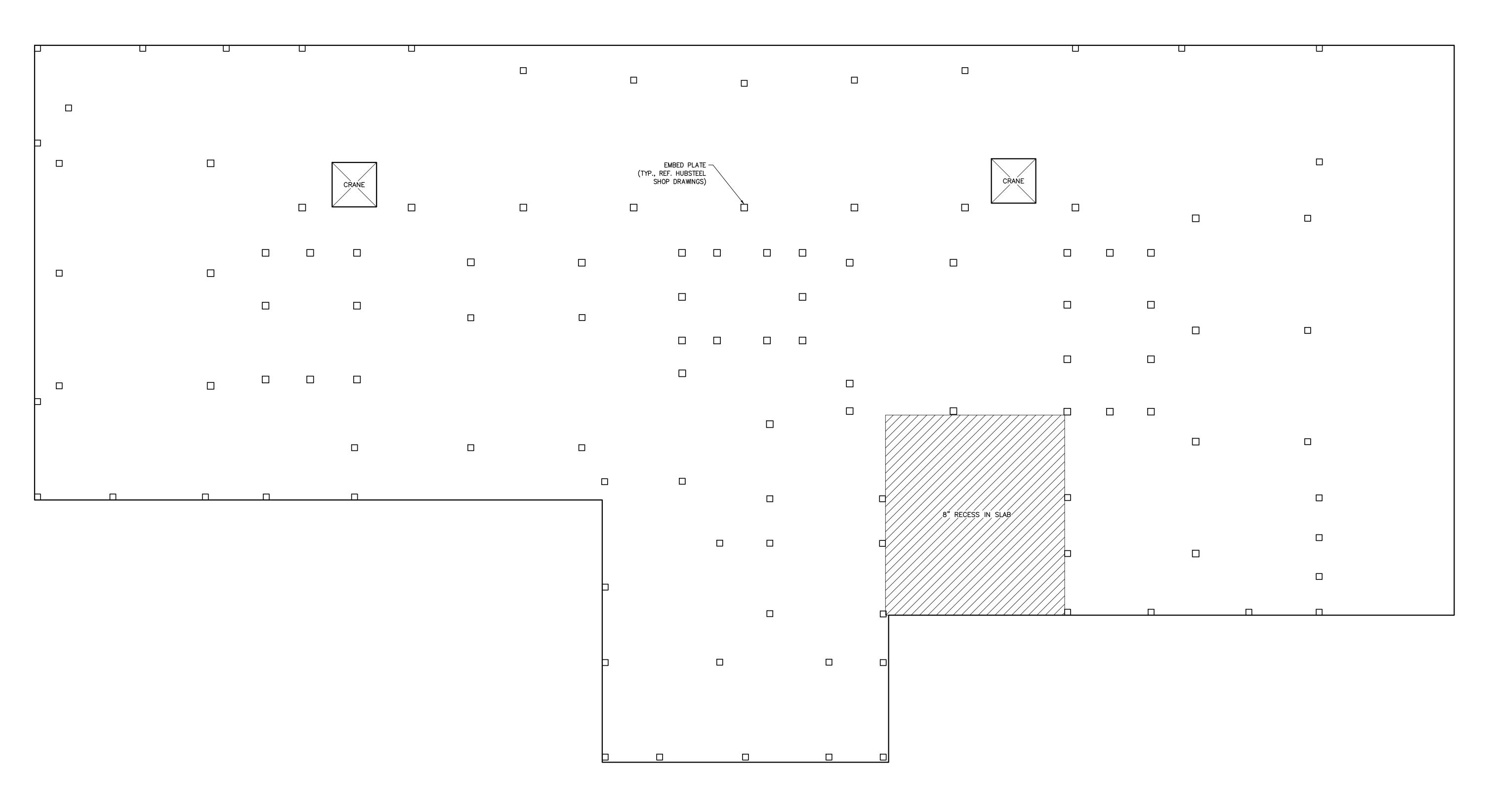
PHASE: CD

DRAFTER: JRN

CHECKER: JBH

CHECKER: JBH
SCALE: AS NOTED
ISSUED: 04/20/2018

S2.1



8TH FLOOR PLAN — HOTEL

SCALE: 3/32" = 1'-0"

PLAN NOTES:

- 1. 36" POST TENSION CONCRETE TRANSFER SLAB (BY OTHERS, TYP. U.N.O.)
- 2. MECHANICAL CHASE LOCATIONS ARE NOT SHOWN ON DRAWING. VERIFY LOCATION WITH MECHANICAL ENGINEER.

MULTIPLE REVISIONS MADE TO THIS DRAWING. REVIEW ENTIRE DRAWING FOR REVISIONS.



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8TH FLOOR PLAN -HOTEL 3500 N. CAUSEWAY BLVD., #1100, METAIRIE, LA 70002

ISSUED: 04/20/2018

PROJECT #: 1601

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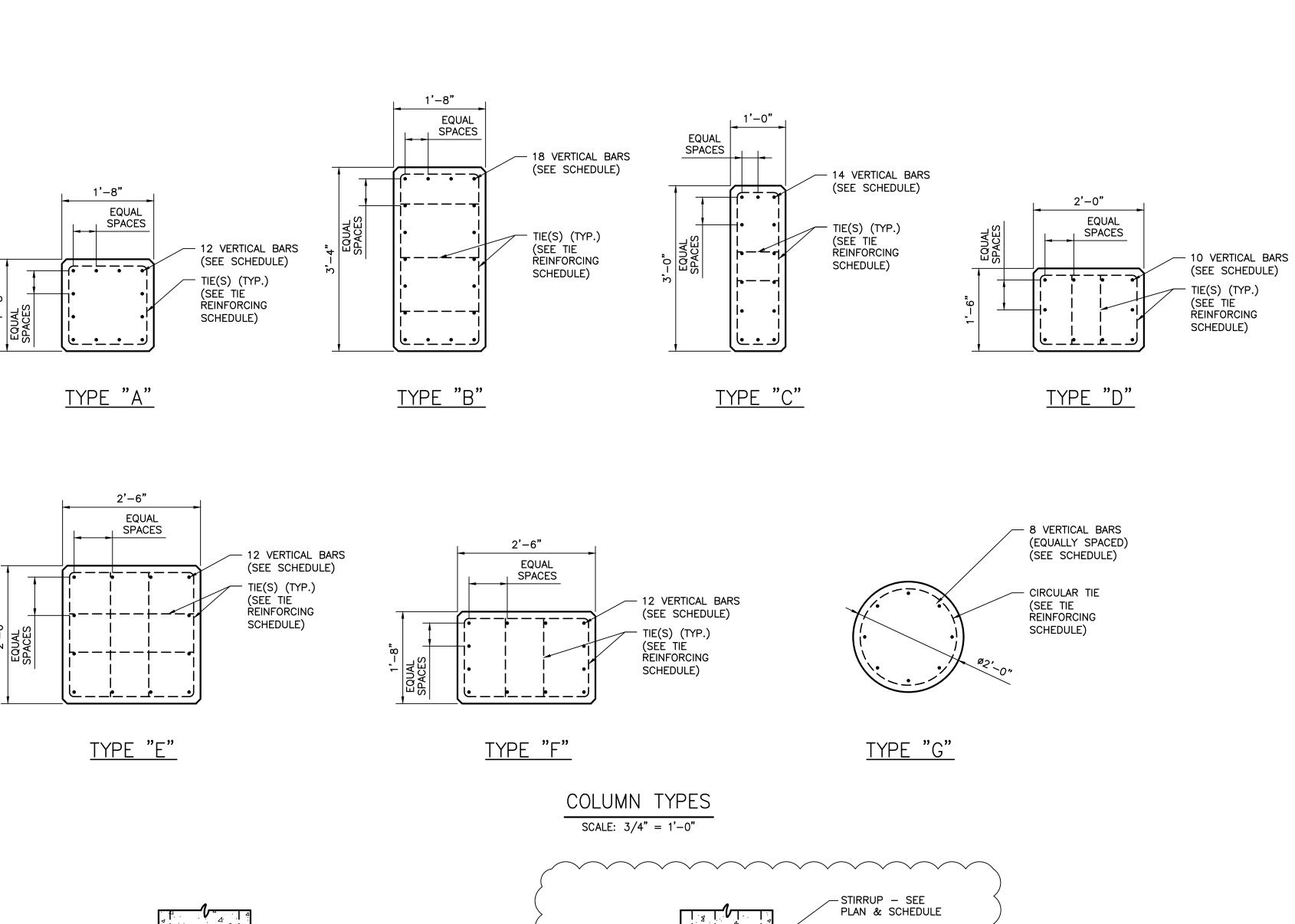
BJD							
05.17.2018							
REVISION OF LOCK-IN SET 2							
_							
	1 REVISION OF LOCK-IN SET 2 05.17.2018 BJD	REVISION OF LOCK-IN SET 2					

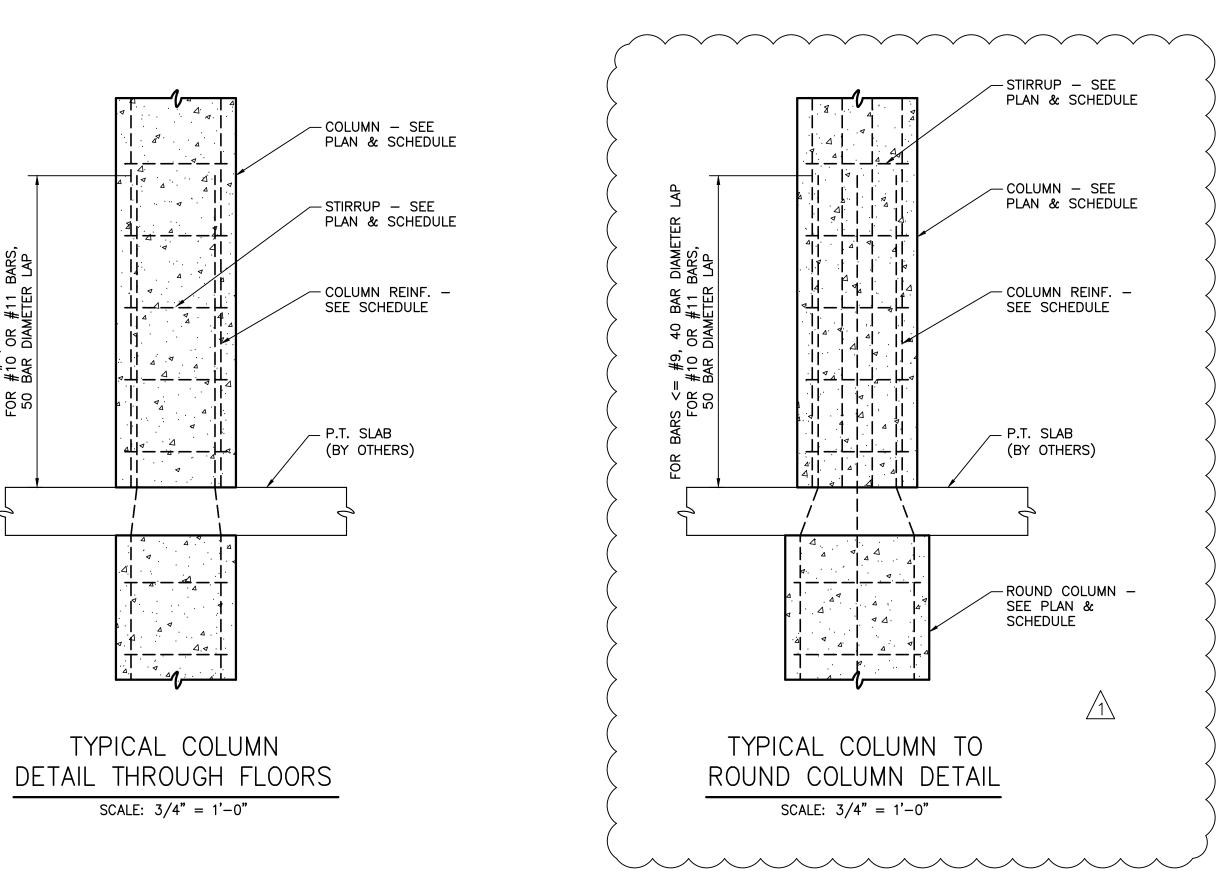
CON	CONCRETE COLUMN SCHEDULE										
	TYPE	Α	В	С	D	E	F	G			
EL. 81'-10" 8TH FLOOR	VERTICAL BARS	#6	#6	#6	#6	#6	#6	N/A			
EL. 69'-10" 7TH FLOOR	VERTICAL BARS	#6	#6	#6	#6	#6	#6	N/A			
EL. 61'-10" 6TH FLOOR	VERTICAL BARS	#6	#6	#6	#6	#7	#7	N/A			
EL. 53'-10" 5TH FLOOR	VERTICAL BARS	#6	#8	#6	#6	#8	#8	N/A			
EL. 45'-10" 4TH FLOOR	VERTICAL BARS	#7	#9	#8	#7	#9	#9	N/A			
EL. 37'-0" 3RD FLOOR	VERTICAL BARS	#7	#9	#8	#7	#9	#9	N/A			
EL. 27'-10" 2ND FLOOR MEZZANINE	VERTICAL BARS	# 7	#9	#8	#7	#9	#9	N/A			
EL. 19'-0" 2ND FLOOR RETAIL	VERTICAL BARS	#8	#9	#8	#7	#9	#9	N/A			
EL. 10'-0" INTERMEDIATE PARKING	VERTICAL BARS	#9	#10	#9	#8	#10	#10	#10			
EL. 0'-0" GROUND FLOOR	VERTICAL BARS	#9	#11	#9	#8	#10	#10	#11			
DOWELS	VERTICAL BARS	#9	#11	#9	#8	#10	#10	#11			

SCHEDULE NOTES:

- 1. SEE TYPICAL PILE CAP COLUMN DETAILS ON DRAWINGS S1.3 S1.9 AND S1.13 S1.15.
- 2. CONTRACTOR SHALL PROVIDE 1 1/2" CONCRETE PROTECTIVE COVER FOR REINFORCEMENT (UNLESS NOTED OTHERWISE).

TIE REIN	FORCING	G SCHEDULE
VERTICAL BAR SIZE	TIE BAR SIZE	TIE BAR SPACING (IN.)
#3	#3	6
#4	#3	8
# 5	#3	10
#6	#3	12
#7	#3	12
#8	#3	12
#9	#3	12
#10	#3	12
#11	#4	12
#14	#4	18
#18	#4	18





RS <= #9, 40 BAR DIAME FOR #10 OR #11 BARS, 50 BAR DIAMETER LAP



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SCALE: AS NOTED ISSUED: 04/20/2018

COLUMN SCHEDULE

& DETAILS

PHASE: CD

DRAFTER: JRN

CHECKER: JBH

PROJECT #: 1601

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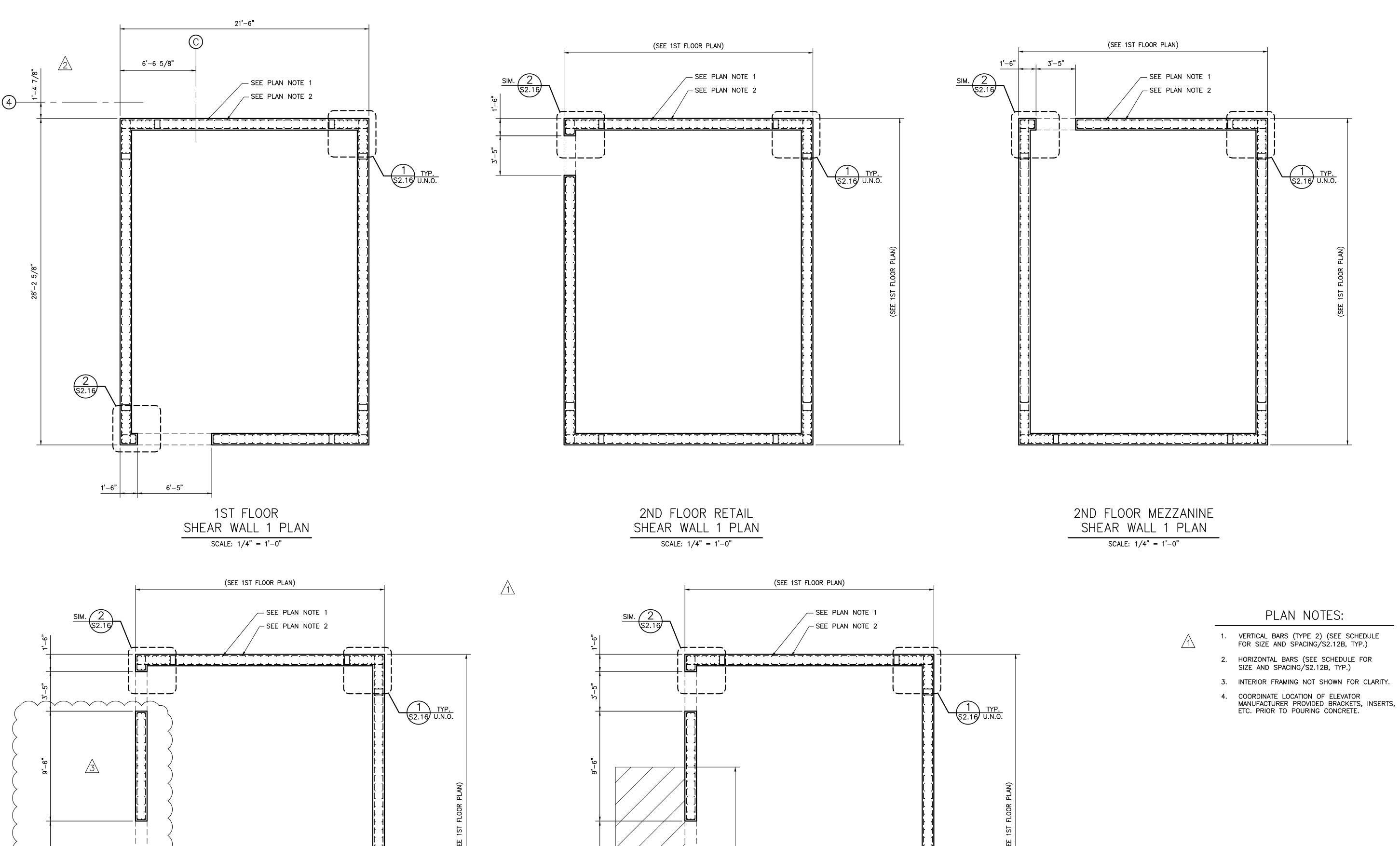
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BLOCK-OUT P.T. SLAB -AND INFILL WITH CAST IN

PLACE CONCRETE.

6'-0"

3RD & 4TH FLOOR SHEAR WALL 1 PLAN SCALE: 1/4" = 1'-0"

5th - 7TH FLOOR SHEAR WALL 1 PLAN SCALE: 1/4" = 1'-0"

- 3. INTERIOR FRAMING NOT SHOWN FOR CLARITY.

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593

SHEAR WALL 1 **PLANS** PROJECT #: 1601 PHASE: CD

DRAFTER: JRN CHECKER: JBH SCALE: AS NOTED

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STREET, LOUISIANA

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REG. No. 31593
REGISTERED
PROFESSIONAL ENGINEER
IN

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ISSUED: 04/20/2018

			SHEAR WA	ALL 1 SCHE	DULE				
	TYPE	VERTICAL BAR	HORIZONTAL BAR	OPENING T &	B BAR	OPENING S	TIRRUP BAR	OPENING H	IORIZ. BAR
		SIZE & SPACING	SIZE & SPACING	SIZE & SP/	ACING	SIZE & S	SPACING	SIZE & SPACING	
EL. 81'-10"	TYPE 2	#8 BARS @ 6" PER FACE	A	A					
8TH FLOOR	TYPE 1	#9 BARS @ 6"							
EL. 69'-10"	TYPE 2	#8 BARS @ 6" PER FACE							
7TH FLOOR	TYPE 1	#9 BARS @ 6"							
EL. 61'-10"	TYPE 2	#8 BARS @ 6" PER FACE							
6TH FLOOR	TYPE 1	#10 BARS @ 6"							
EL. 53'-10"	TYPE 2	#8 BARS @ 6" PER FACE							
5TH FLOOR	TYPE 1	#10 BARS @ 6"							
EL. 45'-10"	TYPE 2	#8 BARS @ 6" PER FACE	#F DADO @ 40" DED FACE						
4TH FLOOR	TYPE 1	#10 BARS @ 6"	- #5 BARS @ 12" PER FACE						
EL. 37'-0"	TYPE 2	#9 BARS @ 6" PER FACE	A						
3RD FLOOR	TYPE 1	#10 BARS @ 6"							
EL. 27'-10"	TYPE 2	#9 BARS @ 6" PER FACE							
2ND FLOOR MEZZANINE	TYPE 1	#10 BARS @ 6"							
EL. 19'-0"	TYPE 2	#9 BARS @ 6" PER FACE							
2ND FLOOR RETAIL	TYPE 1	#10 BARS @ 6"							
EL. 10'-0"	TYPE 2	#9 BARS @ 6" PER FACE	#6 DADS @ 40" DED 5405						
INTERMEDIATE PARKING	TYPE 1	#11 BARS @ 6"	- #6 BARS @ 12" PER FACE						
EL. 0'-0" GROUND	TYPE 2	#9 BARS @ 6" PER FACE	#C DADC @ 0" DED 5405	#0 DADO @ 4 4 /0)" DED 5105	#4 5.5		E DADO 0 1	
FLOOR	TYPE 1	#11 BARS @ 6"	- #6 BARS @ 8" PER FACE	#8 BARS @ 1 1/2	YER FACE	#4 BAR	RS @ 6"	HO RAK2 @ .	12" PER FACE
DOWE! C	TYPE 2	#9 BARS @ 6" PER FACE	N1 /A	h1 /4			/A	All	/A
DOWELS	TYPE 1	#11 BARS @ 6"	- N/A	N/A		N/A		N/A	

SCHEDULE NOTES:

- 1. #4 TIES @ 12" FOR #10 BARS AND #11 BARS #3 TIES @ 12" FOR #5 THRU #9 BARS.
- 2. SEE SECTION 13/S1.14 FOR DOWEL DETAILS.
- SEE SECTION 15/51.14 TOK DOWLE DETAILS.
 CONTRACTOR SHALL PROVIDE 1 1/2" CONCRETE PROTECTIVE COVER FOR REINFORCEMENT, UNLESS NOTED OTHERWISE.

HARRY
BAKER
SMITH
SMITH
SMITH
ARCHITECTS II
PLUG

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(504)885-4477
central@hbsaii.com :: hbsaii.com
SINCE 1961

1031 CANAL STREET
NEW ORLEANS, LOUISIAN

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REVISION HISTORY

BJD								
04.23.2018								
1 REV. OF LOCK-IN SET 2 (DRAWING ADDED)								
_								
SHEAR WALL 1								

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RESIDENTIAL | COMMERCIAL | INDUSTRIAL

CIVIL / STRUCTURAL ENGINEERING
3500 N. CAUSEWAY BLVD., #1100, METAIRIE, LA 70002

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128
LICENSE NUMBER: 31593

SCALE: AS NOTED
ISSUED: 04/20/2018

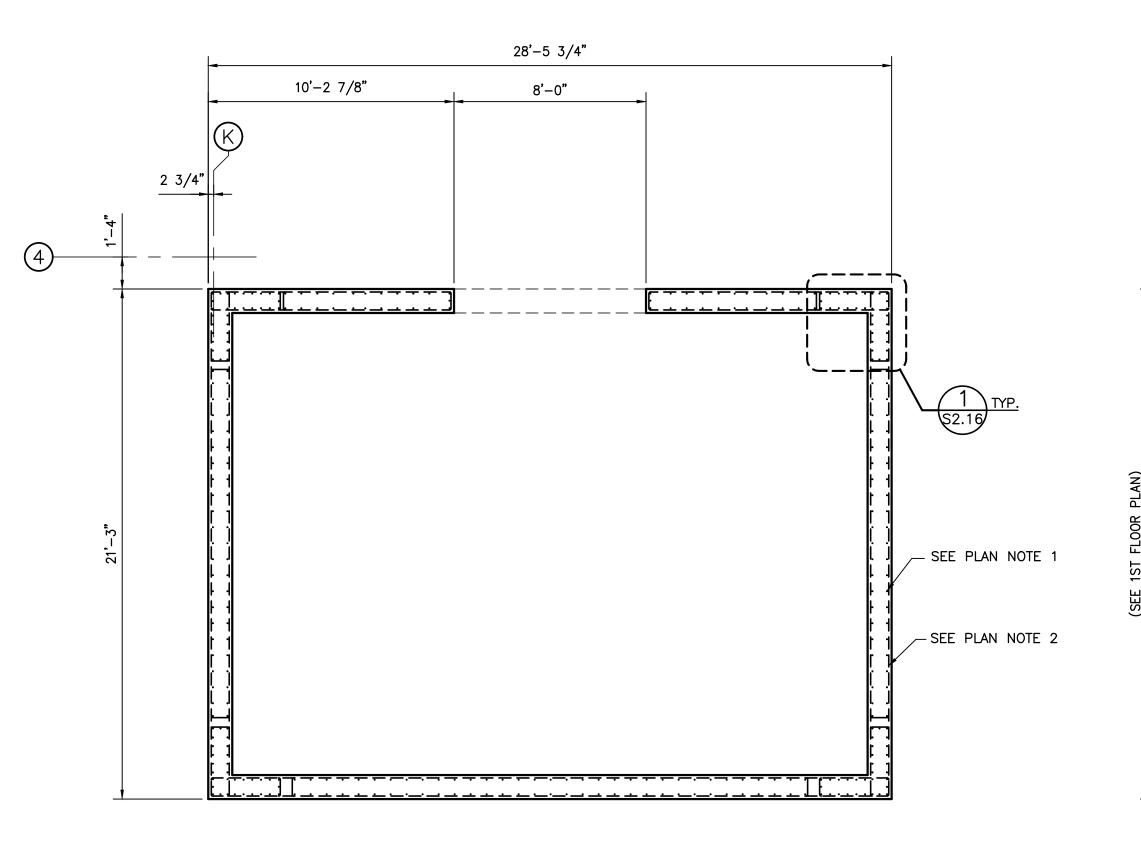
PLANS

PHASE: CD

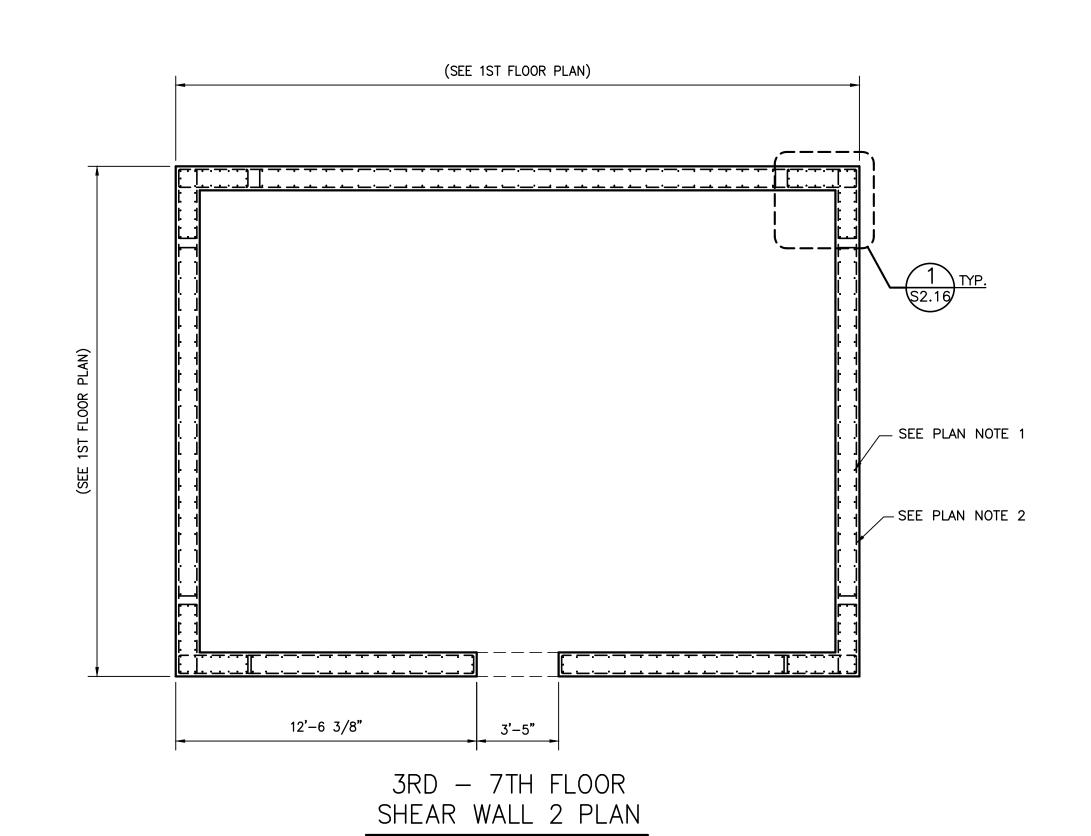
PROJECT #: 1601

DRAFTER: JRN

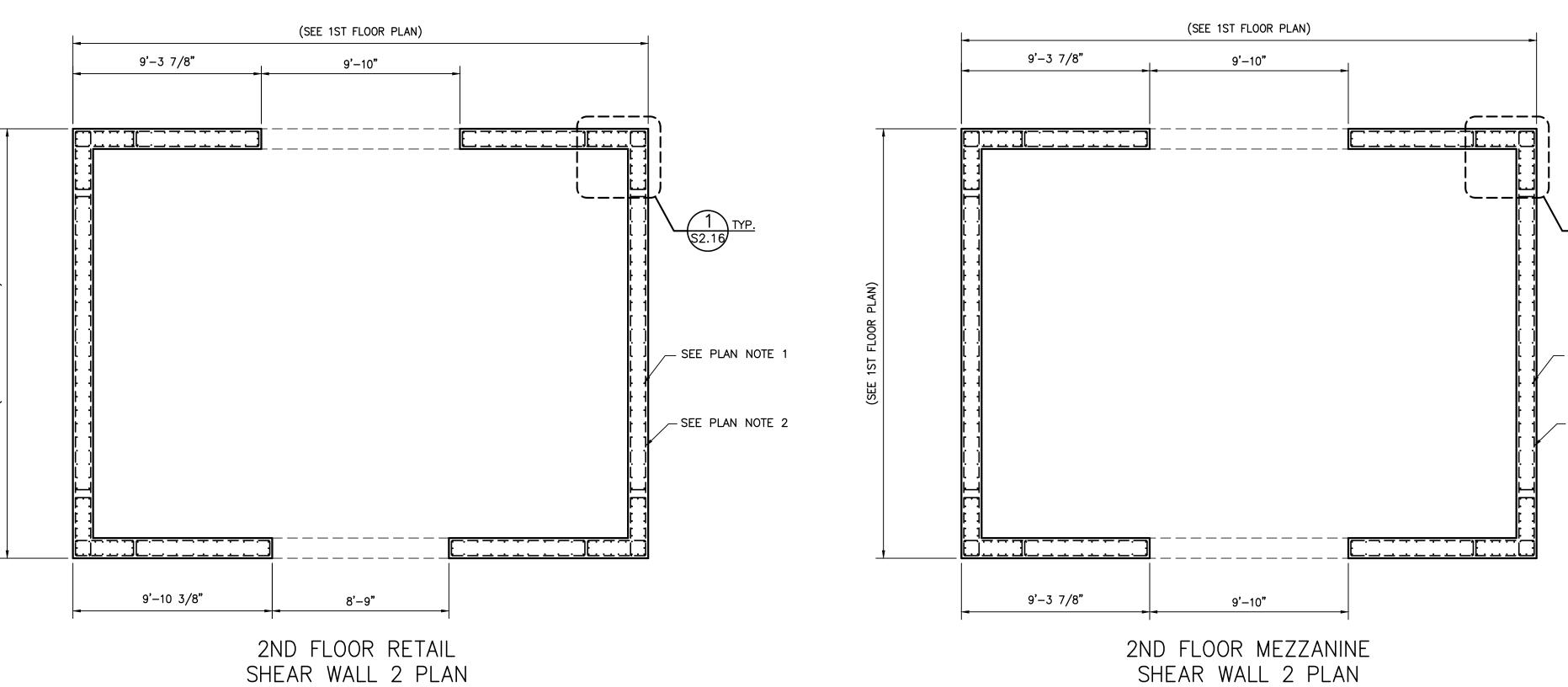
CHECKER: JBH



1ST FLOOR SHEAR WALL 2 PLAN SCALE: 1/4" = 1'-0"



SCALE: 1/4" = 1'-0"

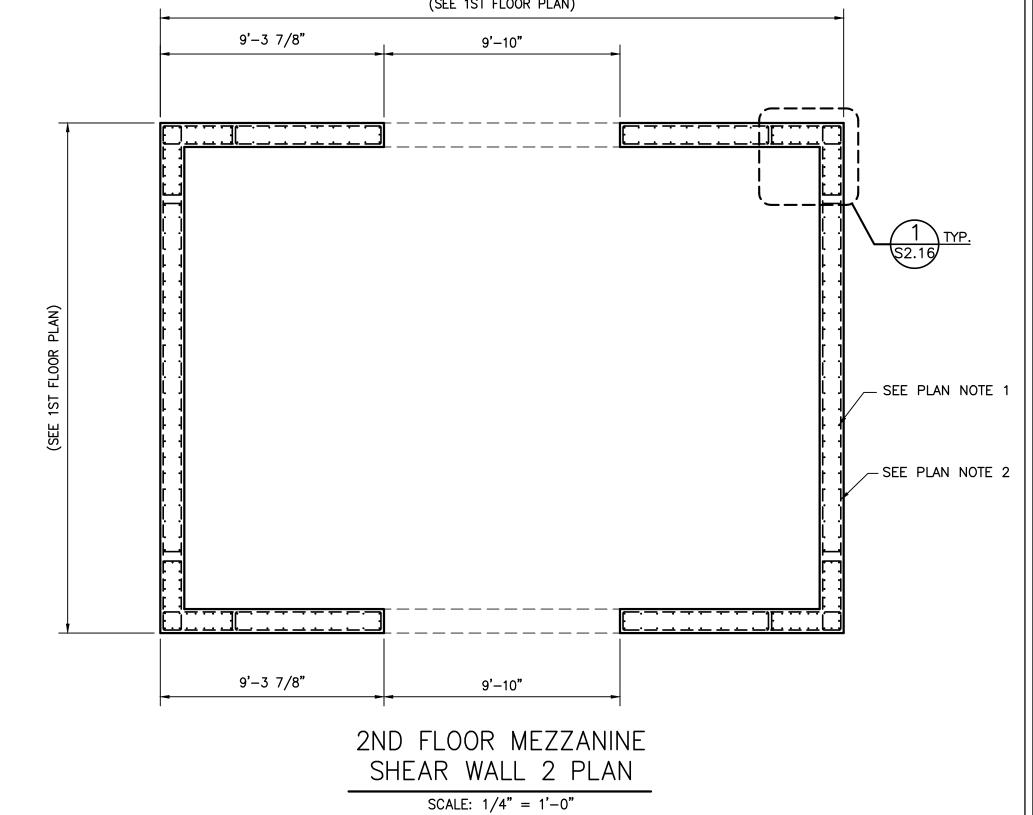


			SH	HEAR WA	ALL 2 SCH	IEDULE				
	TYPE	VERTICAL BAR	HORIZONTAL BAR SIZE & SPACING		OPENING T & B BAR SIZE & SPACING		OPENING STIRRUP BAR SIZE & SPACING		OPENING HORIZ. BAI	
		SIZE & SPACING								
EL. 81'-10"	TYPE 2	#8 BARS @ 6" PER FACE				A		†		†
8TH FLOOR	TYPE 1	#9 BARS @ 6"								
EL. 69'–10"	TYPE 2	#8 BARS @ 6" PER FACE								
7TH FLOOR	TYPE 1	#9 BARS @ 6"								
EL. 61'-10"	TYPE 2	#8 BARS @ 6" PER FACE								
6TH FLOOR	TYPE 1	#9 BARS @ 6"								
EL. 53'-10"	TYPE 2 #8 BARS @ 6" PER FACE		WE DADS @ 1	0" DED E40E						
5TH FLOOR	TYPE 1	#9 BARS @ 6"	#5 BARS @ 12" PER FACE							
EL. 45'–10"	TYPE 2	#8 BARS @ 6" PER FACE								
4TH FLOOR	TYPE 1	#9 BARS @ 6"								
EL. 37'-0"	TYPE 2	#9 BARS @ 6" PER FACE								
3RD FLOOR	TYPE 1	#10 BARS @ 6"								
EL. 27'-10"	TYPE 2	#9 BARS @ 6" PER FACE								
2ND FLOOR MEZZANINE	TYPE 1	#10 BARS @ 6"								
EL. 19'-0"	TYPE 2	#9 BARS @ 6" PER FACE								
2ND FLOOR RETAIL	TYPE 1	#10 BARS @ 6"								
EL. 10'-0"	TYPE 2	#9 BARS @ 6" PER FACE	#6 PAPS @ 1	12" PER FACE						
INTERMEDIATE PARKING	TYPE 1	#10 BARS @ 6"	#U DAKS W	IZ FER FAUE						
EL. 0'-0" GROUND	TYPE 2	#9 BARS @ 6" PER FACE	#6 DADO @ 3)" DED EASE	#8 BARS @ 1 1/2" PER FACE		#4 BARS @ 6"		#E DADO @	10" DED 5405
FLOOR	TYPE 1	#10 BARS @ 6"	- #6 BARS @ 8	D PER FACE					#5 BARS @	IZ PER FACE
DOWEL C	TYPE 2	#9 BARS @ 6" PER FACE		' A	_		1174			/4
DOWELS	TYPE 1	#10 BARS @ 6"	N/A		N/A		N/A		N,	/A

SCALE: 1/4" = 1'-0"

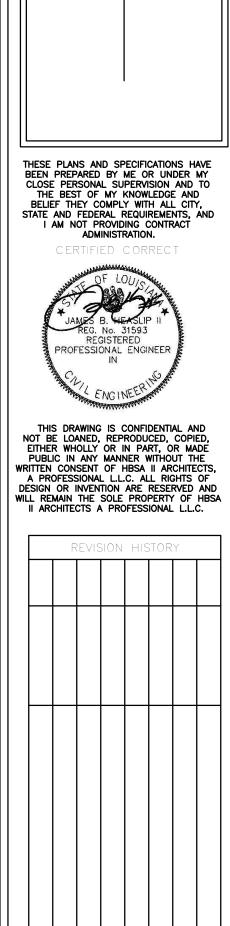
SCHEDULE NOTES:

- 1. #4 TIES @ 12" FOR #10 BARS AND #11 BARS #3 TIES @ 12" FOR #5 THRU #9 BARS.
- 2. SEE SECTION 13/S1.14 FOR DOWEL DETAILS. 3. CONTRACTOR SHALL PROVIDE 1 1/2" CONCRETE PROTECTIVE COVER FOR REINFORCEMENT, UNLESS NOTED OTHERWISE.



PLAN NOTES:

- 1. VERTICAL BARS (TYPE 2) (SEE SCHEDULE FOR SIZE AND SPACING, THIS DRAWING, TYP.)
- 2. HORIZONTAL BARS (SEE SCHEDULE FOR SIZE AND SPACING, THIS DRAWING, TYP.)
- 3. INTERIOR FRAMING NOT SHOWN FOR CLARITY.
- 4. COORDINATE LOCATION OF ELEVATOR MANUFACTURER PROVIDED BRACKETS, INSERTS, ETC. PRIOR TO POURING CONCRETE.



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SINCE 1961

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593

ISSUED: 04/20/2018

SHEAR WALL 2

PROJECT #: 1601

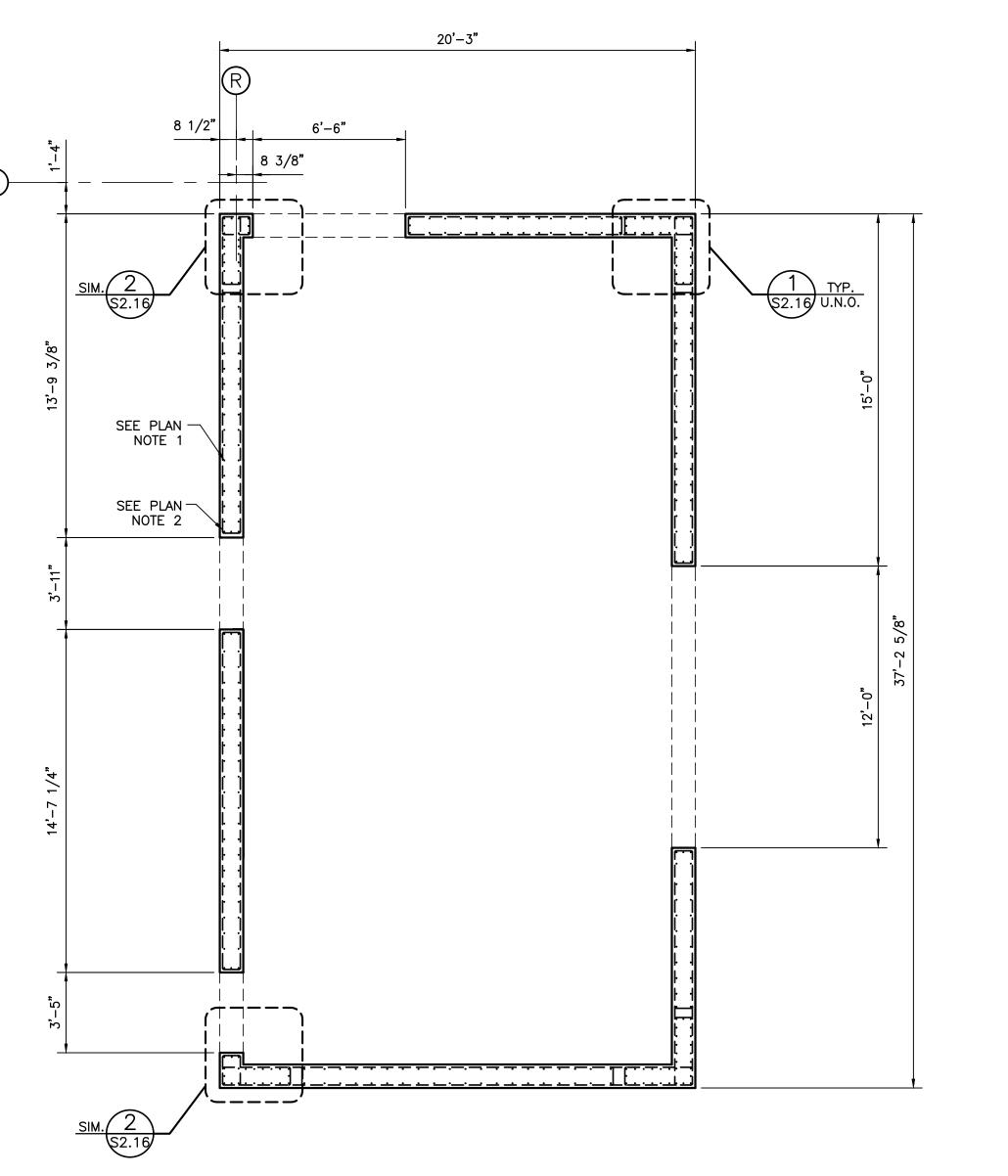
PLANS

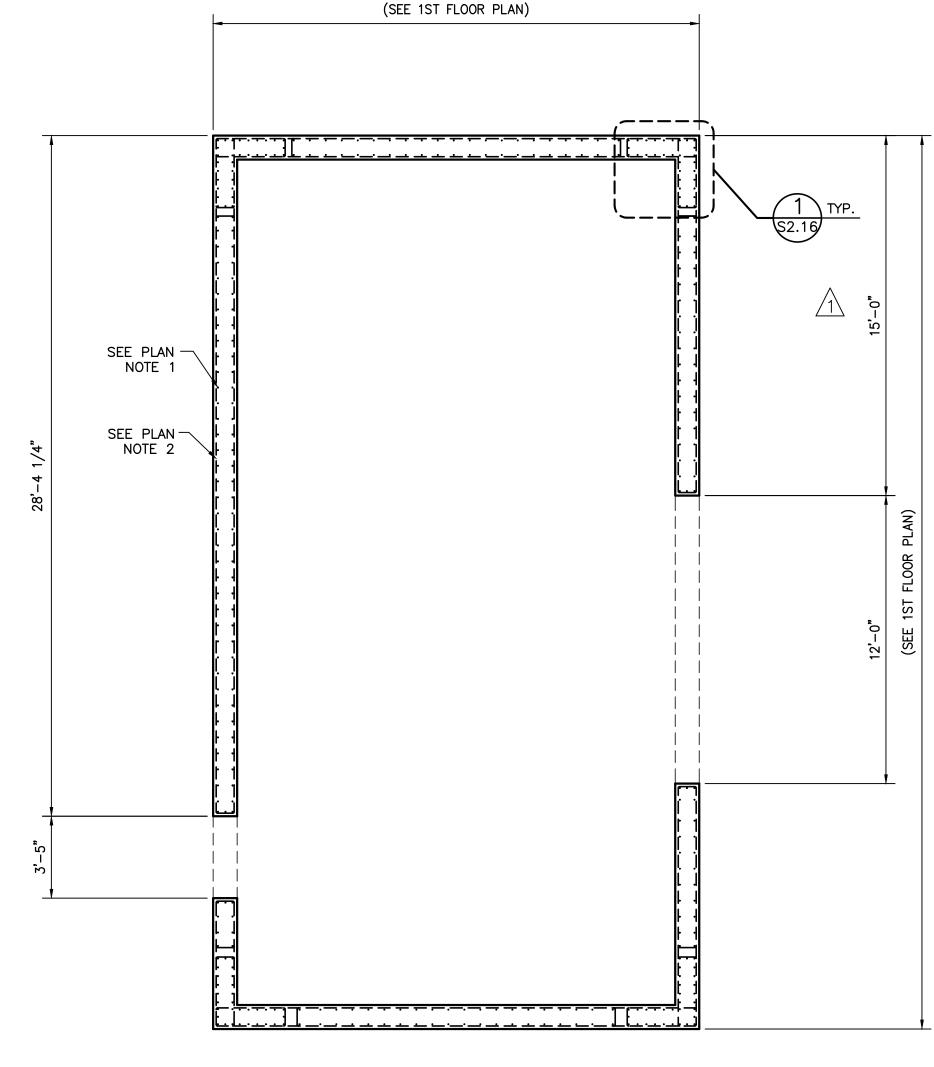
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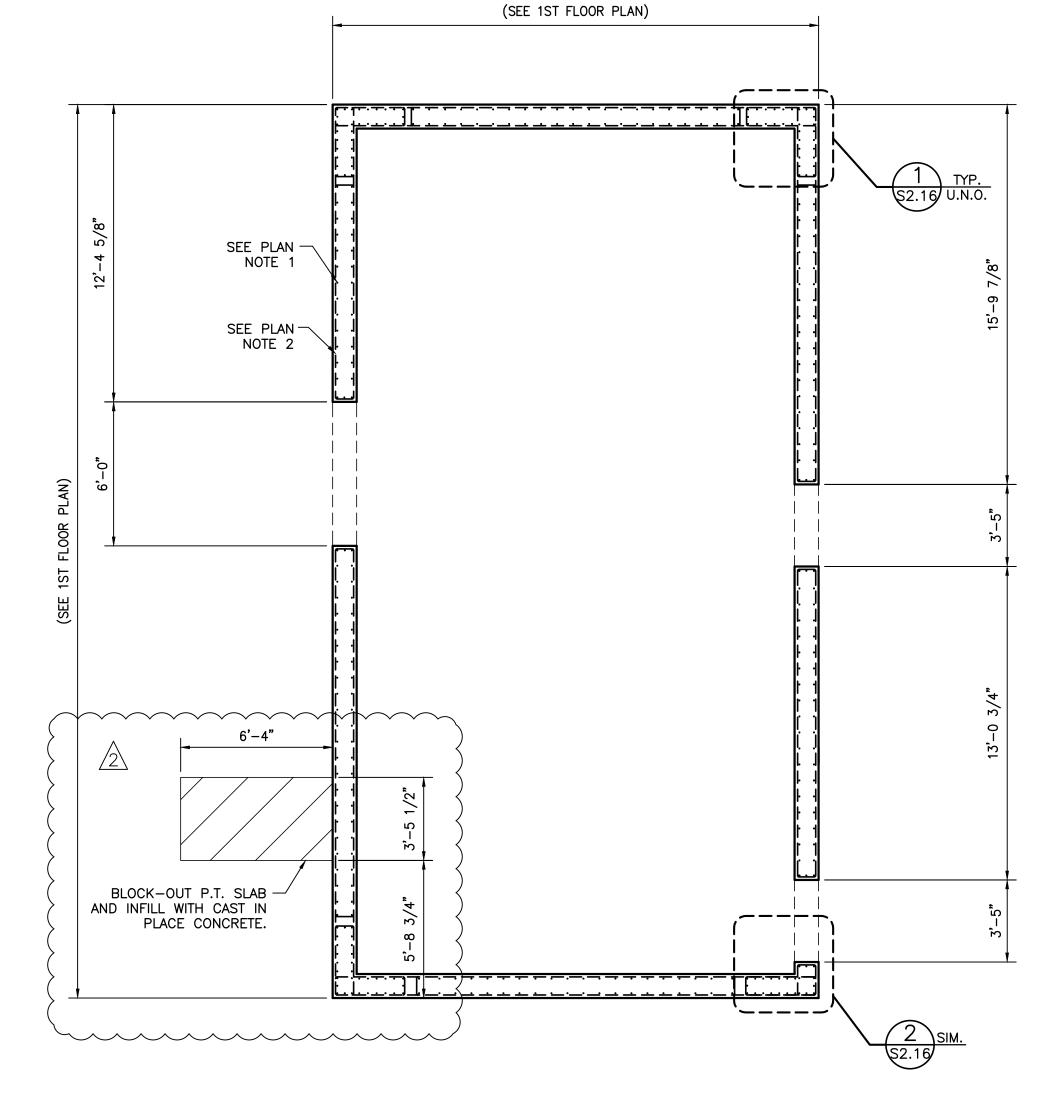
DRAFTER: JRN

CHECKER: JBH

SCALE: AS NOTED







2ND FLOOR RETAIL SHEAR WALL 3 PLAN SCALE: 1/4" = 1'-0"

1ST FLOOR SHEAR WALL 3 PLAN SCALE: 1/4" = 1'-0"

		SCALE:	1/4" = 1'-0"						
			SH	HEAR WA	ALL 3 SCHEDULE				
	TYPE	VERTICAL BAR	HORIZON	NTAL BAR	OPENING T & B BAR	OPENING STIRRUP BAR		OPENING HORIZ. BAF	
		SIZE & SPACING	SIZE &	SPACING	SIZE & SPACING	SIZE & S	SIZE & SPACING		SPACING
EL. 81'-10"	TYPE 2	#8 BARS @ 6" PER FACE	•	1	4			1	
8TH FLOOR	TYPE 1	#9 BARS @ 6"	1						
EL. 69'-10"	TYPE 2	#8 BARS @ 6" PER FACE							
7TH FLOOR	TYPE 1	#9 BARS @ 6"							
EL. 61'-10"	TYPE 2	#8 BARS @ 6" PER FACE							
6TH FLOOR	TYPE 1	#10 BARS @ 6"							
EL. 53'-10"	TYPE 2	#8 BARS @ 6" PER FACE	- #5 BARS @ 12" PER FACE						
5TH FLOOR	TYPE 1	#10 BARS @ 6"							
EL. 45'-10"	TYPE 2	#8 BARS @ 6" PER FACE							
4TH FLOOR	TYPE 1	#10 BARS @ 6"							
EL. 37'-0"	TYPE 2	#9 BARS @ 6" PER FACE							
3RD FLOOR	TYPE 1	#10 BARS @ 6"							
EL. 27'-10"	TYPE 2	#9 BARS @ 6" PER FACE							
2ND FLOOR MEZZANINE	TYPE 1	#10 BARS @ 6"							
EL. 19'-0"	TYPE 2	#9 BARS @ 6" PER FACE							
2ND FLOOR RETAIL	TYPE 1	#10 BARS @ 6"							
EL. 10'-0"	TYPE 2	#9 BARS @ 6" PER FACE	#6 DADS @ 1	o" DED EACE					
INTERMEDIATE PARKING	TYPE 1	#11 BARS @ 6"	- #6 BARS @ 12" PER FACE						
EL. 0'-0" GROUND	TYPE 2	#9 BARS @ 6" PER FACE	- #6 BARS @ 8" PER FACE		#0 DADC @ 1 1/0" DED FACE	#4 BARS @ 6"		#E DADC @	10" DED E40E
FLOOR	TYPE 1	#11 BARS @ 6"			#8 BARS @ 1 1/2" PER FACE			#5 BARS @ 12" PER FACE	
DOWEL C	TYPE 2	#9 BARS @ 6" PER FACE	A1./	A	NI /A				
DOWELS	TYPE 1	#11 BARS @ 6"	- N/A		N/A	N/A		N/A	
		-				i e			

INTERMEDIATE PARKING FLOOR SHEAR WALL 3 PLAN

SCALE: 1/4" = 1'-0"

PLAN NOTES:

- VERTICAL BARS (TYPE 2) (SEE SCHEDULE FOR SIZE AND SPACING, THIS DRAWING, TYP.)
- 2. HORIZONTAL BARS (SEE SCHEDULE FOR SIZE AND SPACING, THIS DRAWING, TYP.)
- 3. INTERIOR FRAMING NOT SHOWN FOR CLARITY.
- 4. COORDINATE LOCATION OF ELEVATOR MANUFACTURER PROVIDED BRACKETS, INSERTS, ETC. PRIOR TO POURING CONCRETE.

SCHEDULE NOTES:

- 1. #4 TIES @ 12" FOR #10 BARS AND #11 BARS #3 TIES @ 12" FOR #5 THRU #9 BARS.
- 2. SEE SECTION 13/S1.14 FOR DOWEL DETAILS. 3. CONTRACTOR SHALL PROVIDE 1 1/2" CONCRETE PROTECTIVE COVER FOR REINFORCEMENT, UNLESS NOTED OTHERWISE.



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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593 LICENSE NUMBER:

ISSUED: 04/20/2018

PLANS

PHASE: CD

DRAFTER: JRN

CHECKER: JBH

SCALE: AS NOTED

PROJECT #: 1601

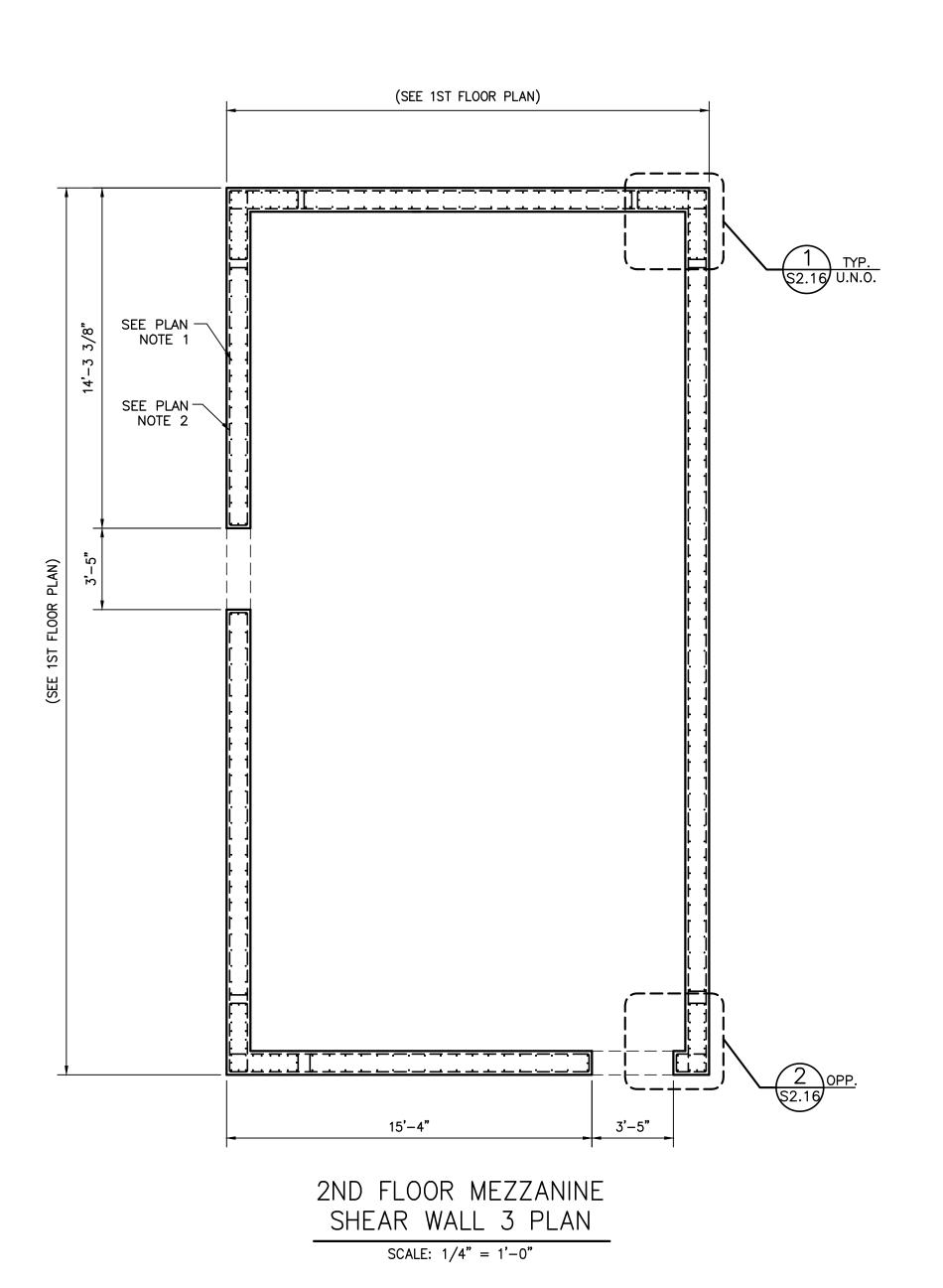
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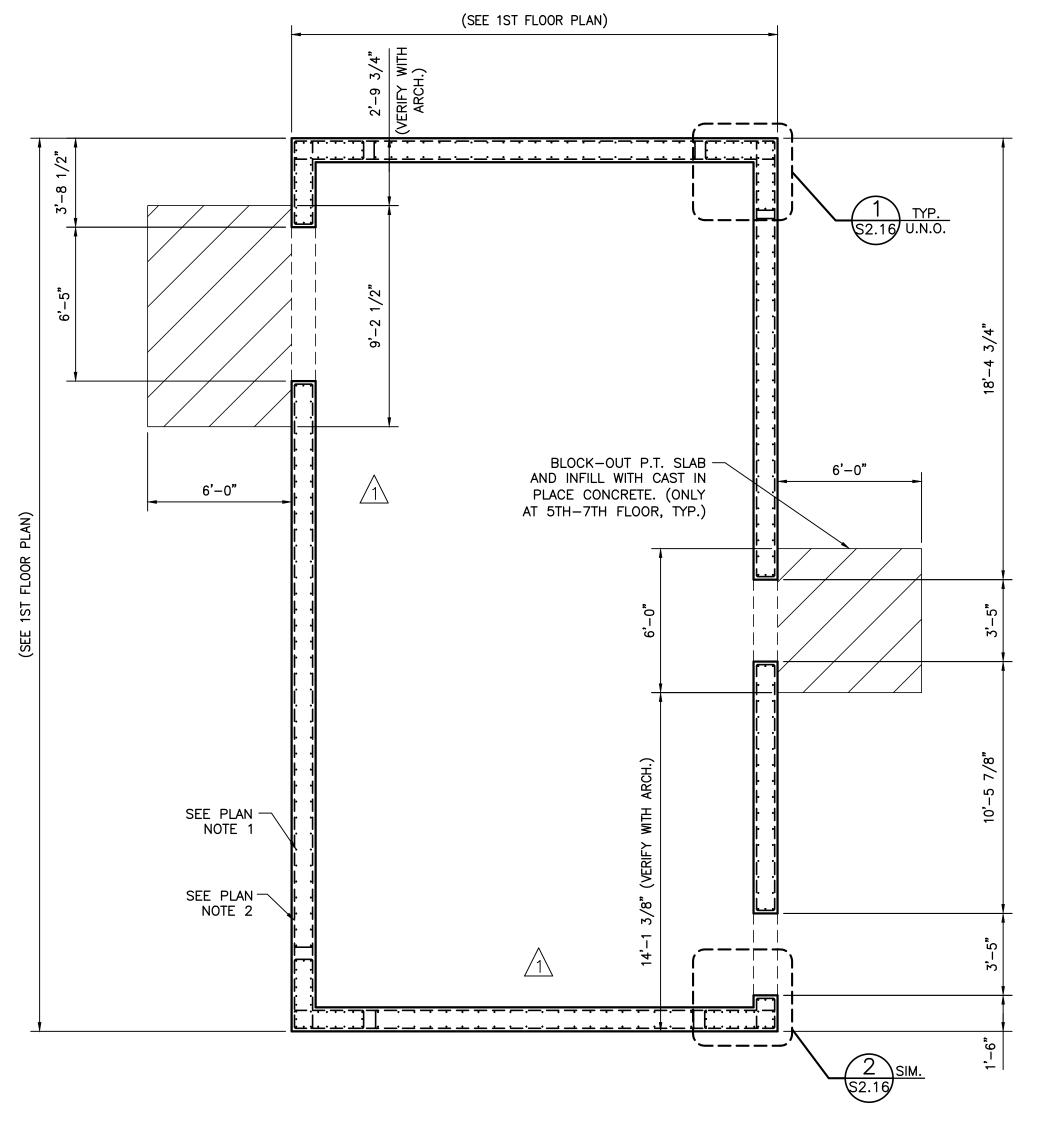
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	BJD	ara								
	04.23.2018	04.24.2018								
	REVISION OF LOCK-IN SET 2	REVISION OF LOCK-IN SET 2								
	1	2								
(SHEAR WALL 3									

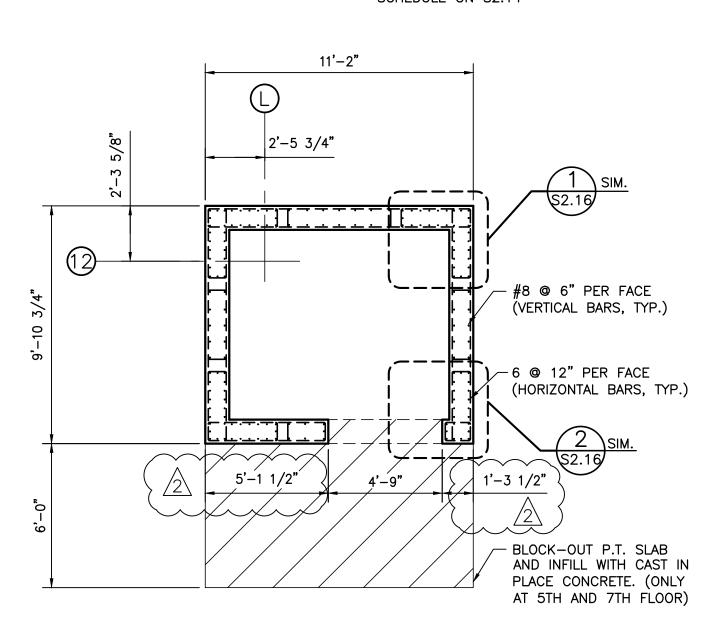




3RD - 7TH FLOOR SHEAR WALL 3 PLAN SCALE: 1/4" = 1'-0"

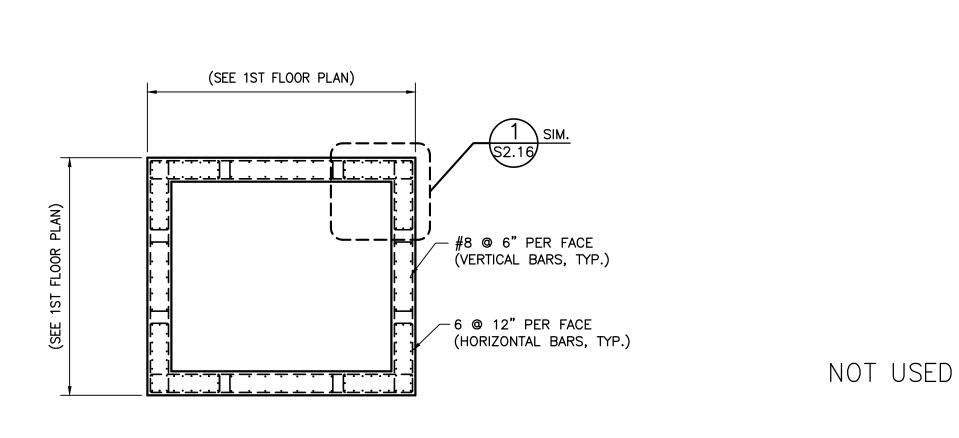


- 1. VERTICAL BARS (TYPE 2) (SEE SCHEDULE FOR SIZE AND SPACING/S2.14, TYP.)
- 2. HORIZONTAL BARS (SEE SCHEDULE FOR SIZE AND SPACING/S2.14, TYP.)
- INTERIOR FRAMING NOT CHOWN FOR CLARITY
- 3. INTERIOR FRAMING NOT SHOWN FOR CLARITY.
- 4. COORDINATE LOCATION OF ELEVATOR MANUFACTURER PROVIDED BRACKETS, INSERTS, ETC. PRIOR TO POURING CONCRETE.
- 5. FOR FLOOR ELEVATIONS, SEE SHEAR WALL SCHEDULE ON S2.14



1ST - 5TH & 7TH FLOOR
SHEAR WALL 4 PLAN

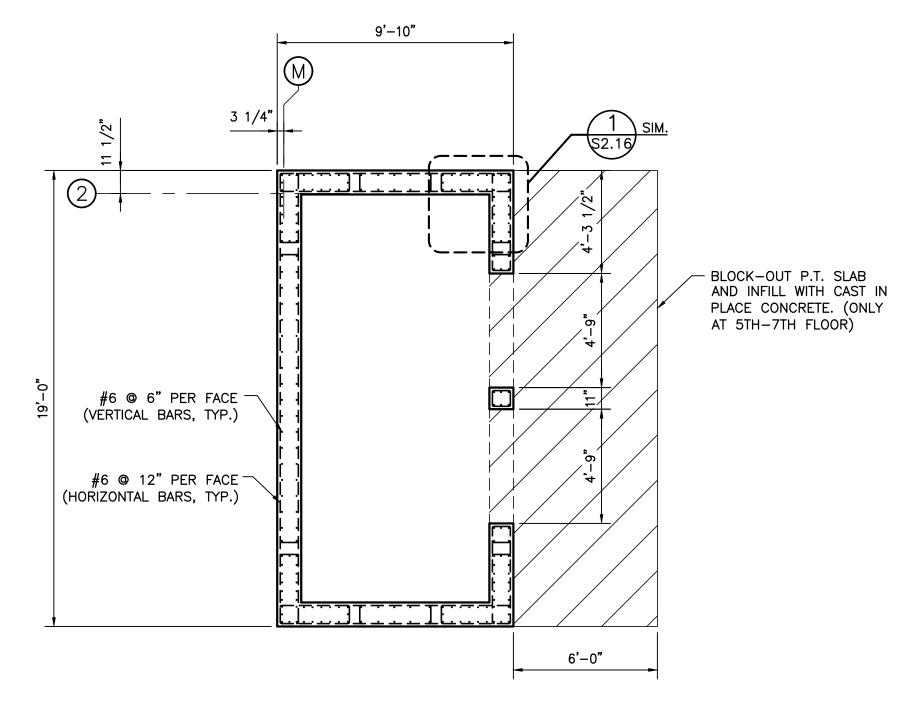
SCALE: 1/4" = 1'-0"



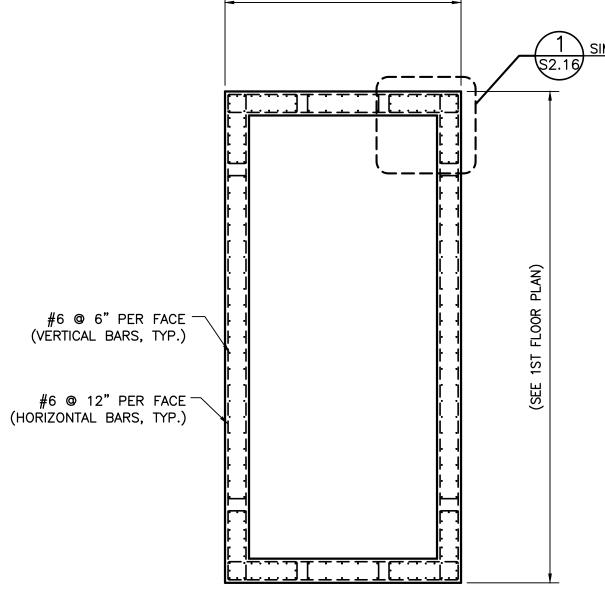
6TH FLOOR
SHEAR WALL 4 PLAN

SCALE: 1/4" = 1'-0"

1ST - 7TH FLOOR SHEAR WALL 5 PLAN SCALE: 1/4" = 1'-0"



1ST & 3RD - 7TH FLOOR SHEAR WALL 6 PLAN SCALE: 1/4" = 1'-0"



(SEE 1ST FLOOR PLAN)

2ND FLOOR & 2ND FLOOR MEZZANINE SHEAR WALL 6 PLAN

SCALE: 1/4" = 1'-0"

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128
LICENSE NUMBER: 31593

HEASLIP ENGINEERING

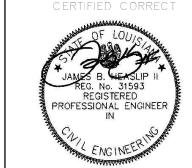
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REVISION HISTORY								
BJD	ara							
04.23.2018	04.30.2018							
REVISION OF LOCK-IN SET 2	GENERAL REVISIONS							
1	2							
SHEAR WALL 3, 4, & 6								

S2.15

PROJECT #: 1601

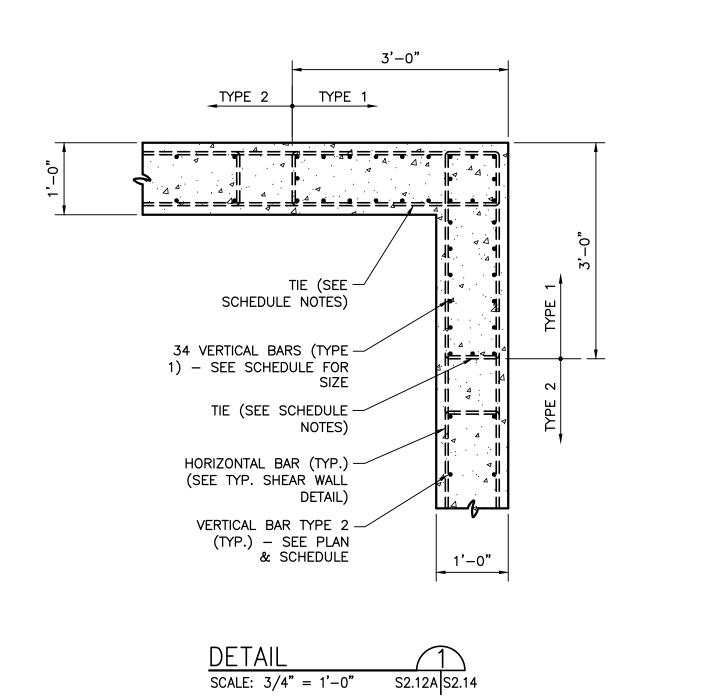
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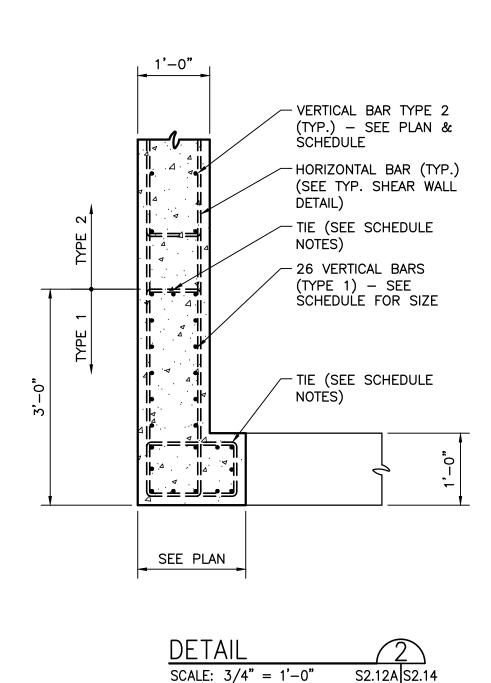
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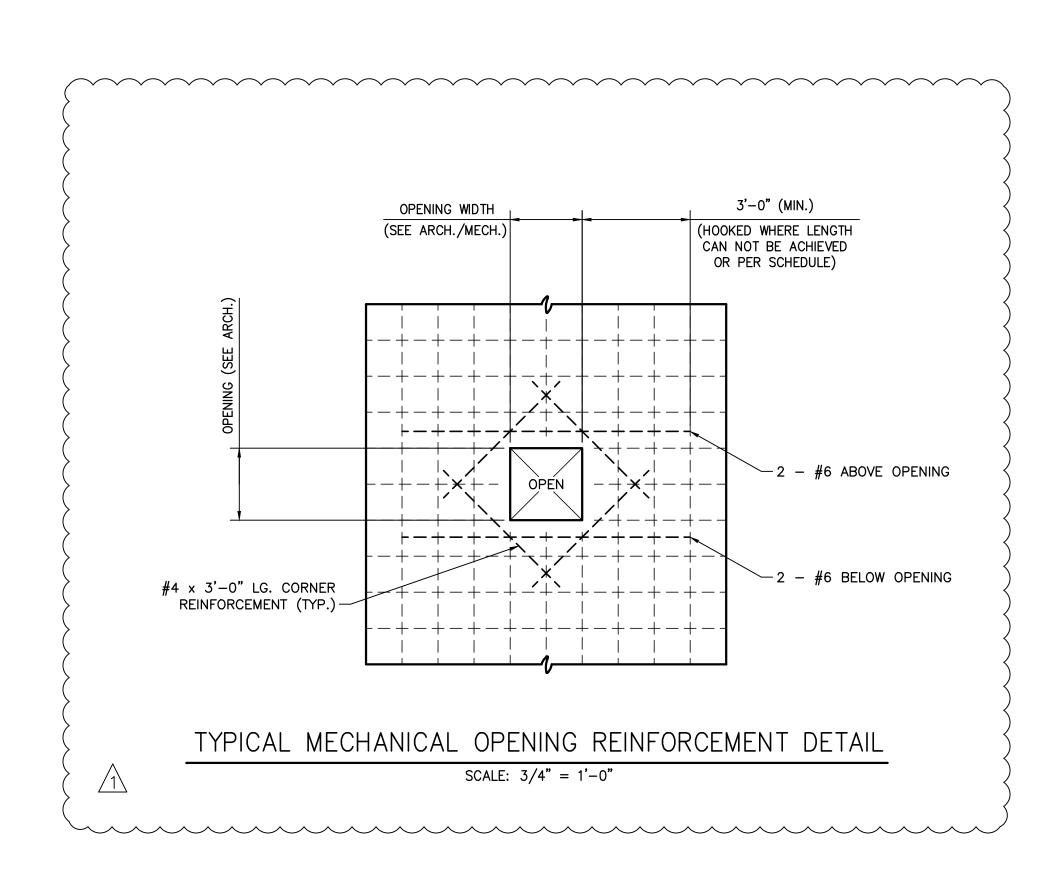
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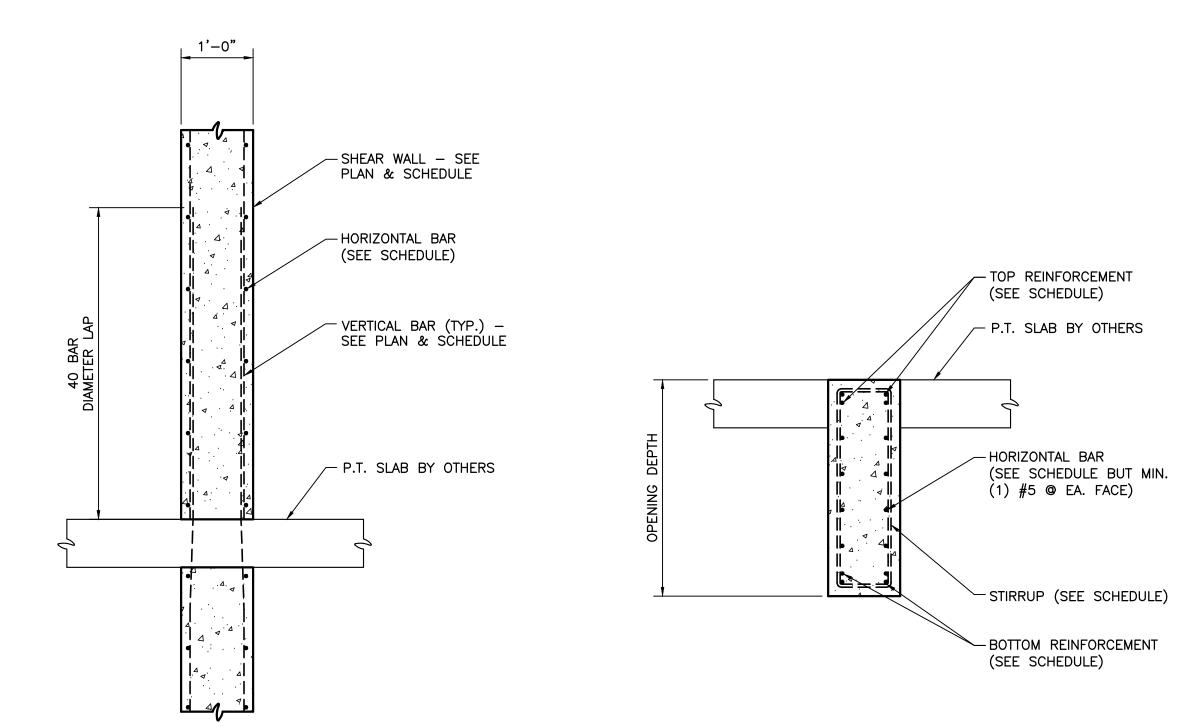
ISSUED: 04/20/2018

PHASE: CD









TYPICAL SHEAR WALL ELEVATION DETAIL

SCALE: 3/4" = 1'-0"

TYPICAL DOOR OPENING SECTION

SCALE: 3/4" = 1'-0"

4'-0" MIN. OPENING WIDTH (HOOKED WHERE LENGTH (SEE PLAN) CAN NOT BE ACHIEVED OR PER SCHEDULE) STIRRUPS (SEE SCHEDULE) -4 TOP & 4 BOT. REINFORCEMENT (SEE SCHEDULE) - HORIZONTAL REINFORCEMENT (SEE SCHEDULE) - VERTICAL REINFORCEMENT (SEE SCHEDULE) _+_+_+ SEE TYPICAL DOOR OPENING SECTION THIS DWG. _ _ + _ + _ + _ + _ + _ + _ + +-+-+-+-+- $-#5 \times 5'-0"$ LG. CORNER REINFORCEMENT (TYP.) . — + — + — + — + — + — + +-+-+-+-+-. — + — + — + — + — + — + +-+-+-+-+--+-+-+-+-+-+-+-++-+-+-+-SOLID" WALL BELOW

TYPICAL DOOR OPENING REINFORCEMENT DETAIL

SCALE: 3/4" = 1'-0"



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504.648.8400 OFFICE
INFO@HEASLIPENG.COM EMAIL

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

LICENSE NUMBER: 31593

LOCAL, REGIONAL AND NATIONAL REQUIREMENTS.

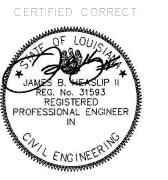
HE PROJECT #: 15128 31593

HARRY
BAKER
SMITH
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ARCHITECTS II
P L C

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BJD								
04.23.2018								
REVISION OF LOCK-IN SET 2								
_								
DETAILS								

PROJECT #: 1601
PHASE: CD
DRAFTER: JRN

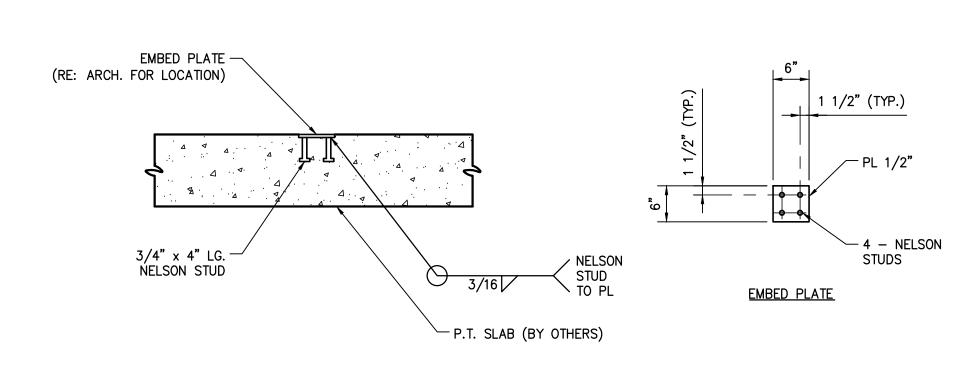
PHASE: CD
DRAFTER: JRN
CHECKER: JBH
SCALE: AS NOTED

ISSUED: 04/20/2018

3/4" x 4" LG. NELSON STUD ® 8" P.T. SLAB 3/4" x 6" LG. NELSON STUD ® 12" P.T. SLAB EMBED PLATE (RE: ARCH. FOR LOCATION) RESON STUD RELSON STUD O RELSON STUDS O

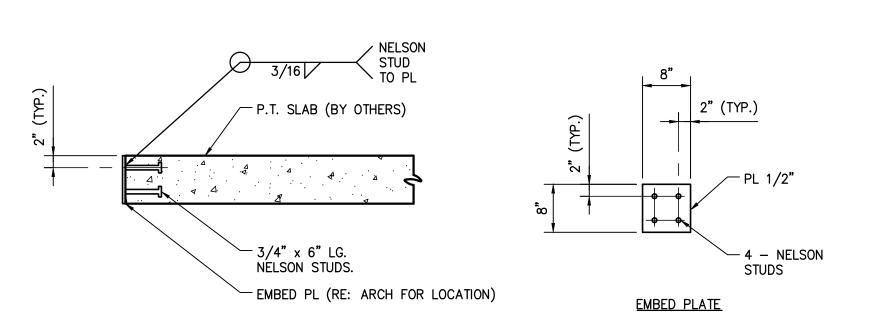
(CHANDELIER EMBED PLATE — BOTTOM SET) (FLAG POLE EMBED PLATE — TOP SET)

TYPICAL EMBED PLATE @ CHANDELIER & FLAG POLE LOCATIONS SCALE: 3/4" = 1'-0"



(BARRIER CABLE EMBED PLATE — TOP SET) (CANOPY PARAPET EMBED PLATE — SIDE SET)

TYPICAL EMBED PLATE IN 8" OR 12" P.T. SLAB DETAIL @ BARRIER CABLE & CANOPY PARAPET LOCATIONS SCALE: 3/4" = 1'-0"







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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

S2.1

31593

DETAILS

PHASE: CD

PROJECT #: 1601

DRAFTER: JRN

CHECKER: JBH

SCALE: AS NOTED

ISSUED: 04/20/2018

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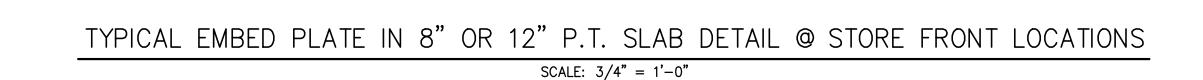
STREET, LOUISIANA

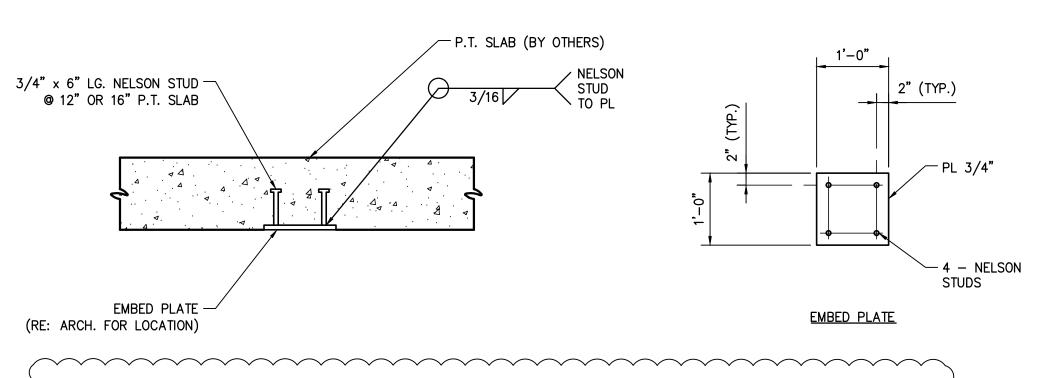
1031 CANAL NEW ORLEANS

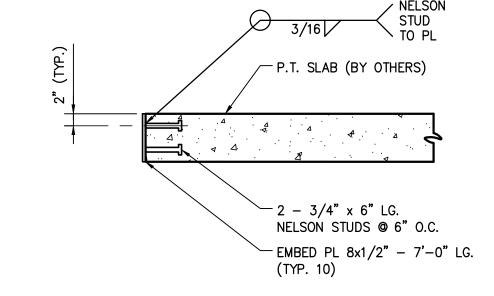
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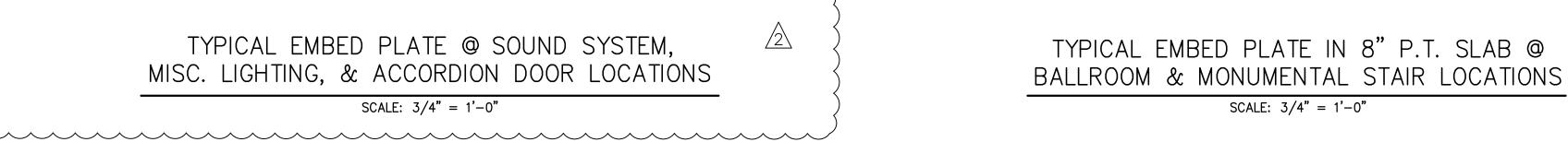
PROFESSIONAL ENGINEER

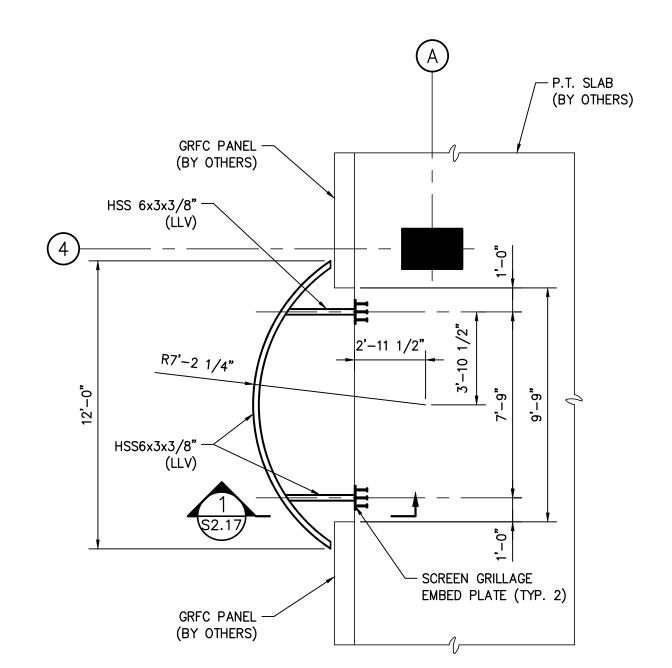
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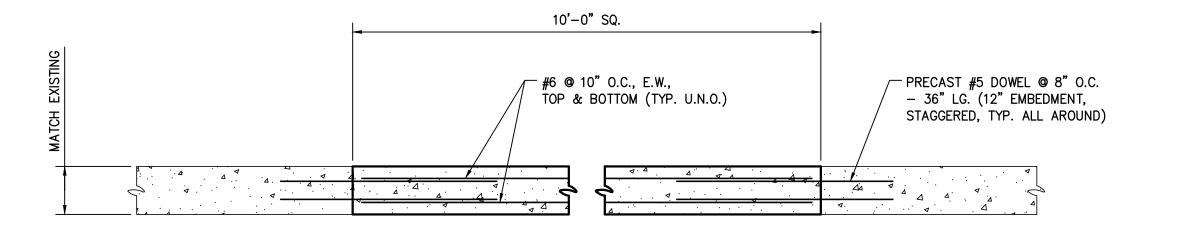






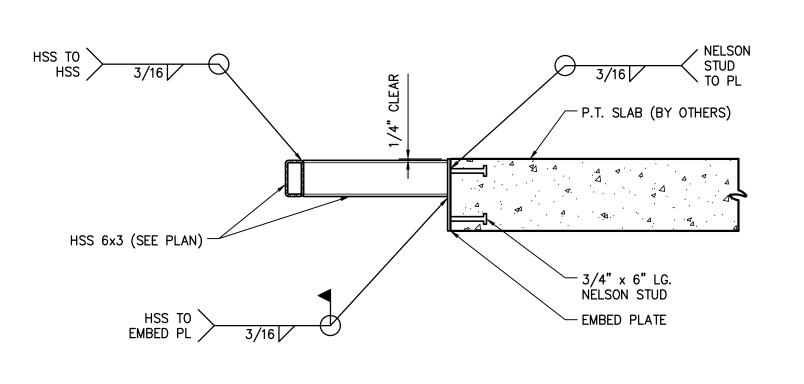
ALUMINUM SCREEN PLAN

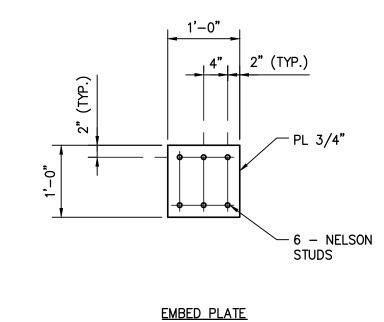
SCALE: 1/4" = 1'-0"



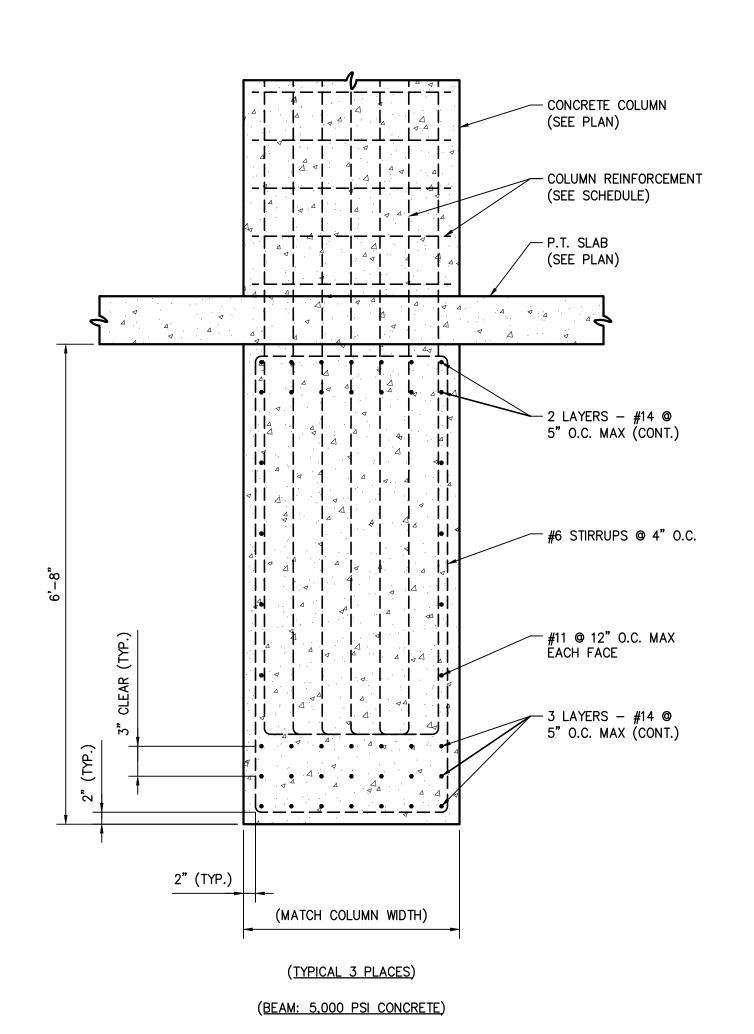
TYPICAL SLAB INFILL DETAIL @ CRANE OPENINGS

SCALE: 3/4" = 1'-0"



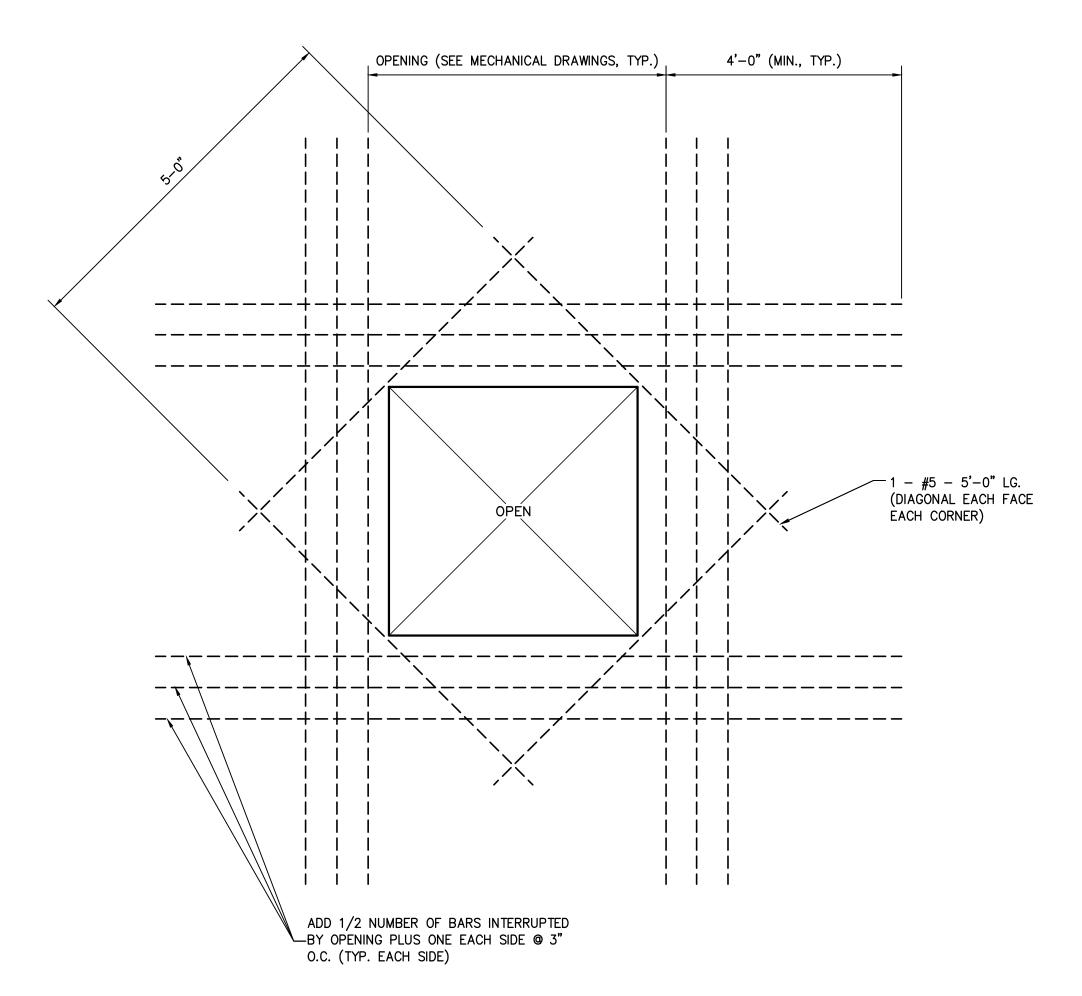


SECTION 1 SCALE: 3/4" = 1'-0" S2.17 S2.17



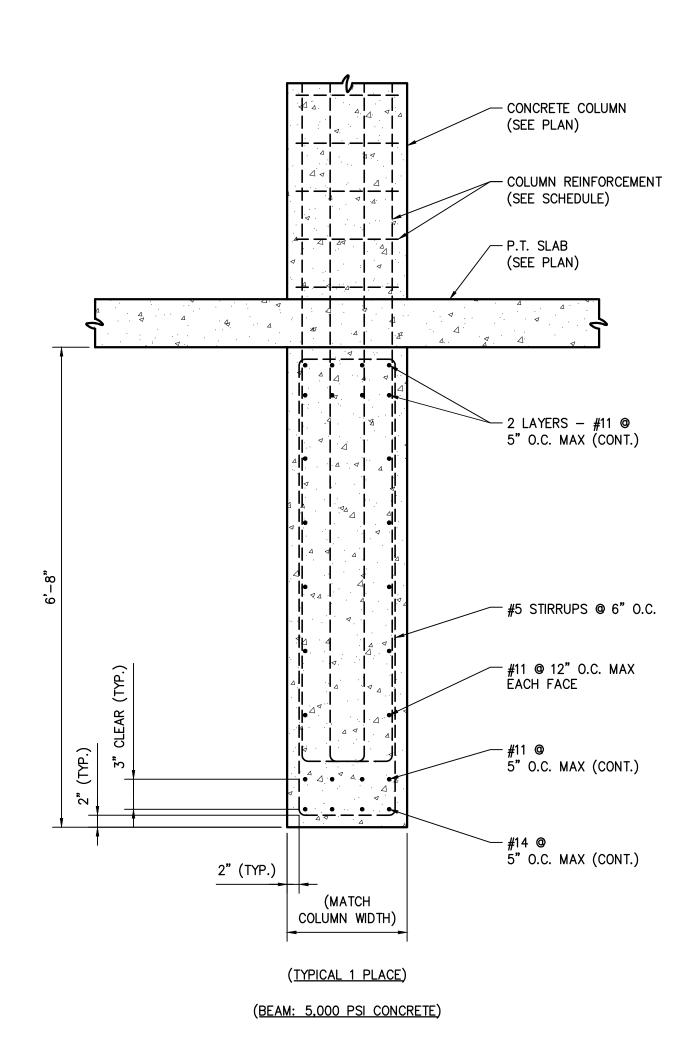
TYPICAL INTERIOR COLUMN SUPPORT BEAM

SCALE: 3/4" = 1'-0"



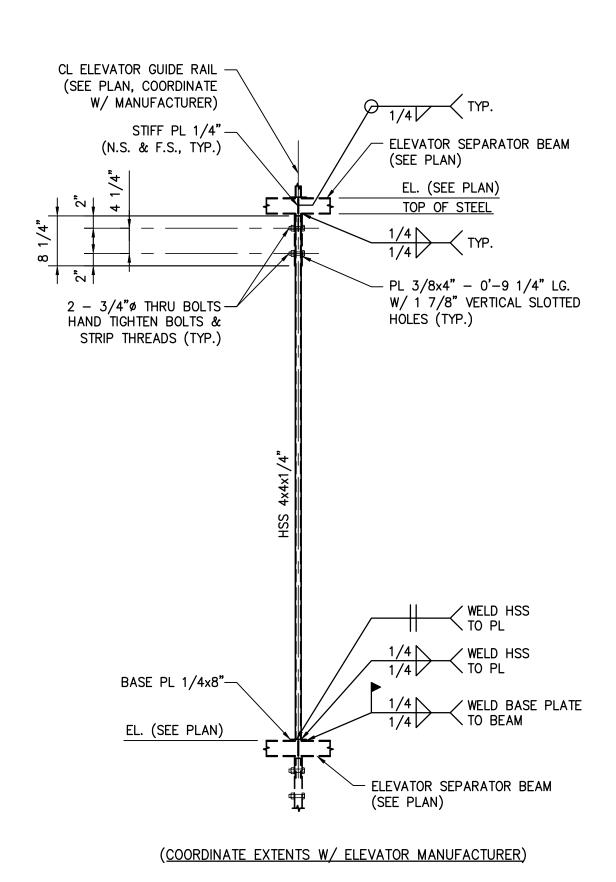
TYPICAL SHEAR WALL OPENING DETAIL

SCALE: 3/4" = 1'-0"



TYPICAL EXTERIOR COLUMN SUPPORT BEAM

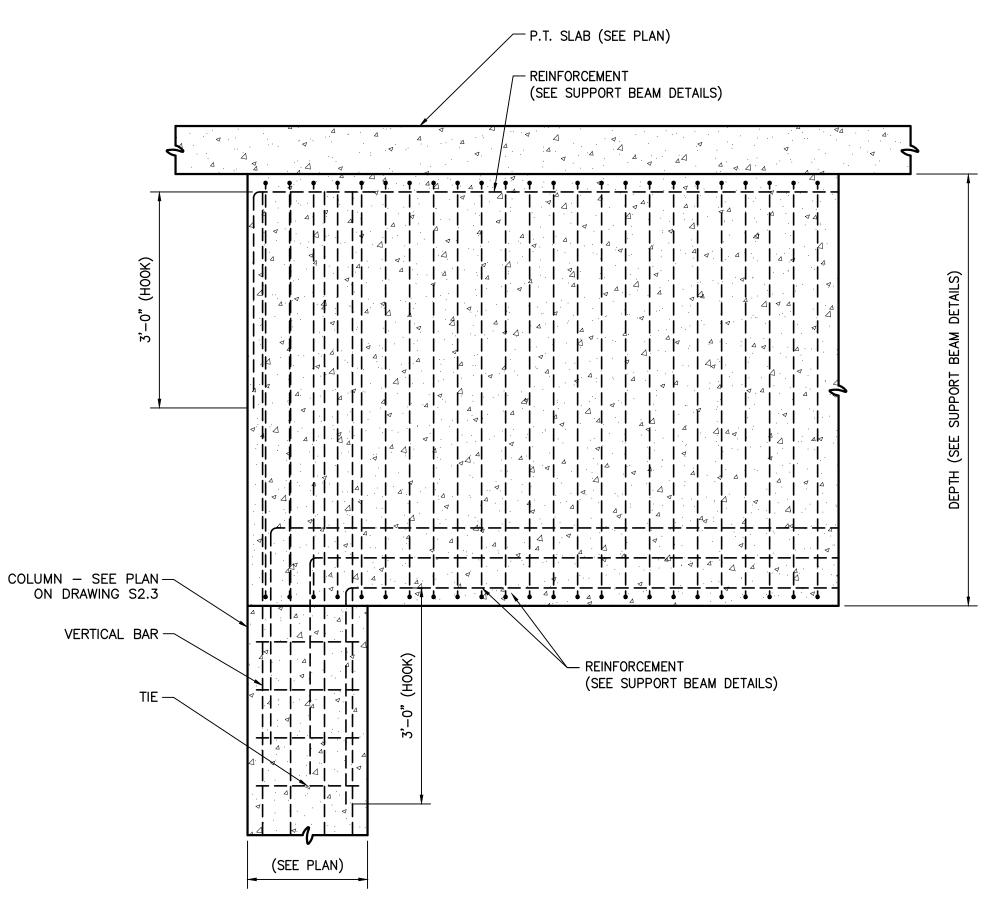
SCALE: 3/4" = 1'-0"



TYPICAL COLUMN

CONNECTION DETAIL @ ELEVATORS

SCALE: N.T.S.

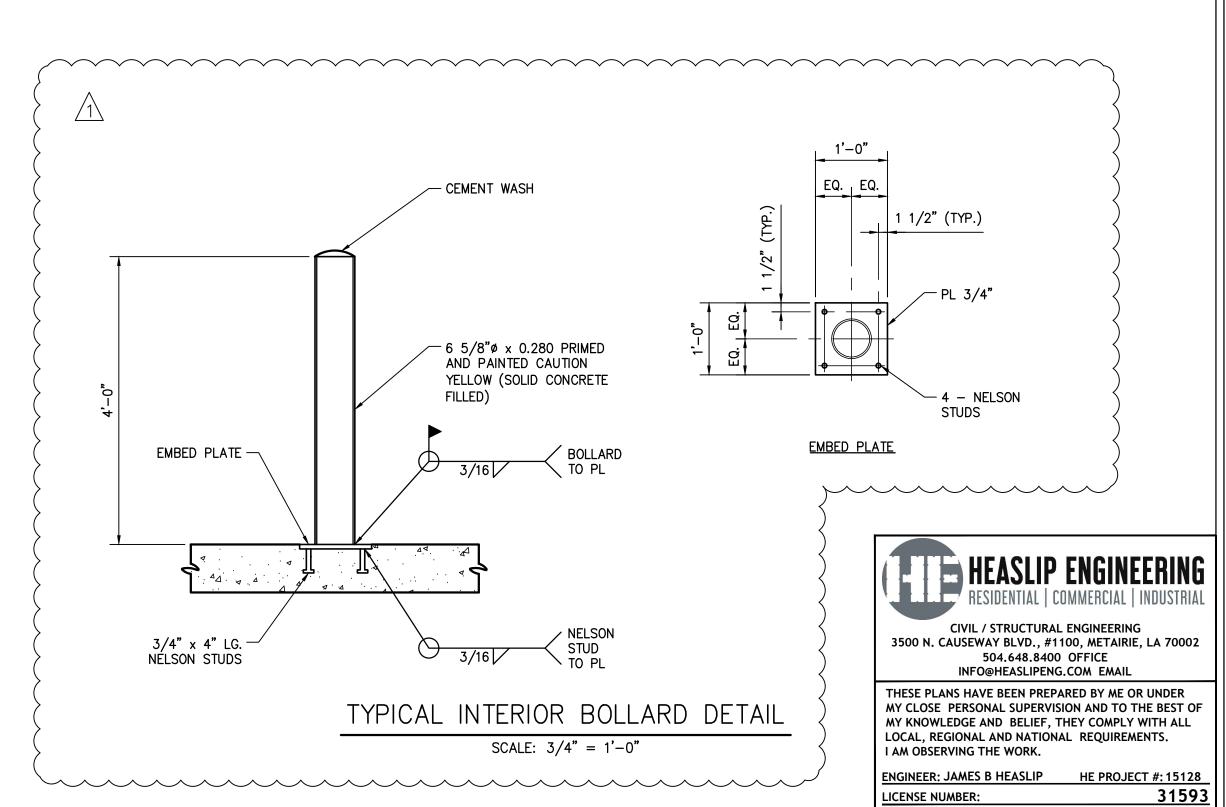


TYPICAL SUPPORT BEAM TO MAIN CONCRETE COLUMN

SCALE: 3/4" = 1'-0"

NOTE:

FOR #14 REBAR, REINFORCING SHALL BE IN ACCORDANCE WITH ASTM 1615 GRADE 75.



HARRY
BAKER
SMITH
SMITH
SMITH
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SMITH
L C

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AL STREET VS, LOUISIANA

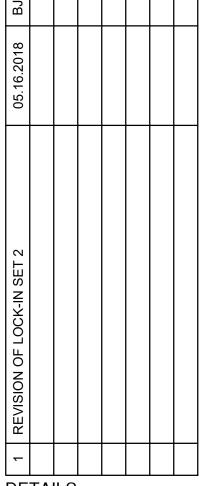
1031 CANAL STF NEW ORLEANS, LO

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CERTIFIED CORRECT

JAMES B. HEASLIP II REG. No. 31593
REGISTERED
PROFESSIONAL ENGINEER
IN

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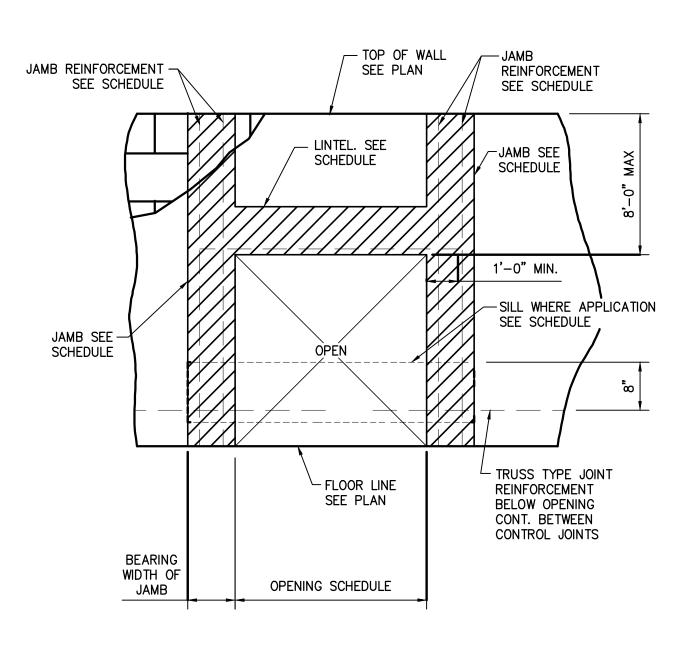


PROJECT #: 1601
PHASE: CD

PHASE: CD
DRAFTER: JRN
CHECKER: JBH
SCALE: AS NOTED

CHECKER: JBH
SCALE: AS NOTED
ISSUED: 04/20/2018

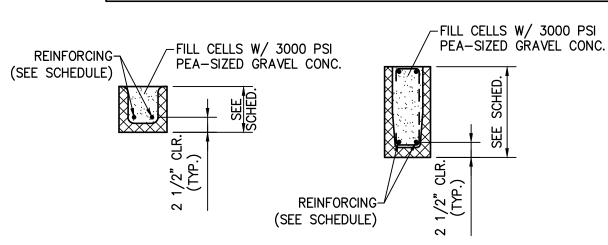
S2.18



CMU LINT	TEL/SILL SCHEDULE-CONCRETE MAS	SONRY UNITS
OPENING SIZE (UP TO & INCLUDING)	LINTEL/SILL "U-BLOCK" SIZE AND REINFORCING	JAMB REINFORCING (BEAR LINTEL EA. END, FULL DIMENSIONS SHOWN)
4'-0"	16" "U-BLOCK" 2 - #4 (CONT., BOT.)	8x16 • • 2 - #6 VERT.
6'-0"	16" "U-BLOCK 2 - #4 (CONT., BOT.)	8x16
9'-0"	16" U-BLOCK" 2 - #5 (CONT., BOT.)	8x16 2 - #6 VERT.
11'-0"	16" "U-BLOCK" 2 - #5 (CONT., TOP & BOT.)	8x16 • • 2 - #7 VERT.
CMU LINTEL/SILL	NOTES: (U.N.O.)	

JAMBS TO EXTEND FROM SUPPORTING STRUCTURE/FLOOR LEVEL BELOW OPENING TO FLOOR LEVEL ABOVE OPENING.

ALL JAMBS ARE TO FILLED SOLID FULL HEIGHT W/ 3000 PSI PEA GRAVEL AGGREGATE CONCRETE 3. AT 16" "U−BLOCKS", PROVIDE #2 ☐ @ 24" O.C.

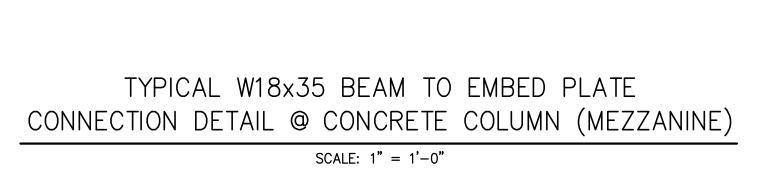


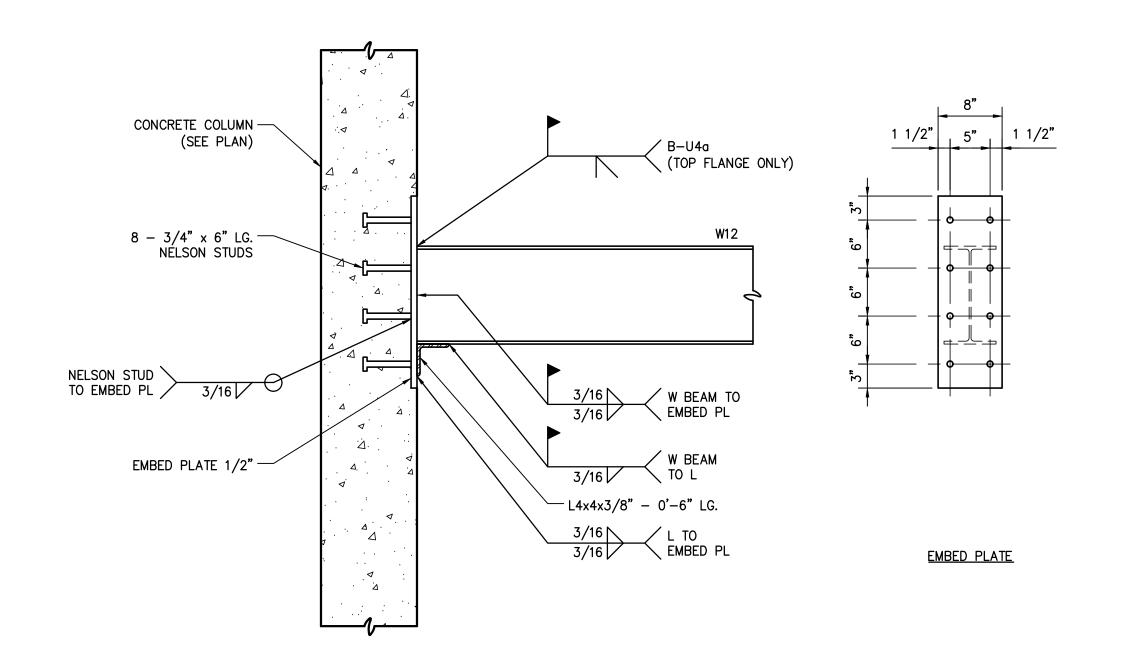
TYPICAL CONCRETE MASONRY LINTEL DETAIL SCALE: 3/4" = 1'-0"

CONCRETE COLUMN -(SEE PLAN) B-U4a (TOP FLANGE ONLY) W24 12 - 3/4" x 6" LG. — NELSON STUDS NELSON STUD TO EMBED PL 3/16 W BEAM TO EMBED PL EMBED PLATE 3/4" EMBED PLATE -14x4x3/8" - 0'-6" LG.3/16 L TO EMBED PL

> TYPICAL W24x55 BEAM TO EMBED PLATE CONNECTION DETAIL @ CONCRETE COLUMN (MEZZANINE) SCALE: 1" = 1'-0"

CONCRETE COLUMN -(SEE PLAN) 1 1/2" 5" 1 1/2" B-U4a (TOP FLANGE ONLY) 8 - 3/4" x 6" LG. — NELSON STUDS NELSON STUD TO EMBED PL 3/16 3/16 W BEAM TO EMBED PL EMBED PLATE 3/4" -14x4x3/8" - 0'-6" LG.EMBED PLATE





TYPICAL W12x26 BEAM TO EMBED PLATE CONNECTION DETAIL @ CONCRETE COLUMN (MEZZANINE) SCALE: 1'' = 1'-0''



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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

31593

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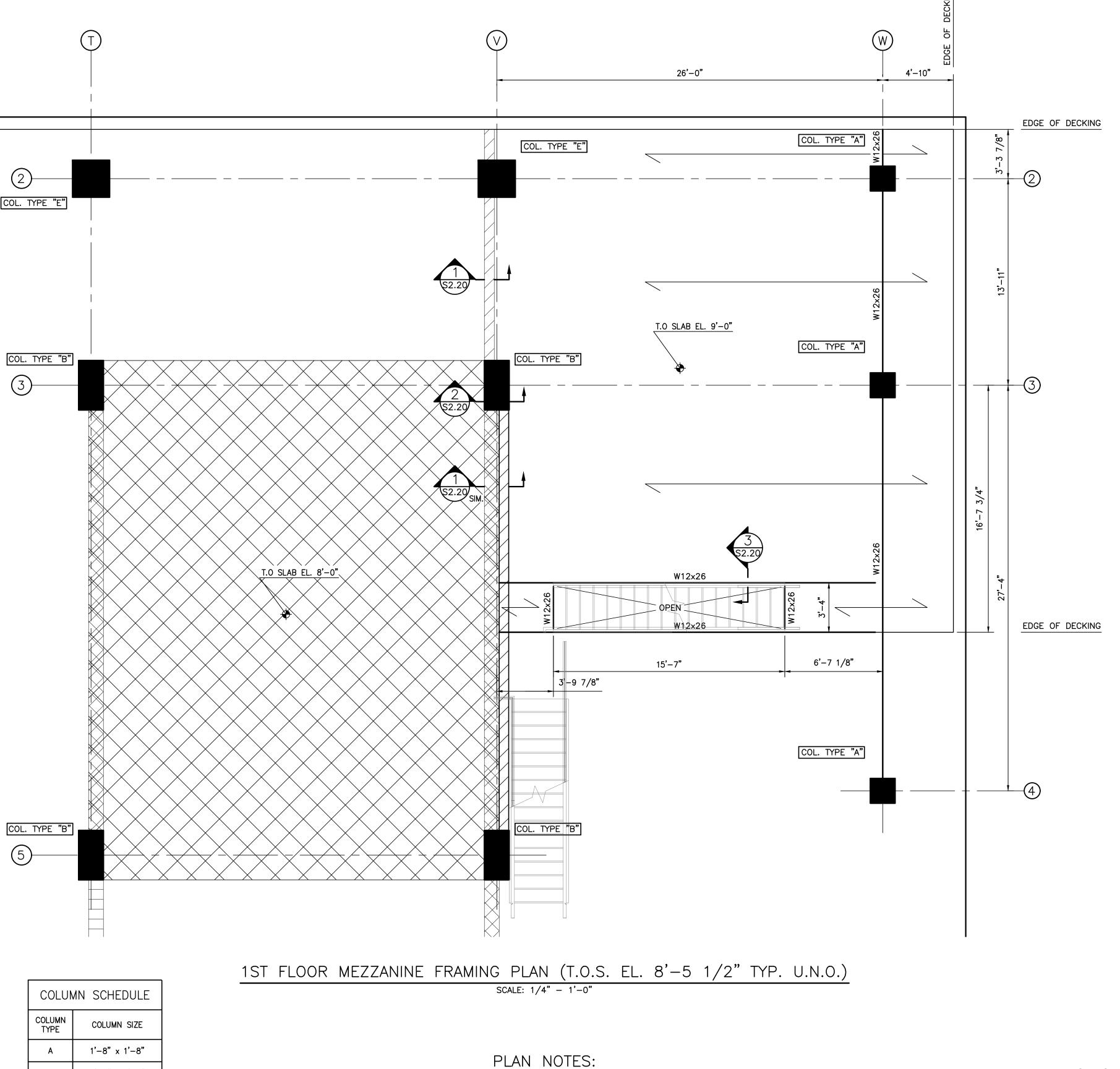
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PROJECT #: 1601 PHASE: CD DRAFTER: JRN

CHECKER: JBH SCALE: AS NOTED ISSUED: 04/20/2018



00201	WIT COMEDCE
COLUMN TYPE	COLUMN SIZE
Α	1'-8" x 1'-8"
В	3'-4" x 1'-8"
С	3'-0" x 1'-0"
D	1'-6" x 2'-0"
E	2'-6" x 2'-6"
F	1'-8" x 2'-6"
G	2'-0" ø
	·

1. SEE DRAWING S2.11 FOR COLUMN DETAILS.

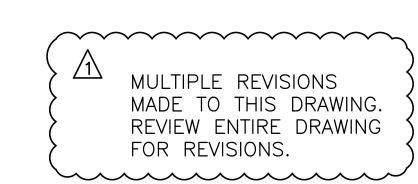
2. — SYMBOL ON PLAN INDICATES METAL DECKING DIRECTION.

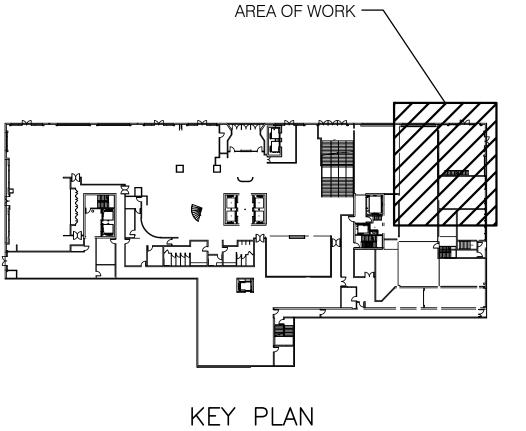
3. CANTALLE - SYMBOL ON PLAN INDICATES 8" CMU BLOCK WALL.

4. SYMBOL ON PLAN INDICATES 12" CMU BLOCK WALL.

5. SYMBOL ON PLAN INDICATES 12" CONCRETE WALL.

- SYMBOL ON PLAN INDICATES 12" POST TENSION CONCRETE SLAB (BY OTHERS).





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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128
LICENSE NUMBER: 31593

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ARCHITECTS II
P L C

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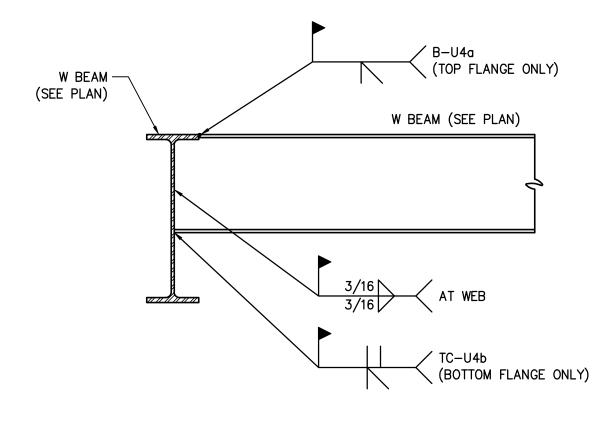


	REVISION HISTORY									
BJD										
05.01.2018										
N OF LOCK-IN SET 2										

1ST FLOOR
MEZZANINE
PROJECT #: 1601

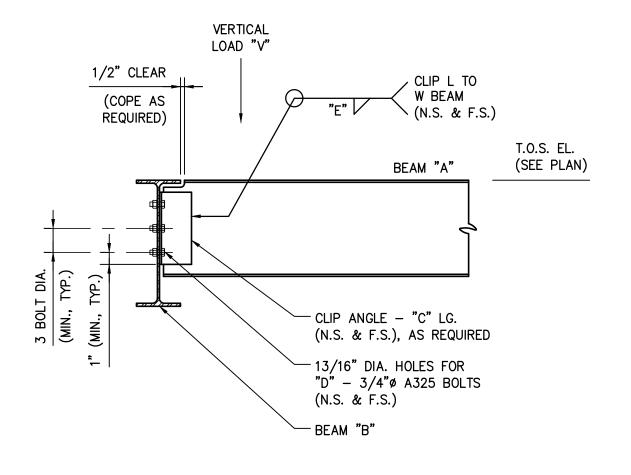
PHASE: CD
DRAFTER: JRN
CHECKER: JBH

CHECKER: JBH
SCALE: AS NOTED
ISSUED: 04/20/2018



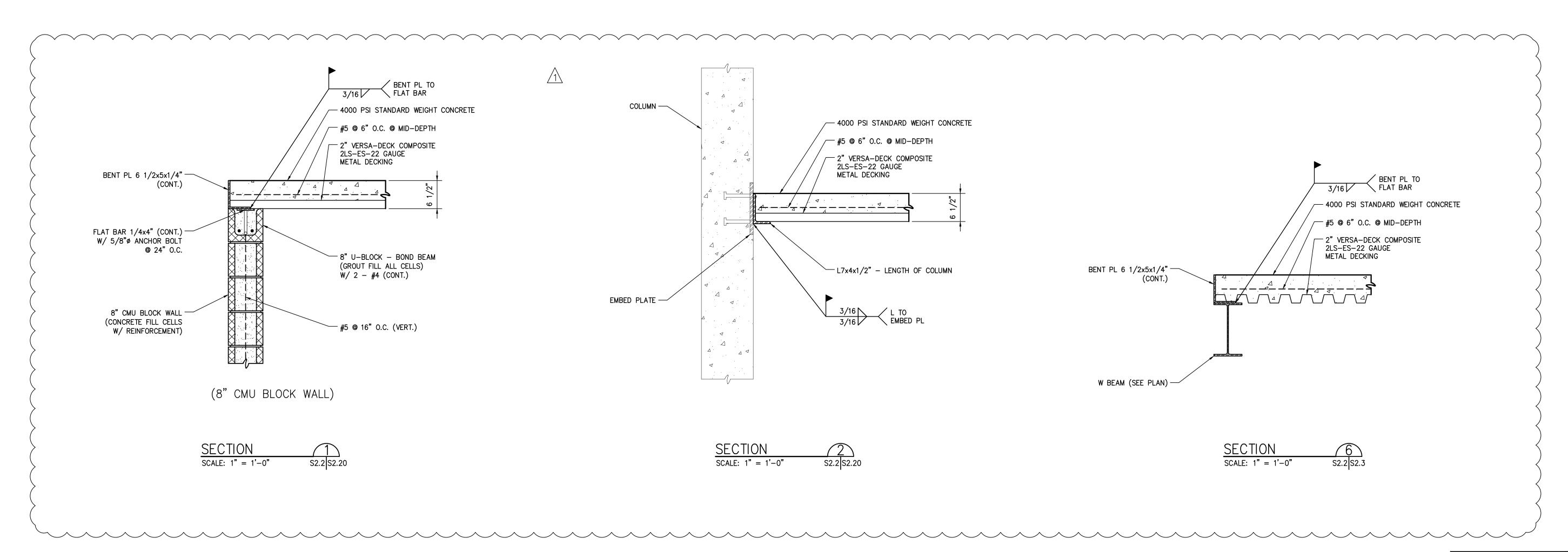
TYPICAL BEAM TO BEAM "SAME" FULL PENETRATION WELD CONNECTION DETAIL SCALE: 1" = 1'-0"

TYPICAL BEAM TO BEAM "DIFFERENT" FULL PENETRATION WELD CONNECTION DETAIL SCALE: 1" = 1'-0"



TYPICAL BEAM TO BEAM CONNECTION DETAIL SCALE: 1" = 1'-0"

BEAM "A"	BEAM "B"	VERTICAL LOAD (KIPS) "V"	CLIP ANGLE LENGTH "C" (MIN.)	NUMBER OF BOLTS "D" (MIN.)	WELD "E" (MIN.)
W12x26	W12x26	5	0'-8 1/2"	2	3/16
W12x26	W18x35	5	0'-8 1/2"	2	3/16
W18x35	W18x35	15	0'-11 1/2"	3	3/16
W18x35	W24x55	15	0'-11 1/2"	3	3/16





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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

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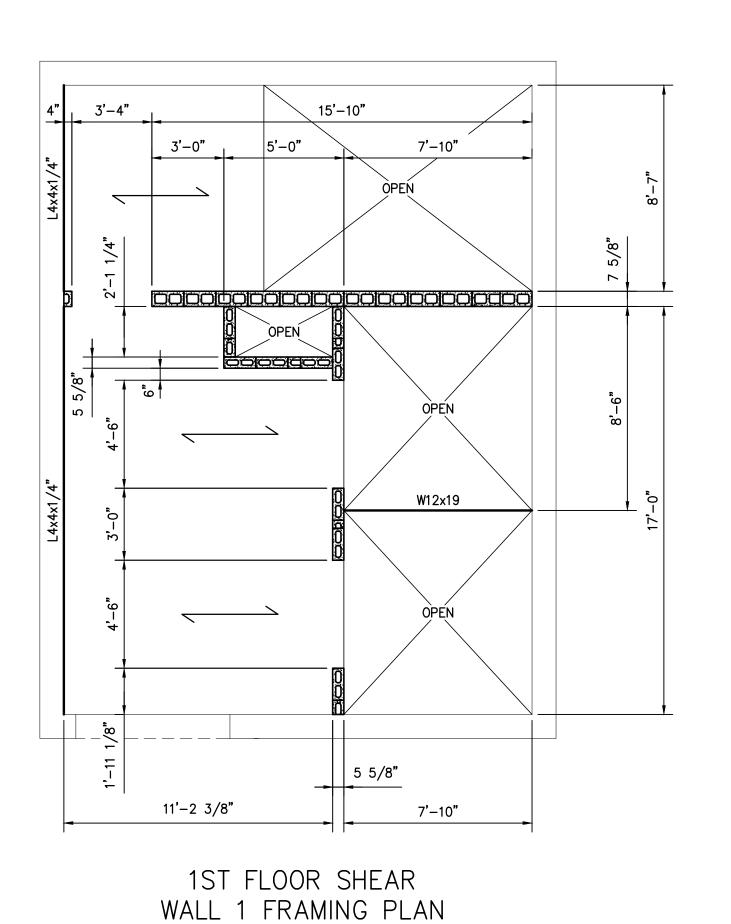
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	 LVIC	DIOIN	ППС	TOR	
BJD					
05.01.2018					
1 REVISION OF LOCK-IN SET 2					

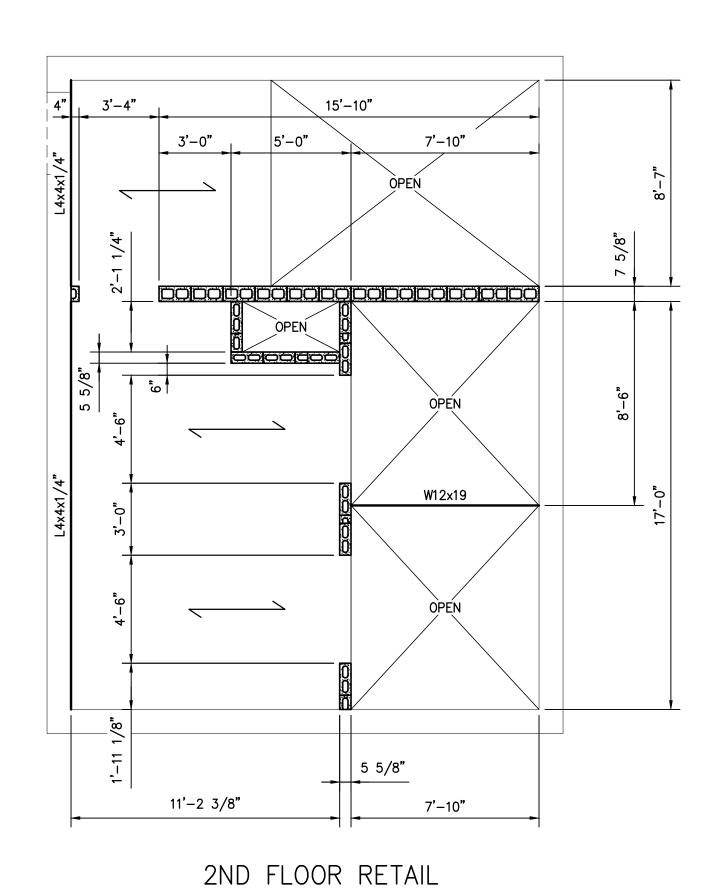
DETAILS PROJECT #: 1601 PHASE: CD

DRAFTER: JRN CHECKER: JBH

SCALE: AS NOTED ISSUED: 04/20/2018

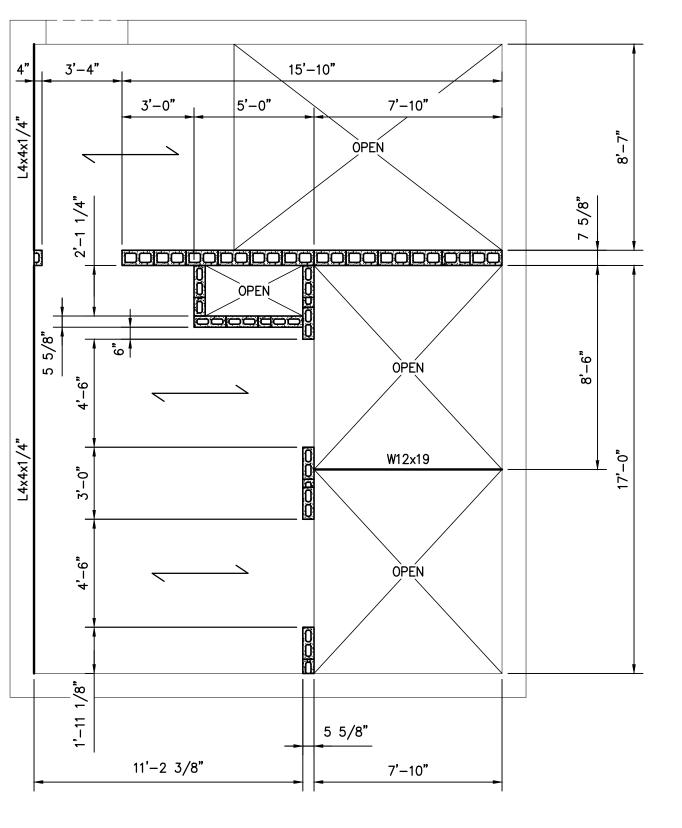


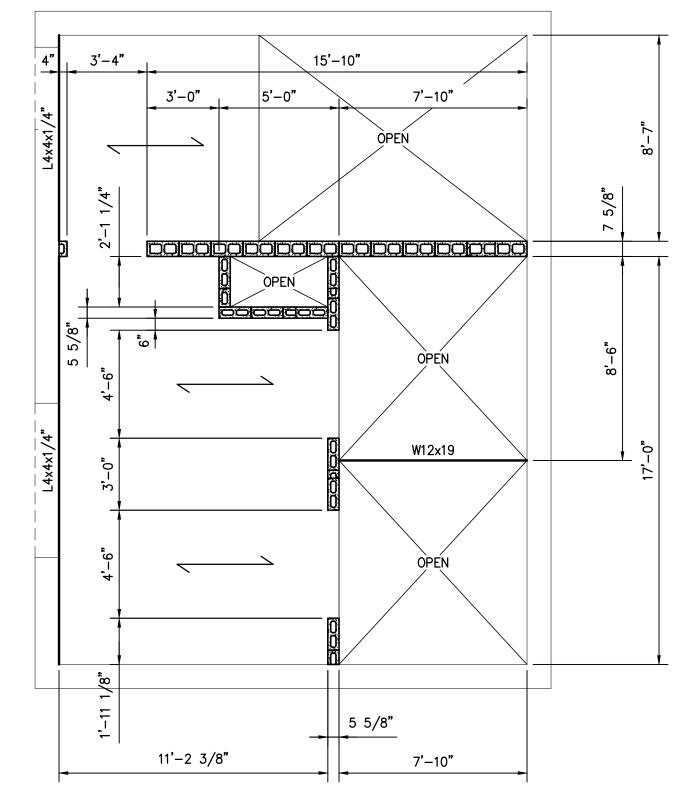
SCALE: 1/4" = 1'-0"



SHEAR WALL 1 FRAMING PLAN

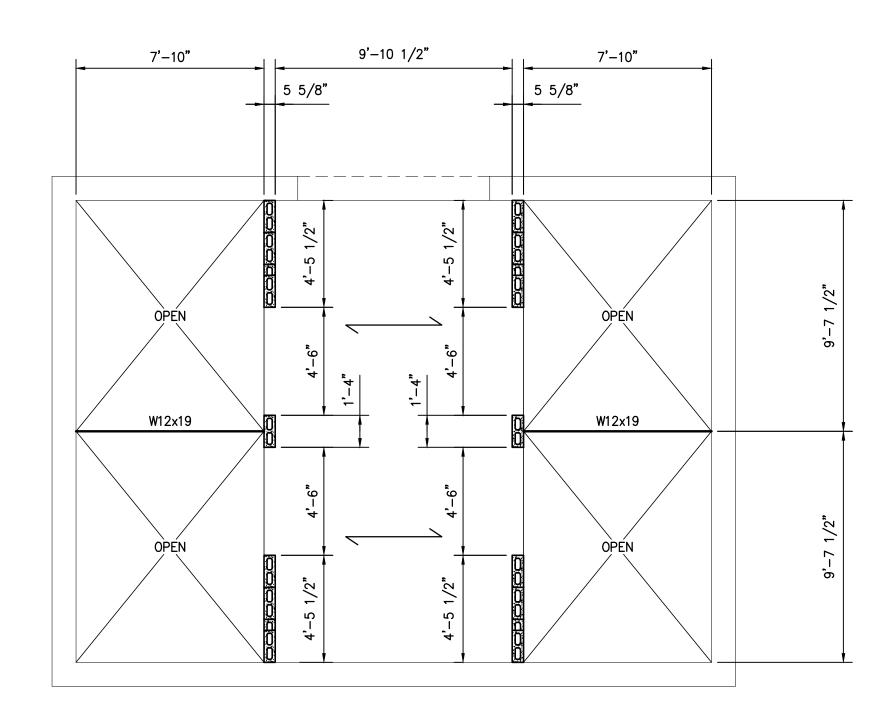
SCALE: 1/4" = 1'-0"



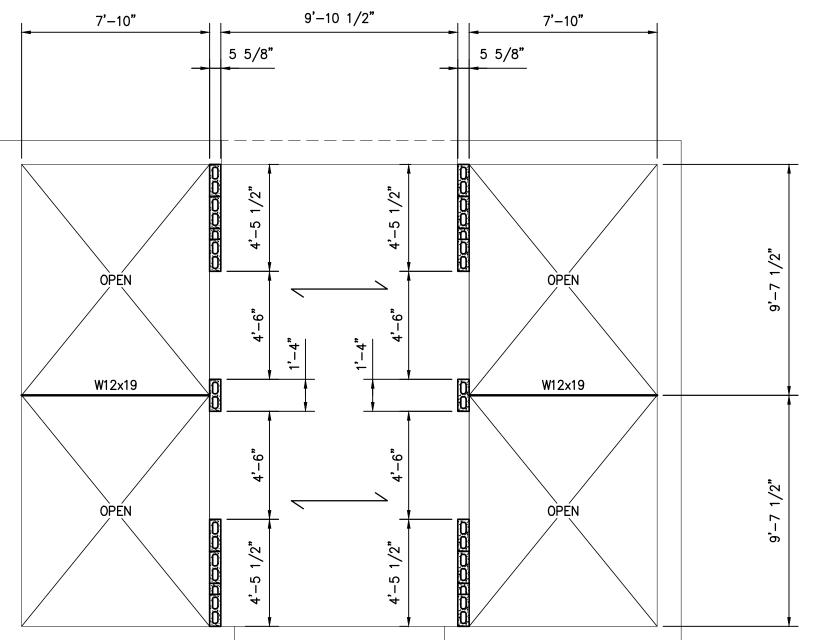


2ND FLOOR MEZZANINE SHEAR WALL 1 FRAMING PLAN SCALE: 1/4" = 1'-0"

3RD - 7TH FLOOR SHEAR WALL 1 FRAMING PLAN SCALE: 1/4" = 1'-0"



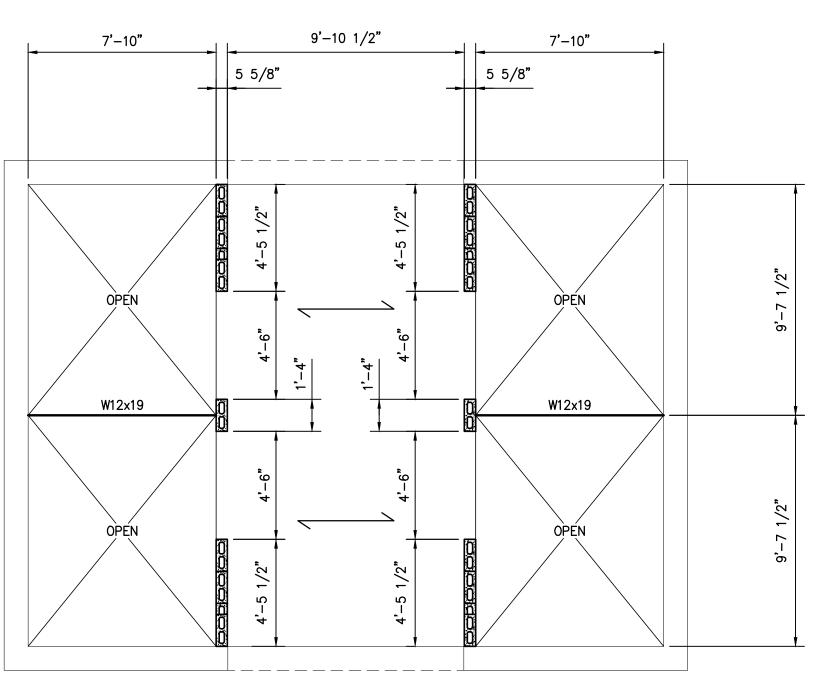




2ND FLOOR RETAIL SHEAR WALL 2 FRAMING PLAN SCALE: 1/4" = 1'-0"

PLAN NOTES:

- 1. FOR ALL NOTES, SEE DRAWING S1.0A S1.0B.
- OOOOOOO SYMBOL ON PLAN INDICATES 6" CMU BLOCK WALL W/ #5 @ 24" O.C. (GROUT FILL ALL CELLS W/ REINFORCEMENT).
- SYMBOL ON PLAN INDICATES 8" CMU BLOCK WALL W/ #5 @ 24" O.C. (GROUT FILL ALL CELLS W/ REINFORCEMENT). 3. 0000000 -
- 4. SYMBOL ON PLAN INDICATES METAL DECKING DIRECTION.



2ND FLOOR MEZZANINE SHEAR WALL 2 FRAMING PLAN SCALE: 1/4" = 1'-0"

> MULTIPLE REVISIONS MADE TO THIS DRAWING. REVIEW ENTIRE DRAWING FOR REVISIONS. ~~~~~~~



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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593 S2.21

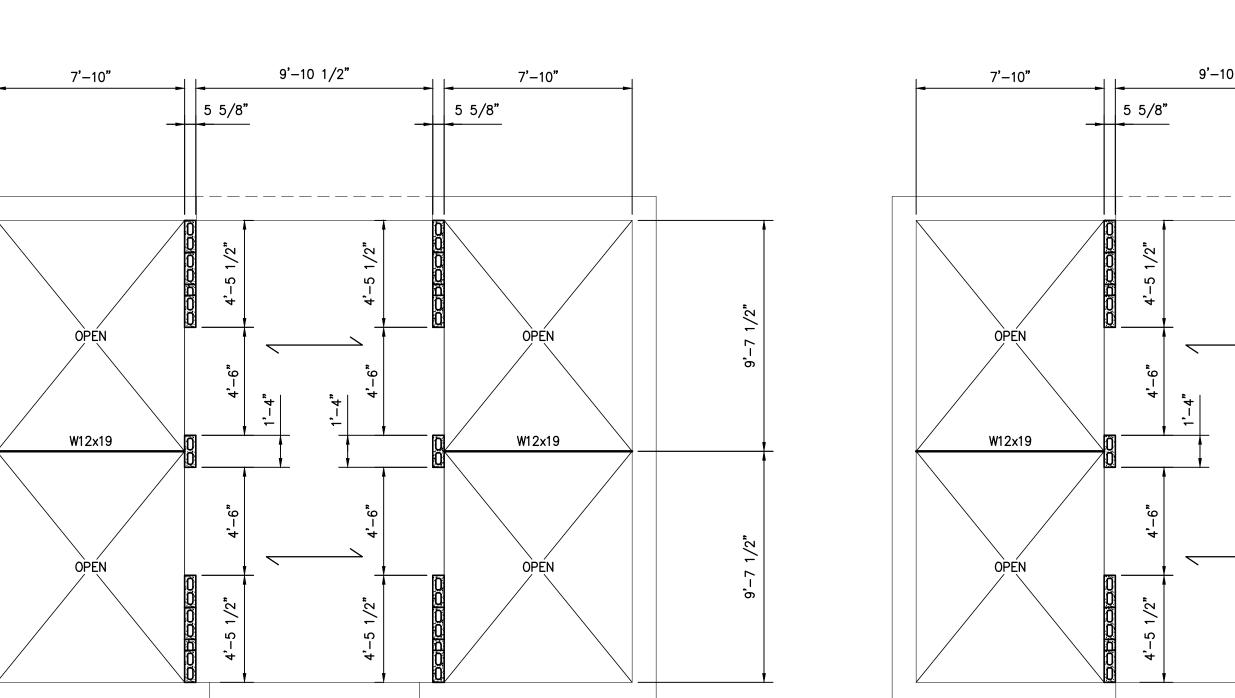
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PROFESSIONAL ENGINEER
IN



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SHEAR WALL

PROJECT #: 1601

PHASE: CD

DRAFTER: JRN

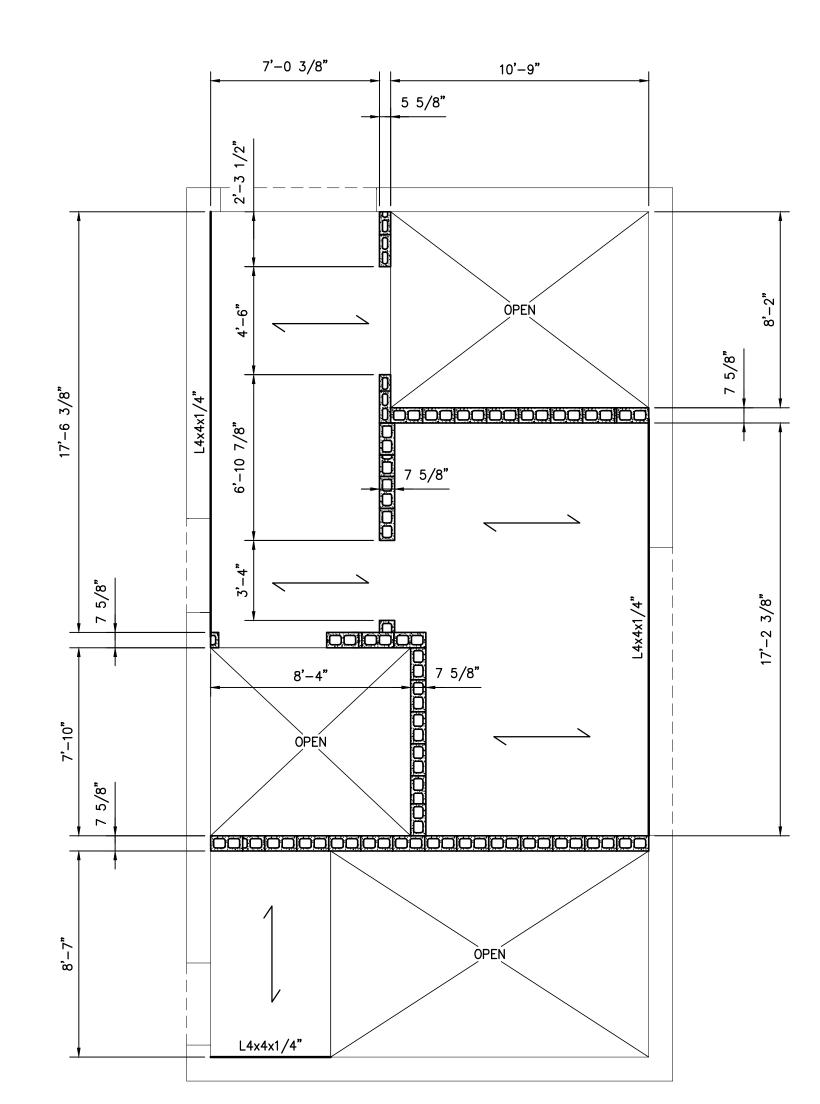
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SCALE: AS NOTED

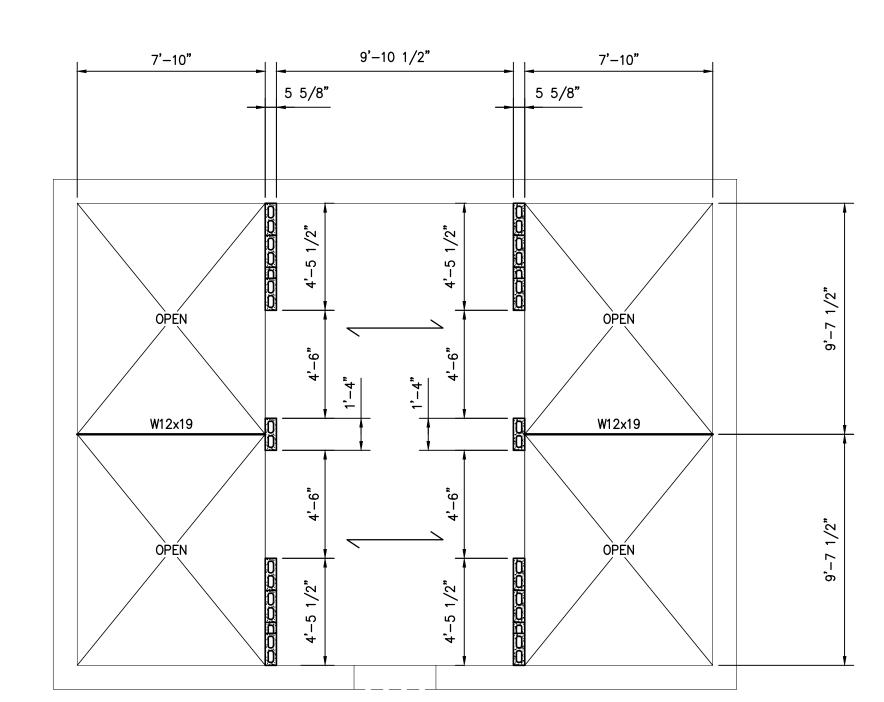
ISSUED: 04/20/2018

FRAMING PLANS

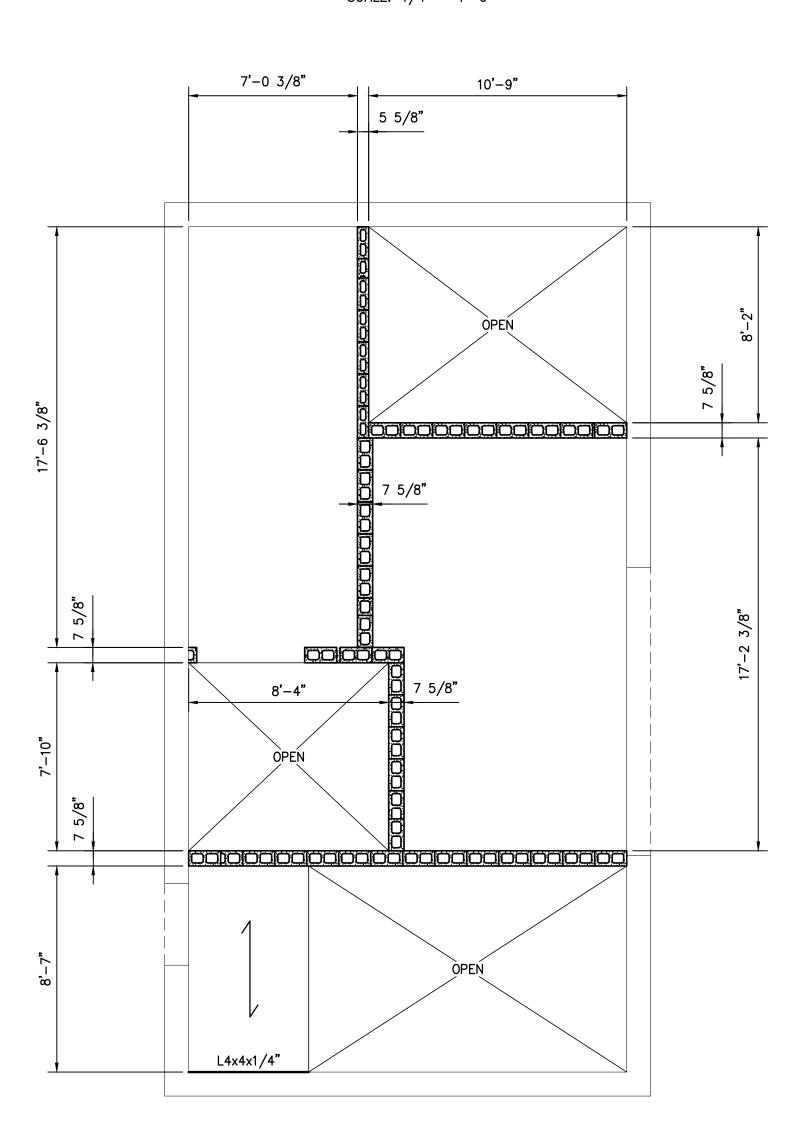
3RD, 4TH, 6TH, & 7TH FLOOR SHEAR WALL 2 FRAMING PLAN SCALE: 1/4" = 1'-0"



1ST FLOOR SHEAR WALL 3 FRAMING PLAN SCALE: 1/4" = 1'-0"



5TH FLOOR SHEAR WALL 2 FRAMING PLAN SCALE: 1/4" = 1'-0"



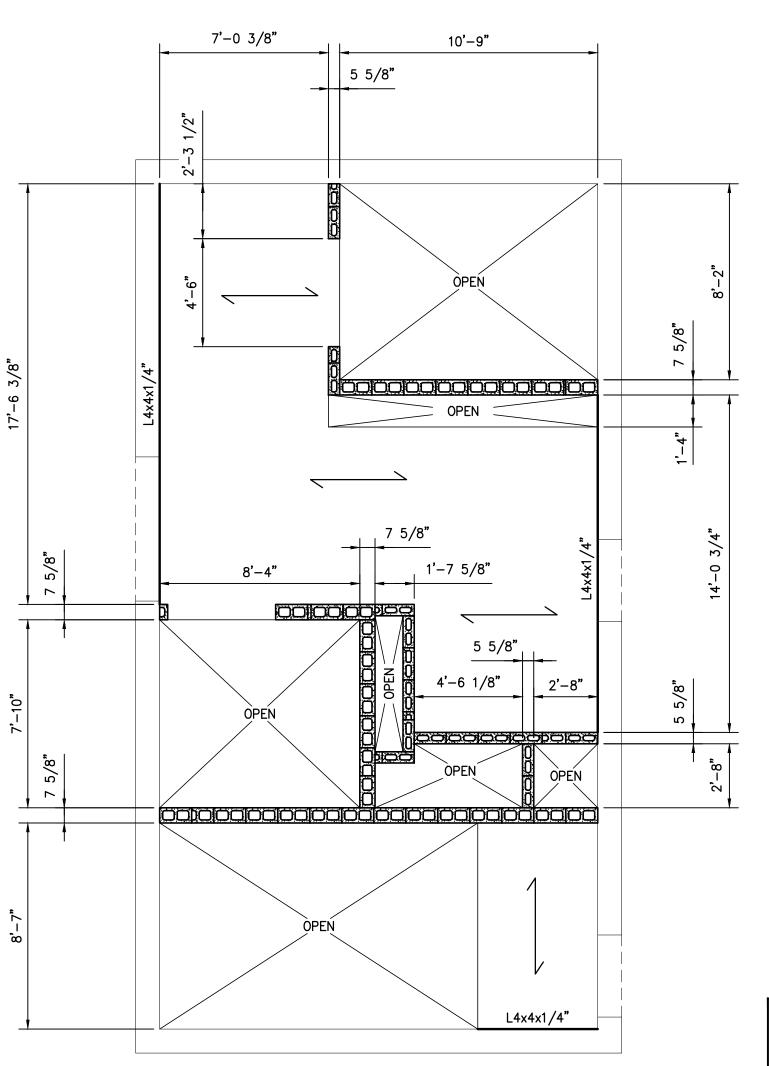
INTERMEDIATE PARKING FLOOR SHEAR WALL 3 FRAMING PLAN SCALE: 1/4" = 1'-0"

PLAN NOTES:

- 1. FOR ALL NOTES, SEE DRAWING S1.0A S1.0B.
- 2. DODODO SYMBOL ON PLAN INDICATES 6" CMU BLOCK WALL W/ #5 @ 24" O.C. (GROUT FILL ALL CELLS W/ REINFORCEMENT).
- 3. DDDDDDD SYMBOL ON PLAN INDICATES 8" CMU BLOCK WALL W/ #5 @ 24" O.C. (GROUT FILL ALL CELLS W/ REINFORCEMENT).
- SYMBOL ON PLAN INDICATES METAL DECKING DIRECTION.

MULTIPLE REVISIONS MADE TO THIS DRAWING. REVIEW ENTIRE DRAWING FOR REVISIONS.

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2ND FLOOR RETAIL SHEAR WALL 3 FRAMING PLAN SCALE: 1/4" = 1'-0"



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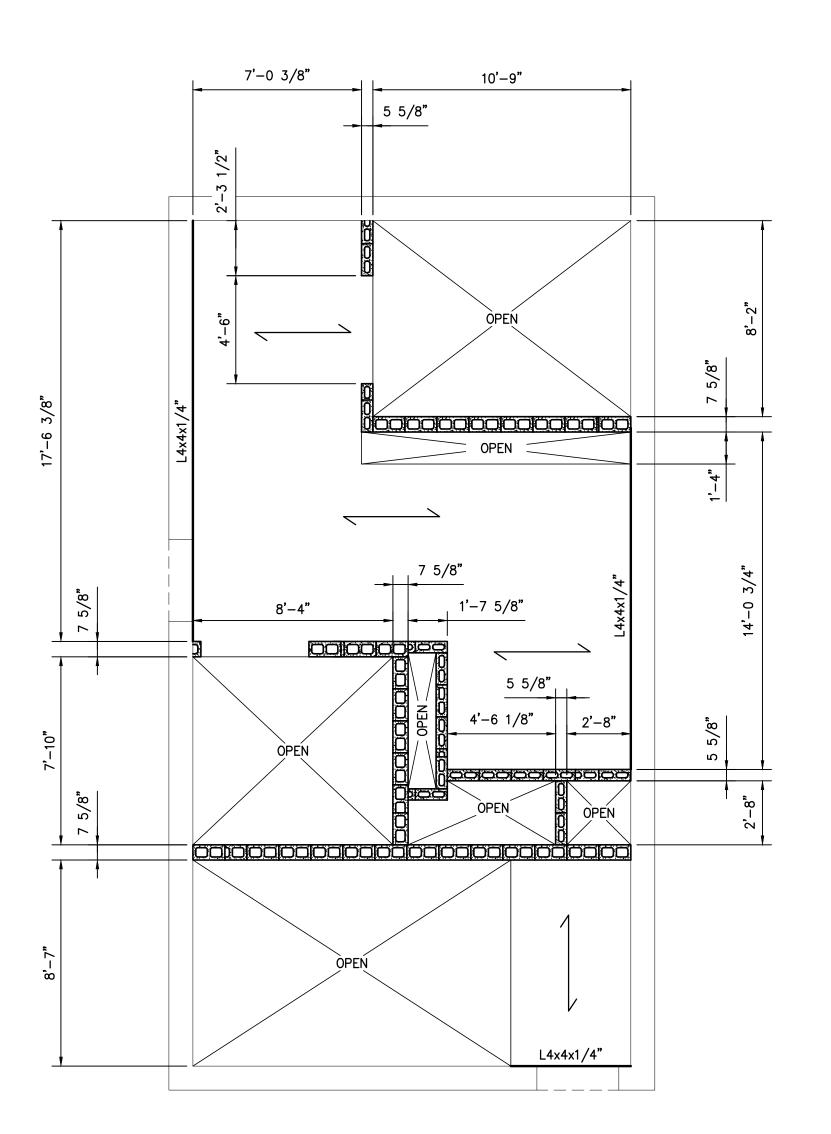


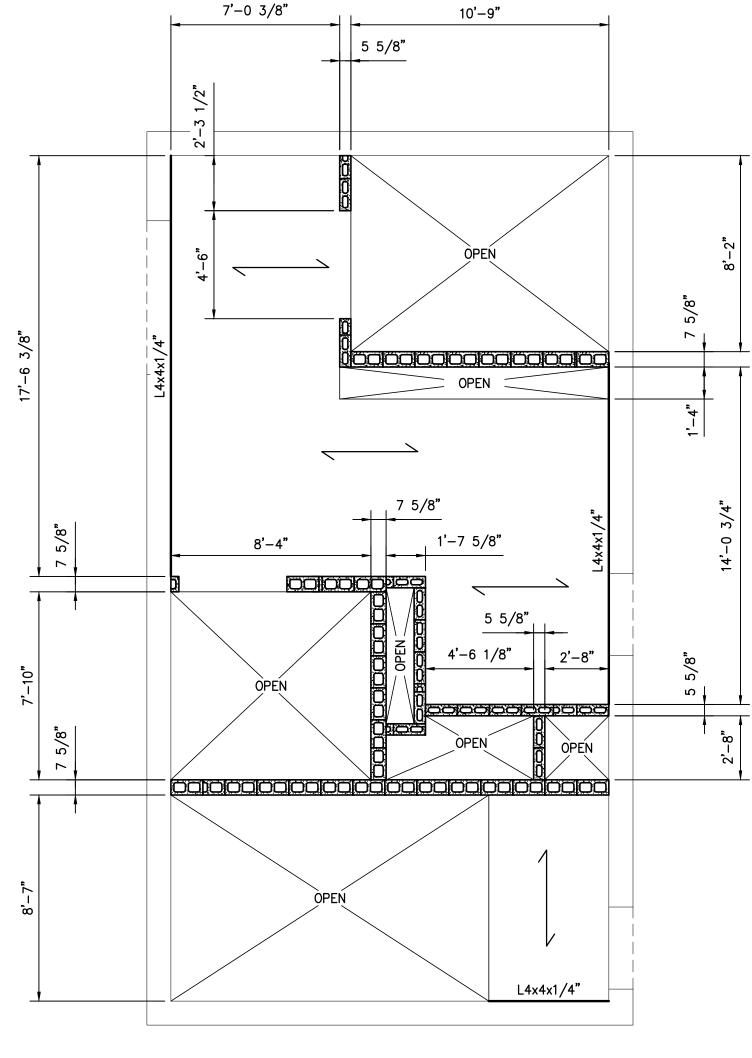
|                           | F                 | REVIS | SION | HIS | STOR | Υ |  |
|---------------------------|-------------------|-------|------|-----|------|---|--|
| BJD                       | ara               |       |      |     |      |   |  |
| 04.30.2018                | 06.06.2018        |       |      |     |      |   |  |
| REVISION OF LOCK-IN SET 2 | GENERAL REVISIONS |       |      |     |      |   |  |

FRAMING PLANS PROJECT #: 1601

DRAFTER: JRN CHECKER: JBH

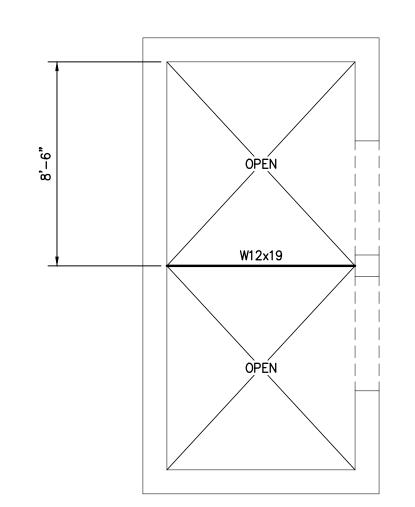
SCALE: AS NOTED ISSUED: 04/20/2018



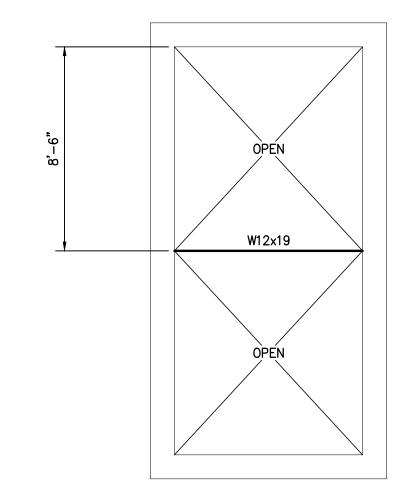


3RD - 7TH FLOOR SHEAR WALL 3 FRAMING PLAN

SCALE: 1/4" = 1'-0"



1ST & 3RD - 7TH FLOOR SHEAR WALL 6 FRAMING PLAN SCALE: 1/4" = 1'-0"



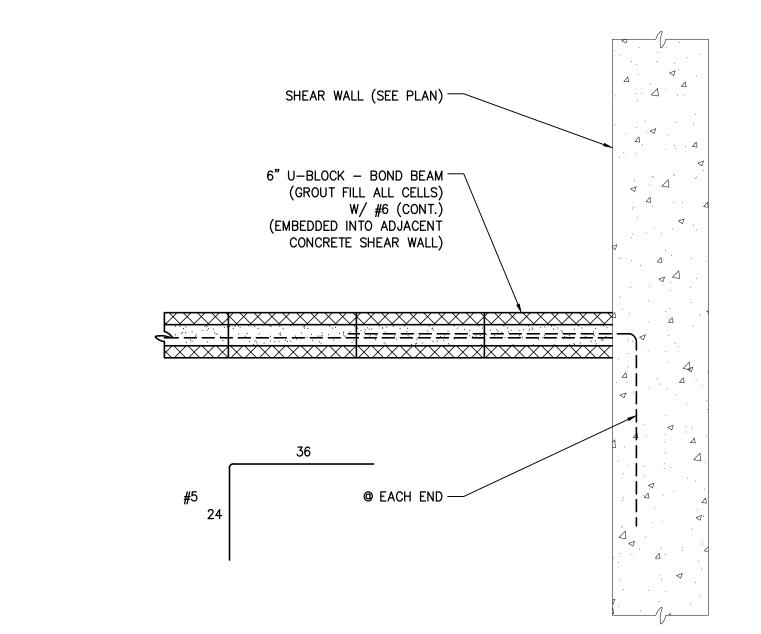
2ND FLOOR & 2ND FLOOR MEZZANINE SHEAR WALL 6 FRAMING PLAN

SCALE: 1/4" = 1'-0"

#### PLAN NOTES:

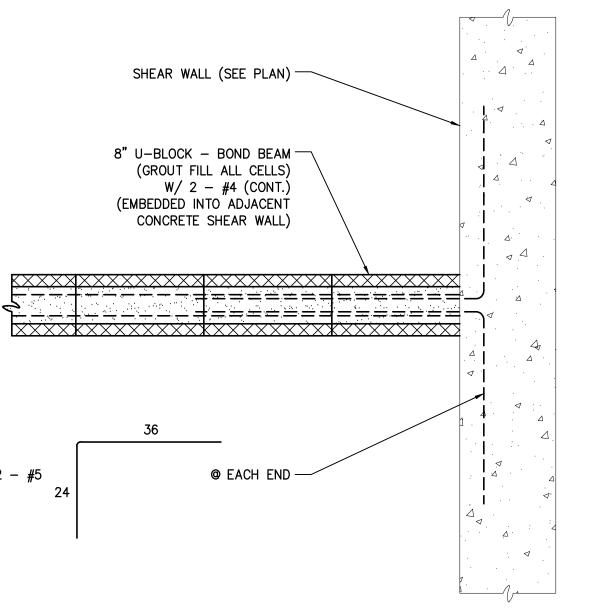
- SYMBOL ON PLAN INDICATES 6" CMU BLOCK WALL W/ #5 @ 24" O.C. (GROUT FILL ALL CELLS W/ REINFORCEMENT).
- SYMBOL ON PLAN INDICATES 8" CMU BLOCK WALL W/ #5 @ 24" O.C. (GROUT FILL ALL CELLS W/ REINFORCEMENT).
- SYMBOL ON PLAN INDICATES METAL DECKING DIRECTION.

2ND FLOOR MEZZANINE SHEAR WALL 3 FRAMING PLAN SCALE: 1/4" = 1'-0"



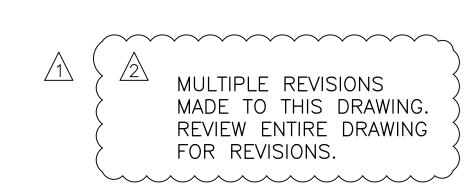
TYPICAL 6" U-BLOCK - BOND BEAM TO SHEAR WALL CONNECTION DETAIL SCALE: 1" = 1'-0"

(PLAN VIEW)



TYPICAL 8" U-BLOCK - BOND BEAM TO SHEAR WALL CONNECTION DETAIL

(PLAN VIEW)





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S2.23 31593

SHEAR WALL

PROJECT #: 1601

DRAFTER: JRN

CHECKER: JBH

SCALE: AS NOTED

ISSUED: 04/20/2018

PHASE: CD

FRAMING PLANS

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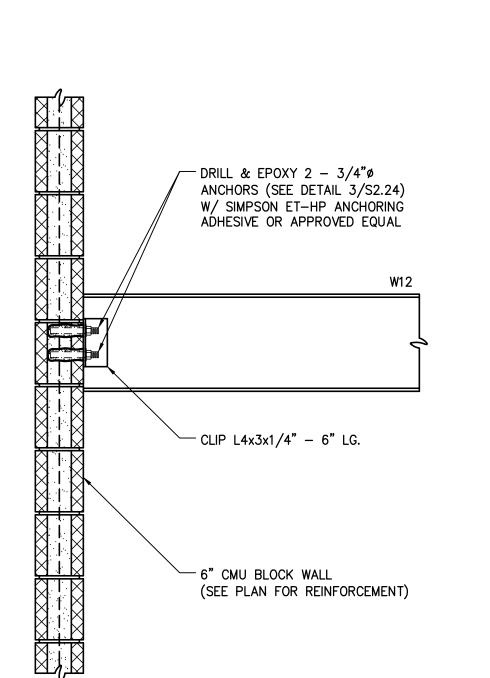
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PROFESSIONAL ENGINEER

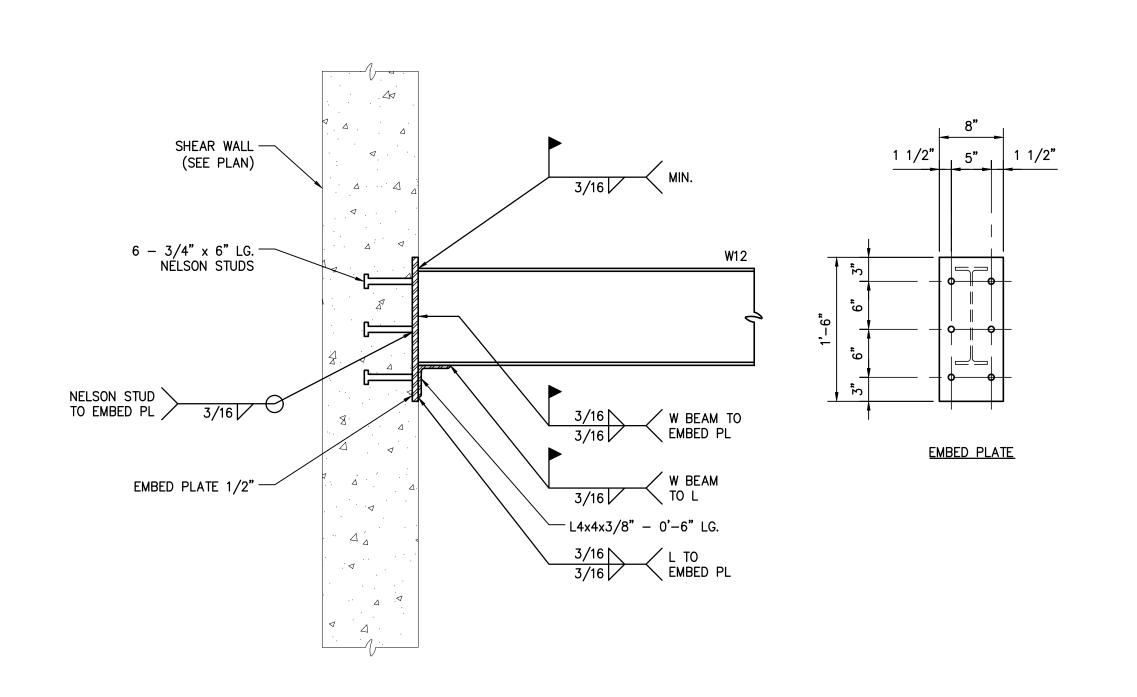
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SCALE: 1" = 1'-0"

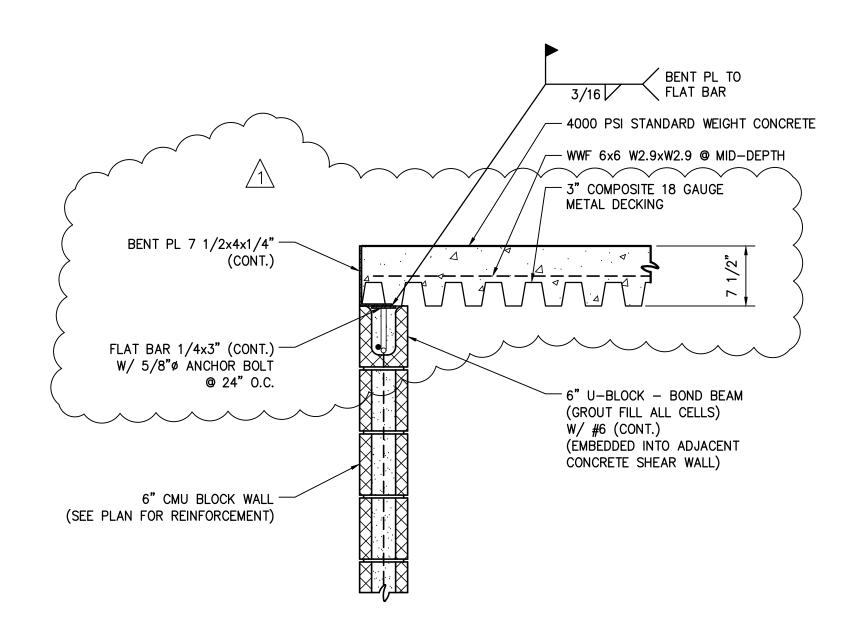


TYPICAL W12x19 TO 6" CMU BLOCK WALL CONNECTION DETAIL SCALE: 1" = 1'-0"

ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

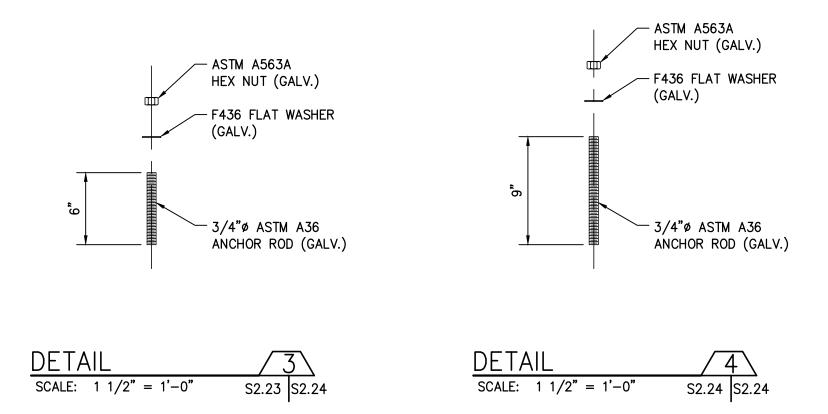


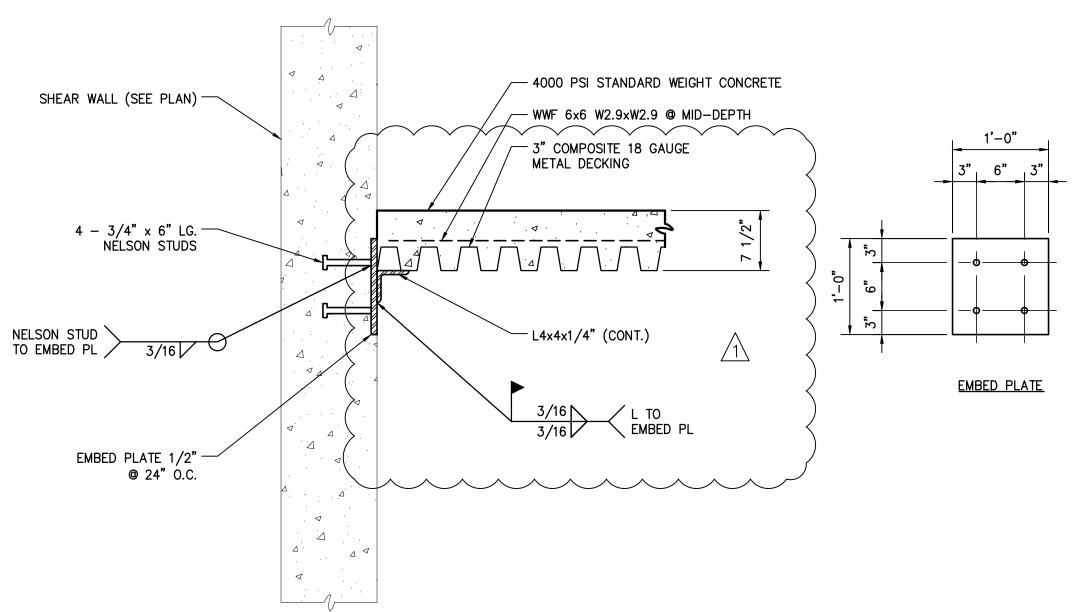
# TYPICAL W12x19 DIVIDER BEAM TO EMBED PLATE CONNECTION DETAIL @ SHEAR WALL SCALE: 1" = 1'-0"



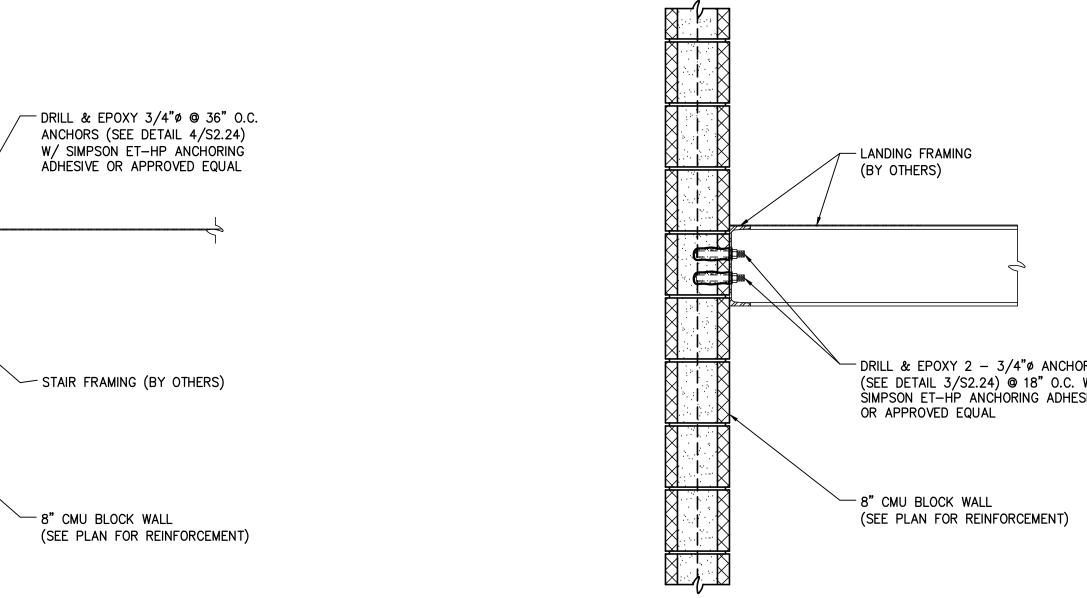
(6" CMU BLOCK WALL)

### TYPICAL SLAB IN-FILL @ 2ND FLOOR THRU 7TH FLOOR SCALE: 1" = 1'-0"



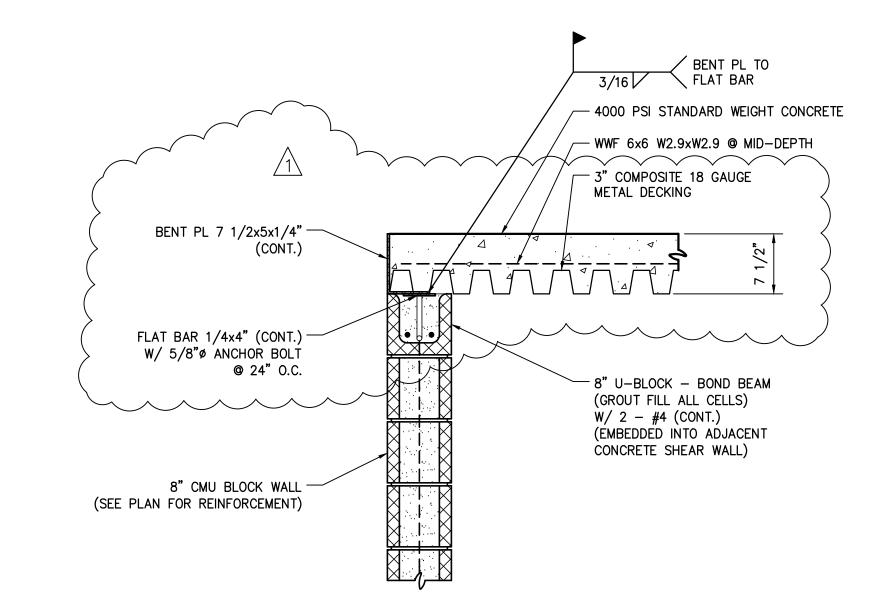


# TYPICAL SLAB IN-FILL @ GROUND FLOOR - 7TH FLOOR SCALE: 1'' = 1'-0''



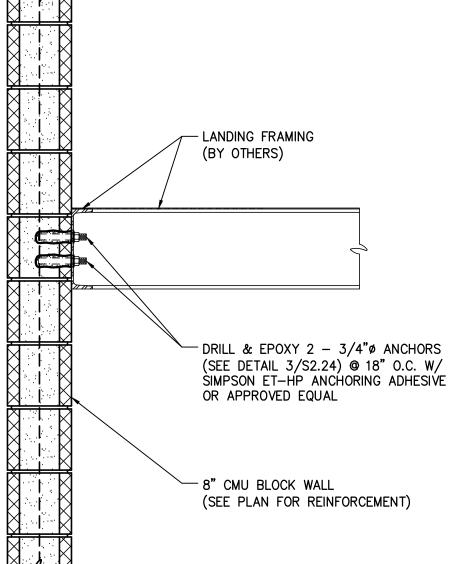
TYPICAL STAIR STRINGER TO 8" CMU BLOCK WALL CONNECTION DETAIL SCALE: 1" = 1'-0"





(8" CMU BLOCK WALL)

TYPICAL SLAB IN-FILL @ 2ND FLOOR THRU 7TH FLOOR SCALE: 1" = 1'-0"





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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593

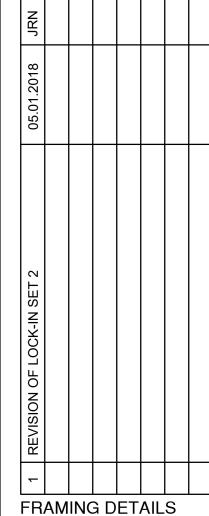
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> STREET, LOUISIANA 1031 CANAL NEW ORLEANS,

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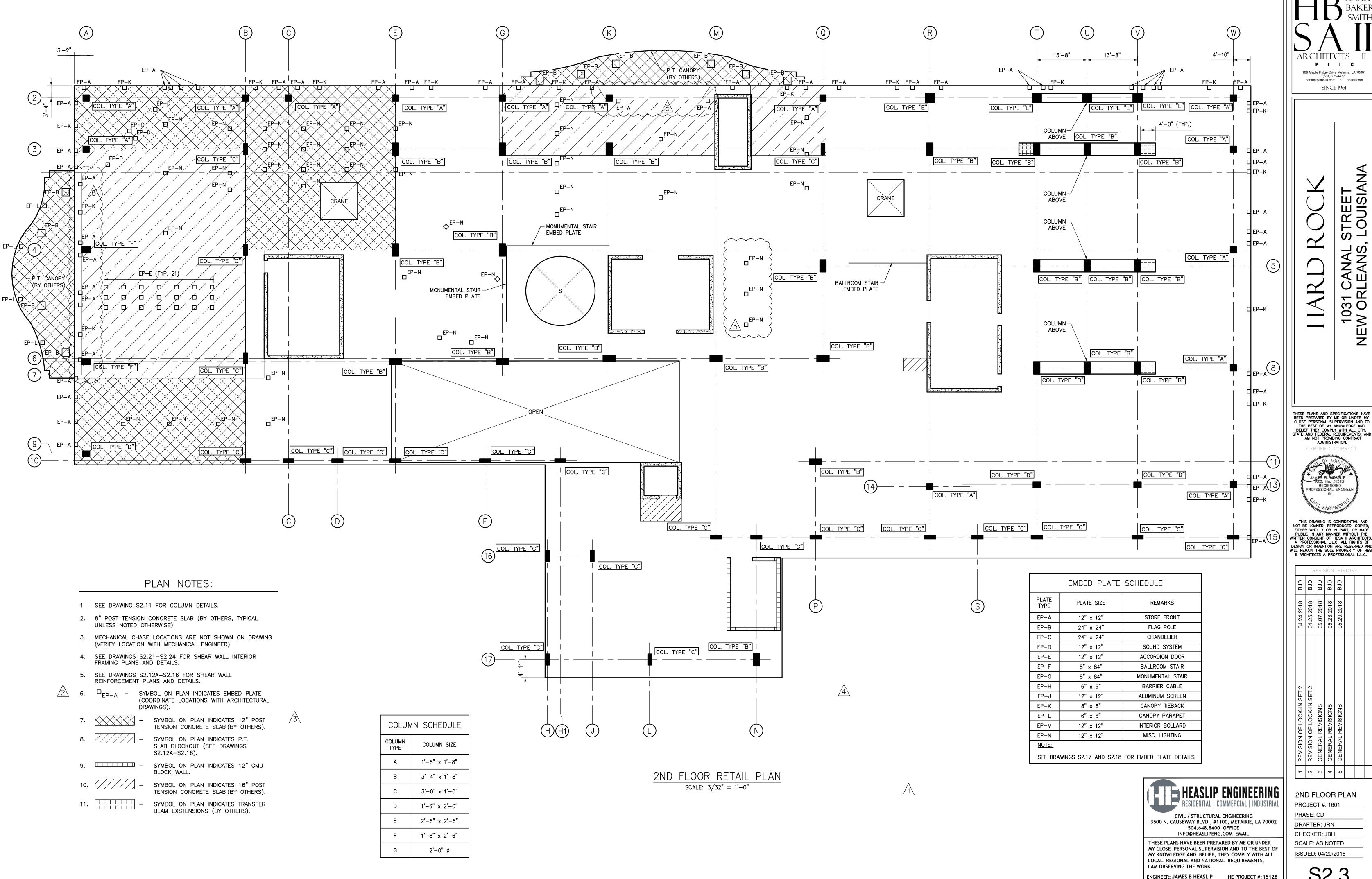
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PROJECT #: 1601 PHASE: CD

DRAFTER: JRN CHECKER: JBH SCALE: AS NOTED

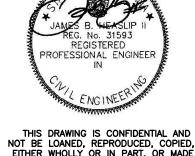
ISSUED: 04/20/2018 S2.24



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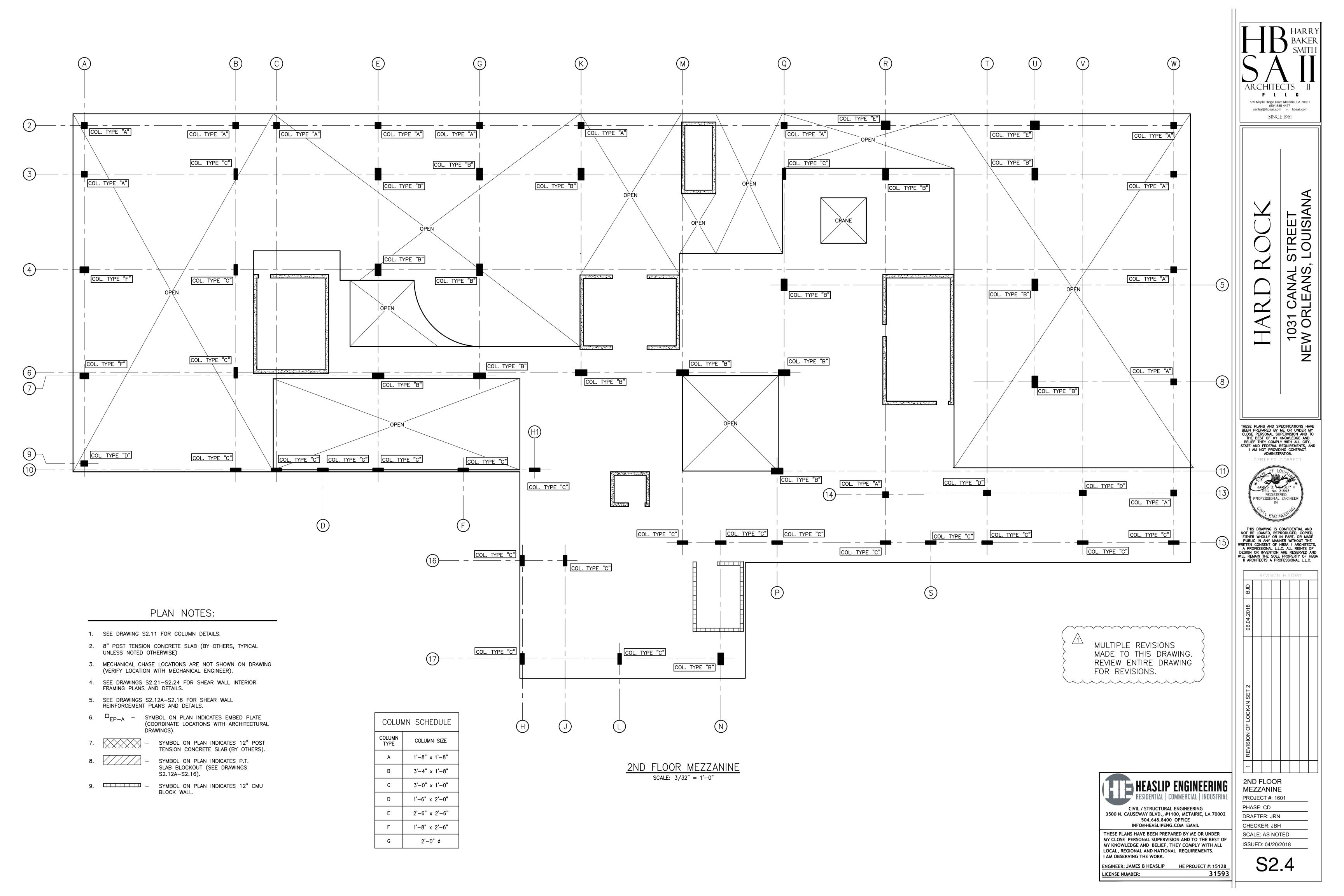


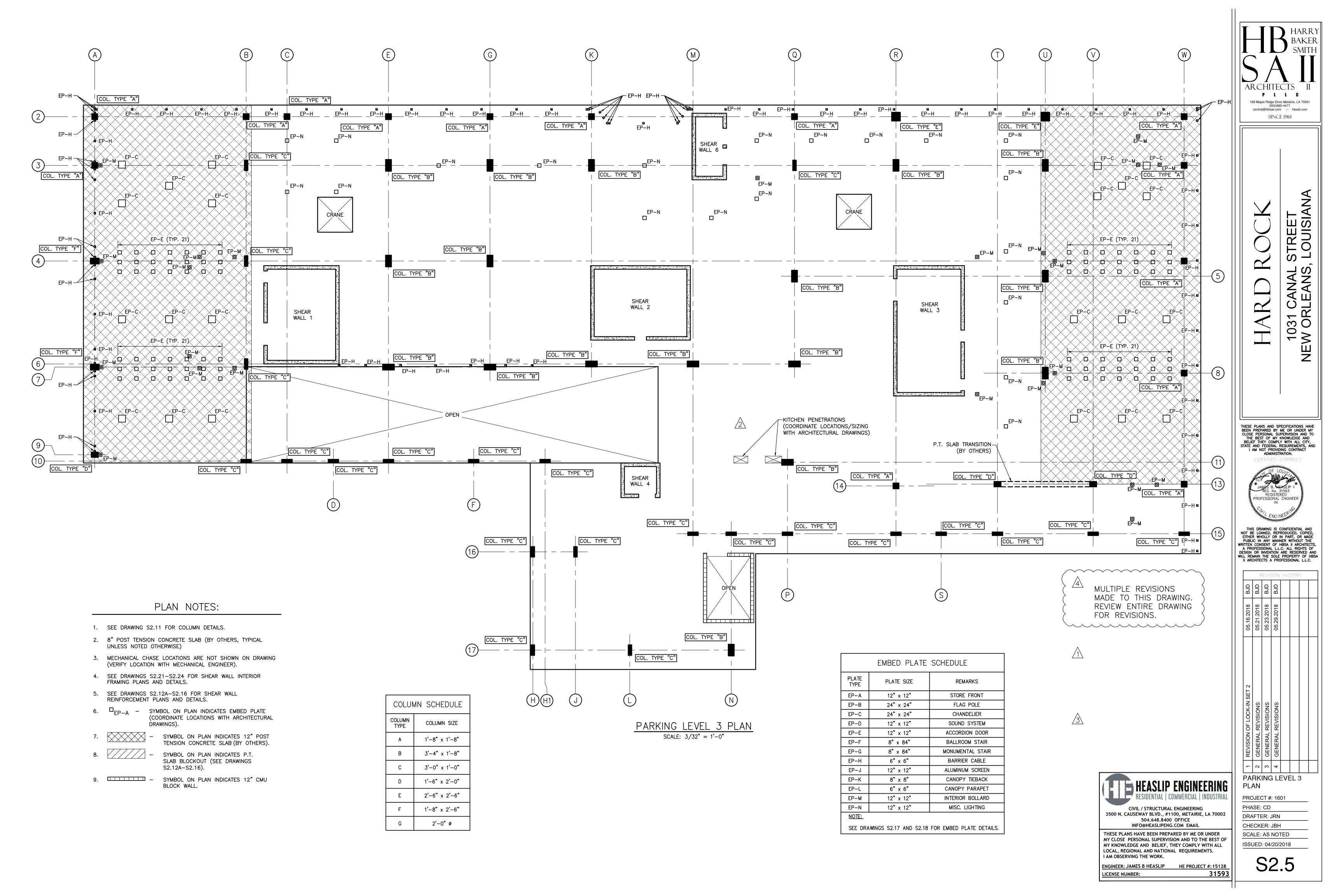
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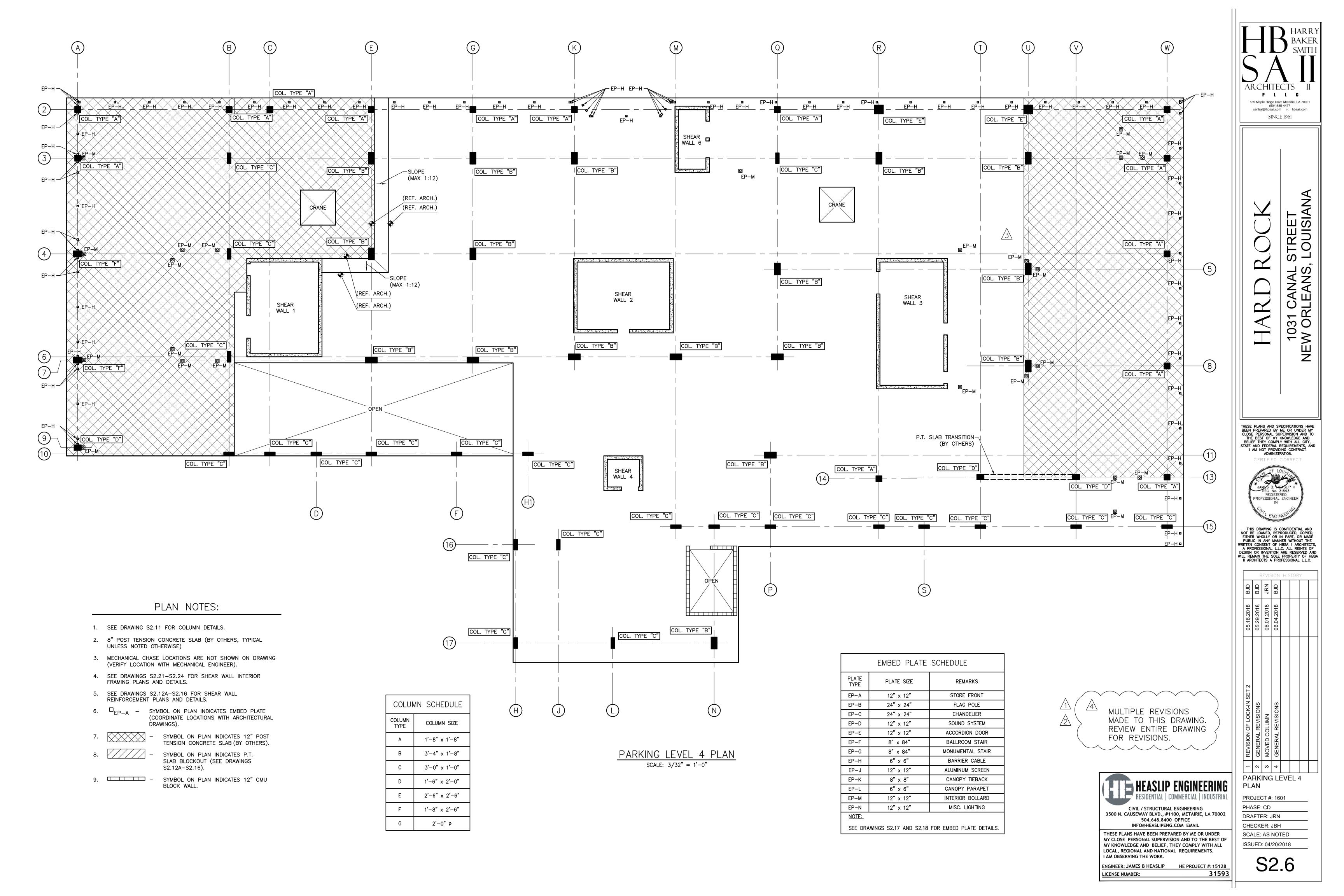
|                           | F                         | EVIS              | SION              | HIS               | STOR | Υ |  |
|---------------------------|---------------------------|-------------------|-------------------|-------------------|------|---|--|
| BJD                       | BJD                       | BJD               | BJD               | BJD               |      |   |  |
| 04.24.2018                | 04.25.2018                | 05.07.2018        | 05.23.2018        | 05.29.2018        |      |   |  |
| REVISION OF LOCK-IN SET 2 | REVISION OF LOCK-IN SET 2 | GENERAL REVISIONS | GENERAL REVISIONS | GENERAL REVISIONS |      |   |  |
| _                         | 2                         | 3                 | 4                 | 2                 |      |   |  |
|                           |                           |                   |                   |                   |      |   |  |

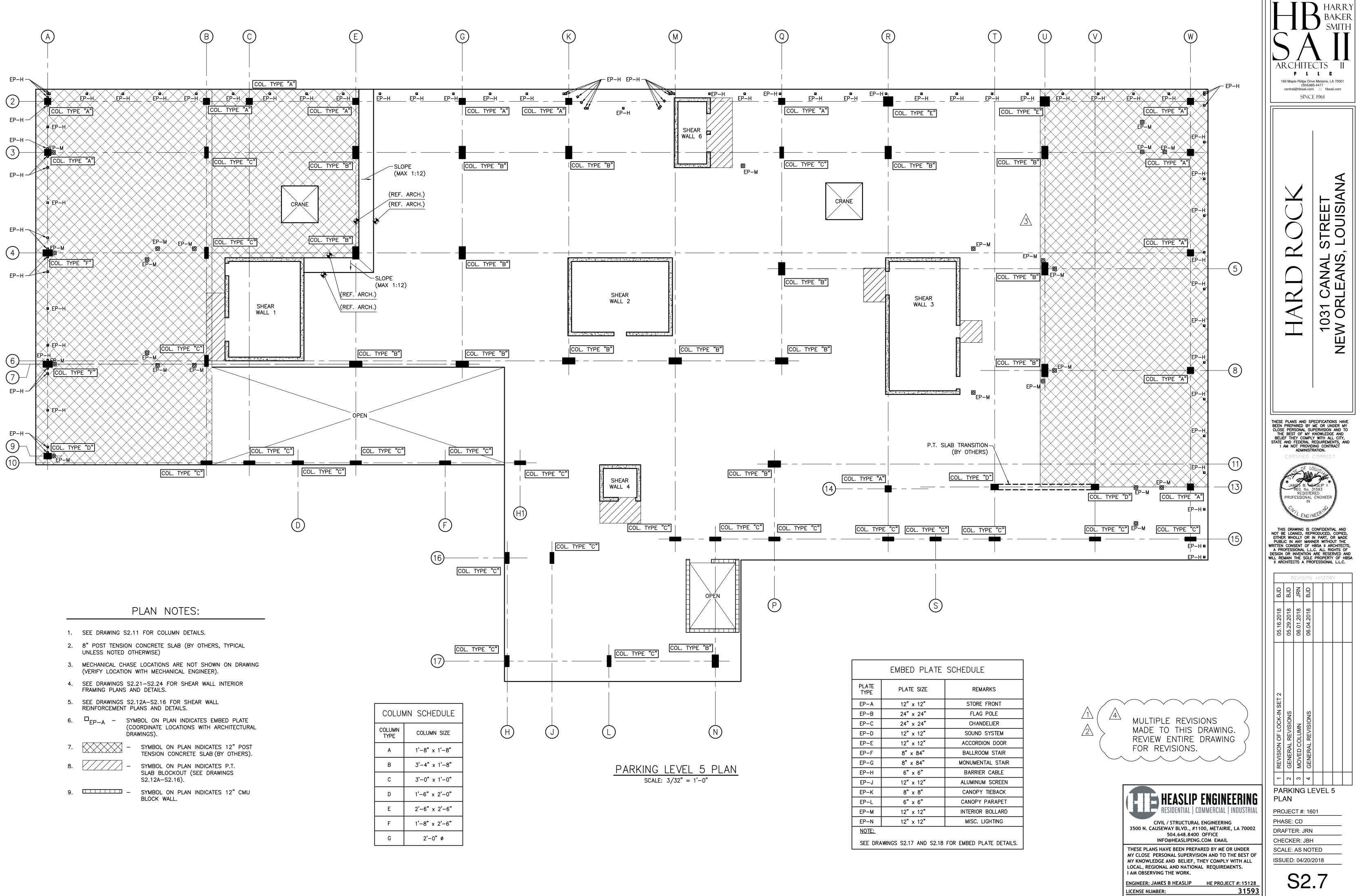
2ND FLOOR PLAN

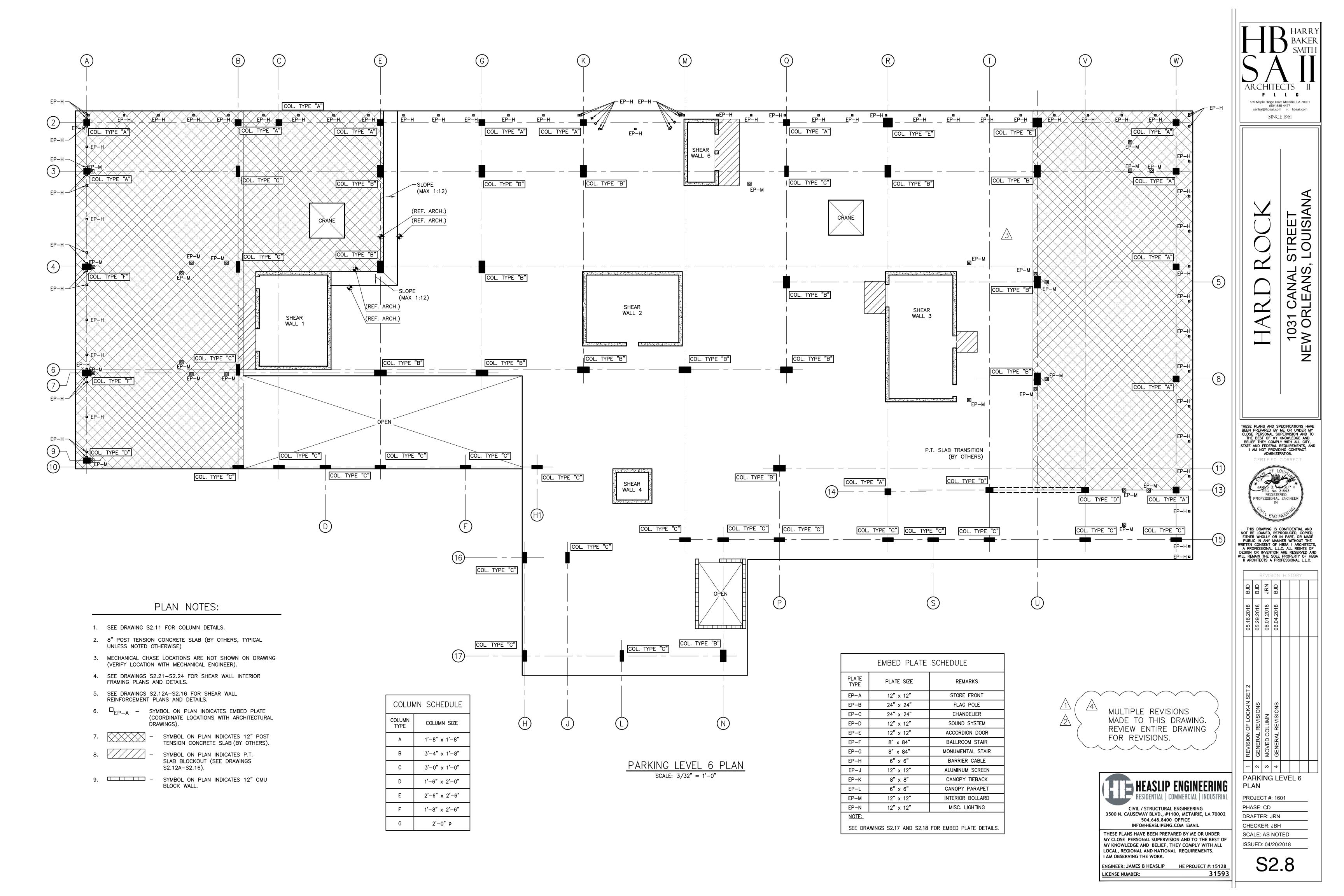
31593

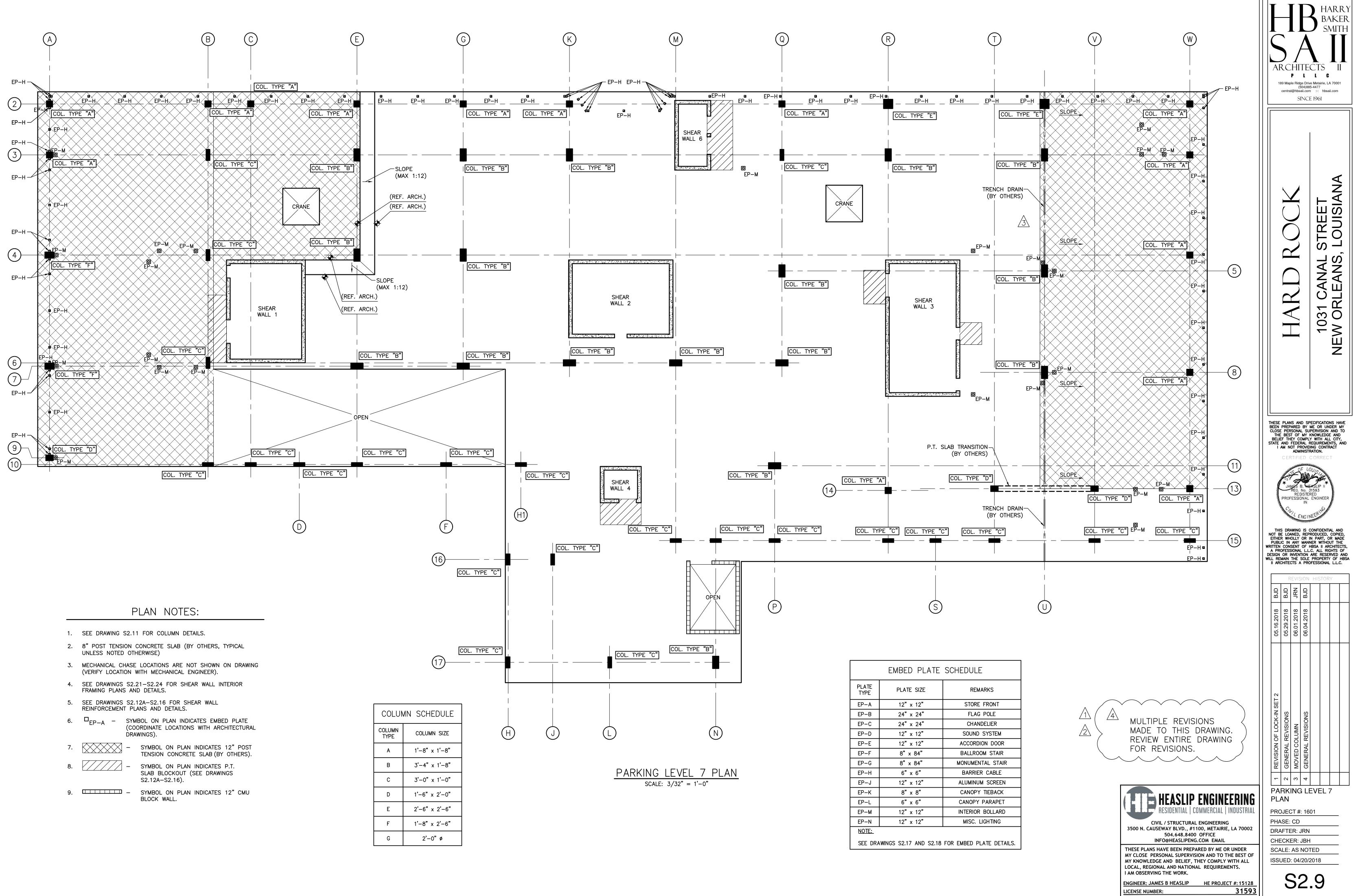


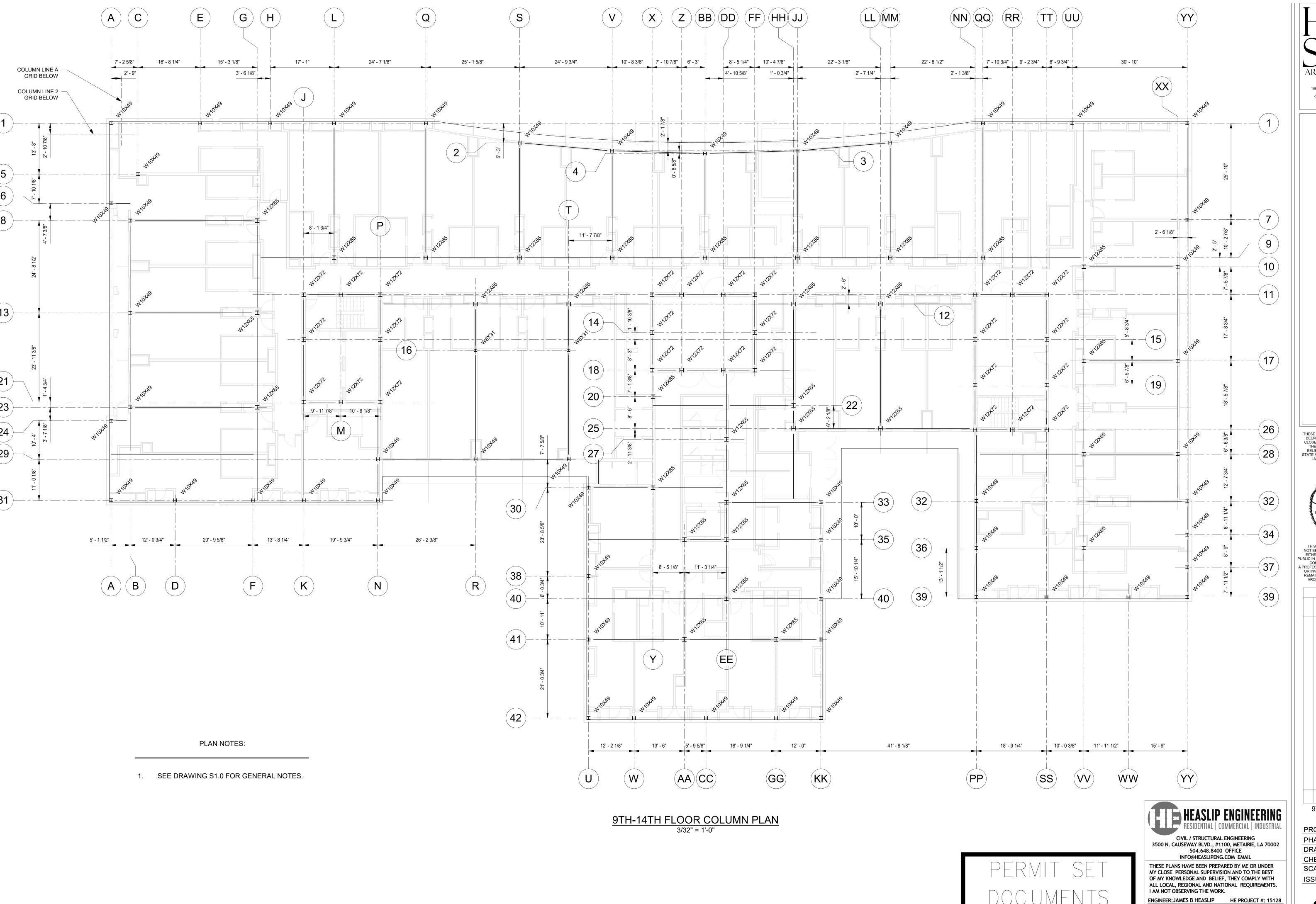












HARRY
BAKER
SMITH
SMITH
ARCHITECTS II
P L C

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central@hbsaii.com :: hbsaii.com
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HARD ROCK

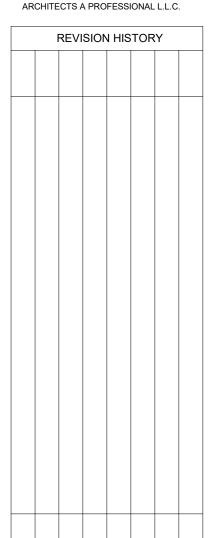
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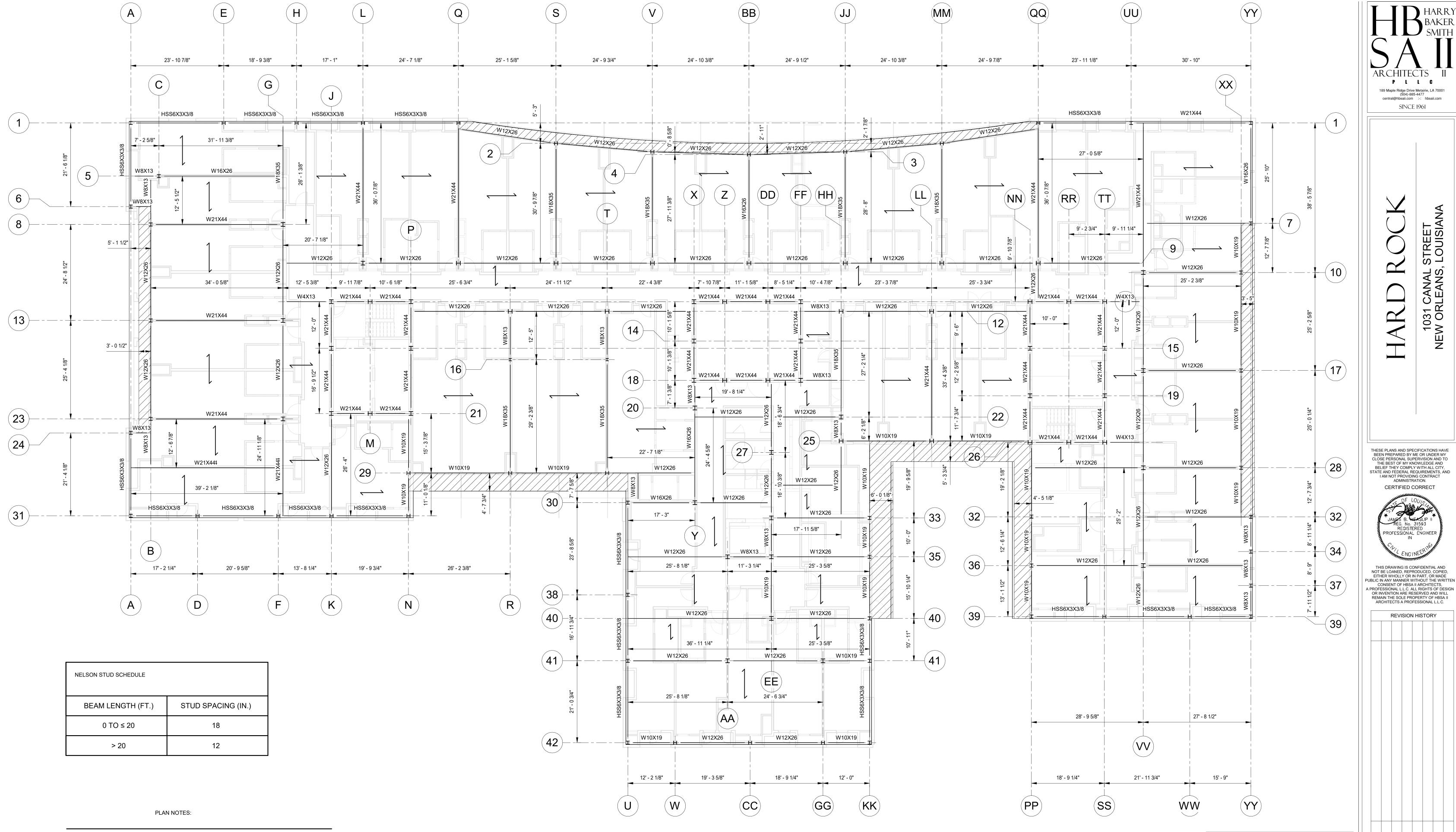


9TH-14TH FLOOR COLUMN PLAN

PROJECT#: 1601
PHASE: PERMIT
DRAFTER: BJD
CHECKER: JBH
SCALE: AS NOTED
ISSUED: 06/08/18

SHEET#:

LICENSE NUMBER:



1. SEE DRAWING S1.0 FOR GENERAL NOTES.

SYMBOL ON PLAN INDICATES 1/4" BENT PLATE.

SYMBOL ON PLAN INDICATES DECK DIRECTION.

9TH-13TH FLOOR FRAMING PLAN 3/32" = 1'-0"





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HEASLIP ENGINEERING

9th-13th FLOOR FRAMING

PLAN PROJECT#: 1601 PHASE: PERMIT DRAFTER: BJD CHECKER: JBH SCALE: AS NOTED ISSUED: 06/08/18

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SINCE 1961

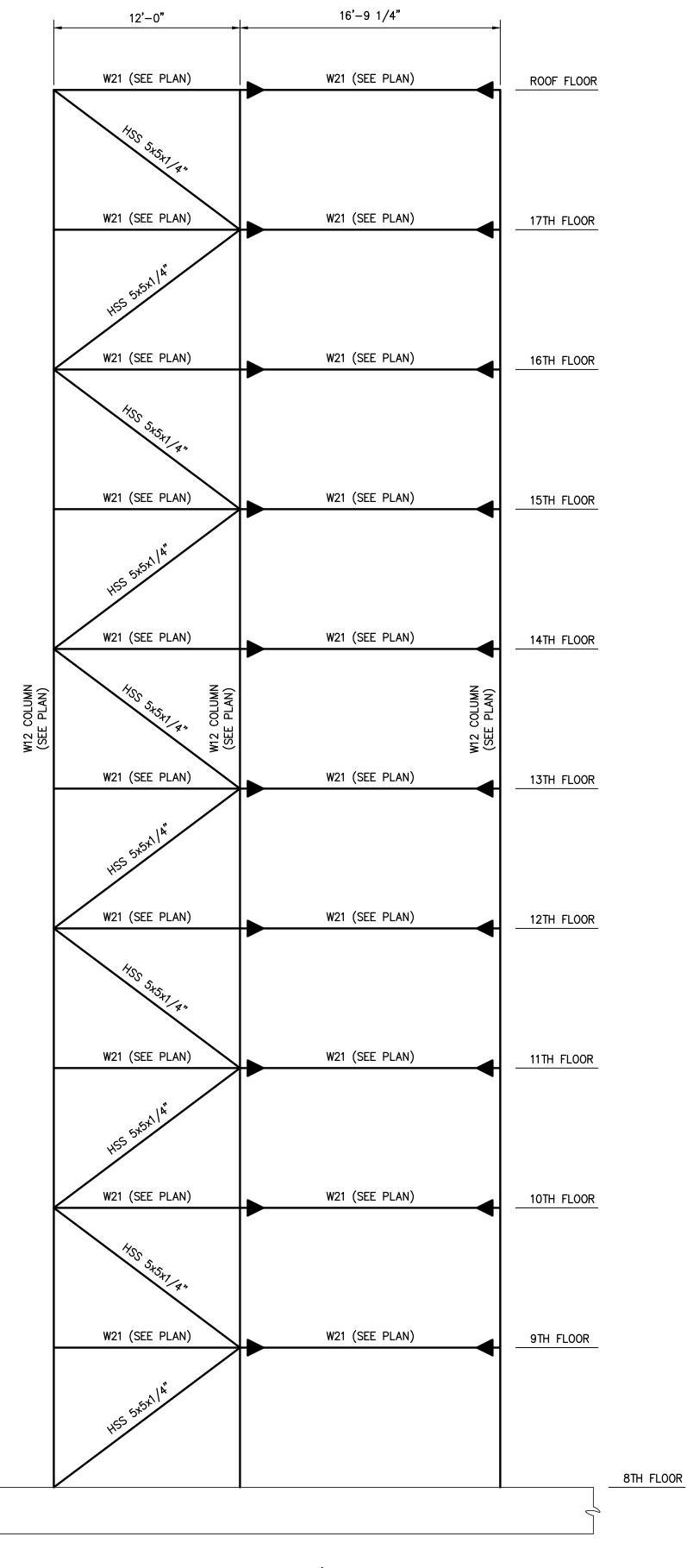
1031 CANAL STREET NEW ORLEANS, LOUISIANA

ADMINISTRATION. CERTIFIED CORRECT

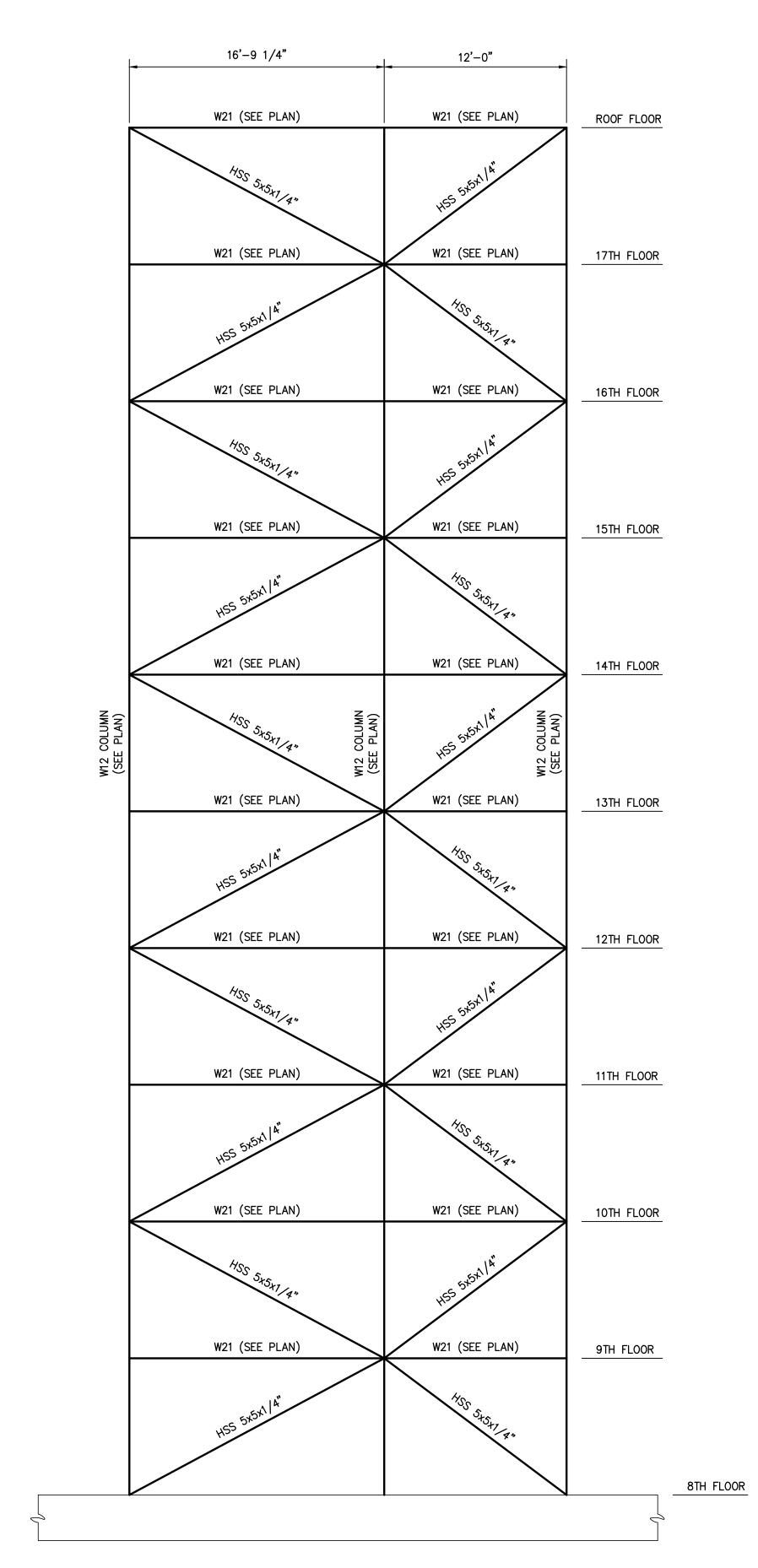
PROFESSIONAL ENGINEER

REVISION HISTORY

SHEET#:

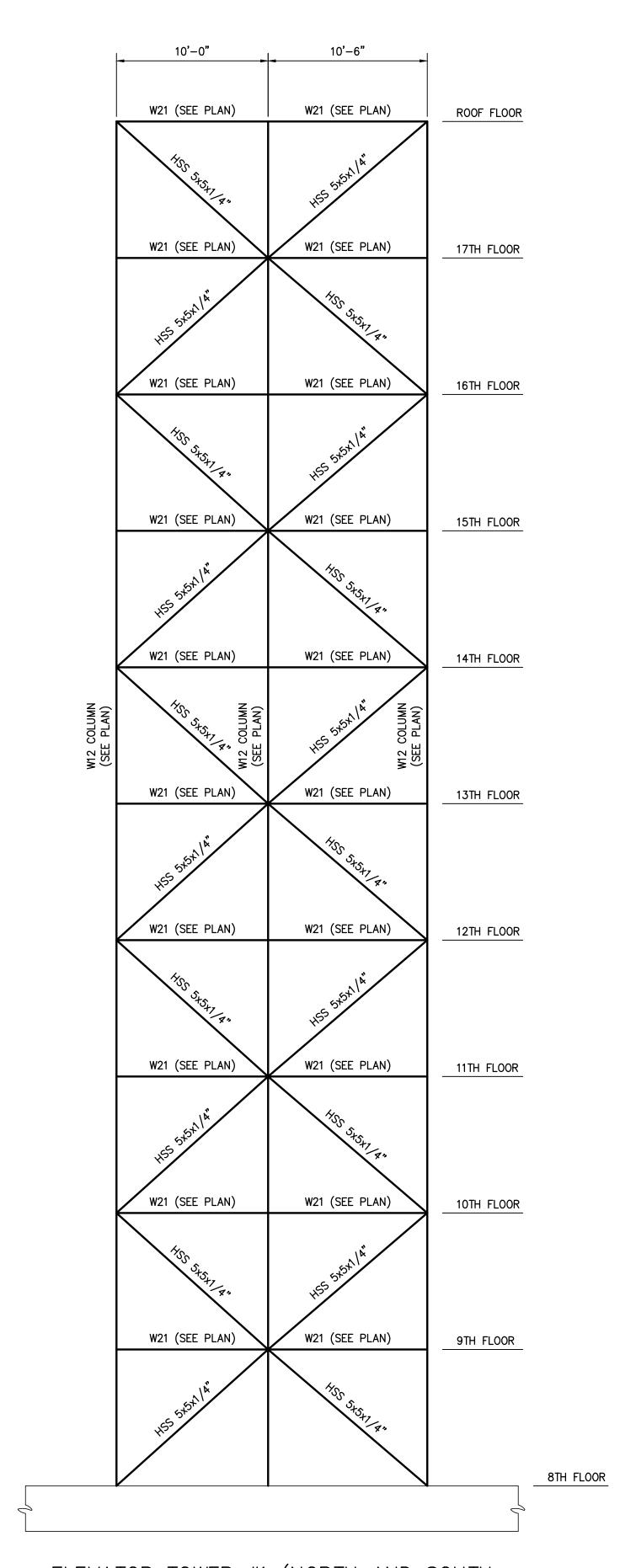


ELEVATOR TOWER #1 (WEST WALL -LOOKING EAST) STEEL FRAMING ELEVATION SCALE: 3/16" = 1'-0"



ELEVATOR TOWER #1 (EAST WALL -LOOKING WEST) STEEL FRAMING ELEVATION SCALE: 3/16" = 1'-0"

## PLAN NOTES:



ELEVATOR TOWER #1 (NORTH AND SOUTH WALL - LOOKING NORTH) STEEL FRAMING ELEVATION SCALE: 3/16" = 1'-0"

PERMIT SET DOCUMENTS



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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593 S3.10

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**ELEVATIONS** 

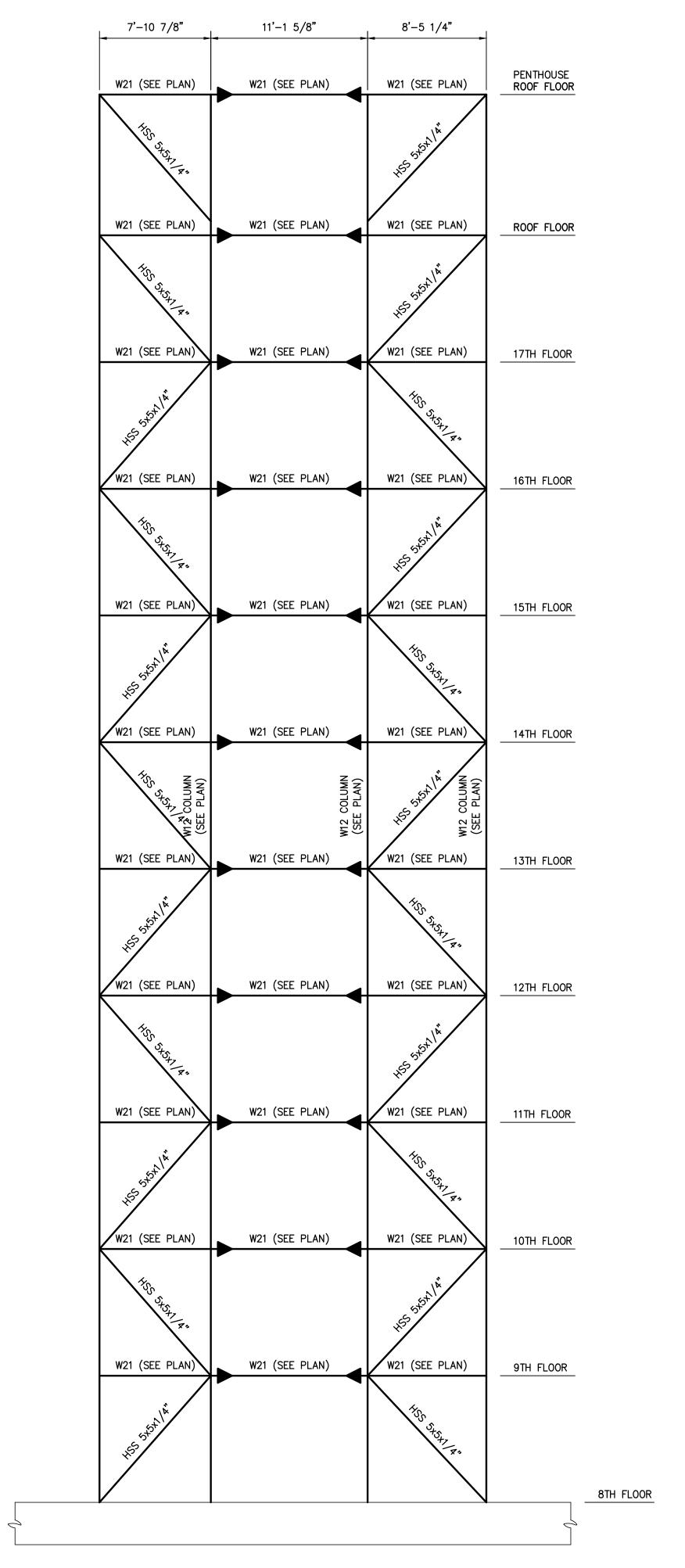
PROJECT #: 1601 PHASE: PERMIT DRAFTER: JRN

CHECKER: JBH SCALE: AS NOTED ISSUED: 04/20/2018

<sup>1.</sup> FOR ALL NOTES, SEE DRAWING S3.0.

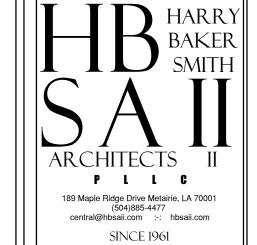
<sup>2.</sup>SYMBOL ON PLAN INDICATES MOMENT CONNECTION.

ELEVATOR TOWER #2 (EAST AND WEST WALL - LOOKING EAST) STEEL FRAMING ELEVATION SCALE: 3/16" = 1'-0"

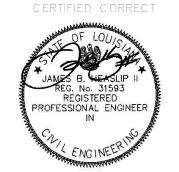


ELEVATOR TOWER #2 (NORTH AND SOUTH WALL - LOOKING NORTH) STEEL FRAMING ELEVATION SCALE: 3/16" = 1'-0"

- 1. FOR ALL NOTES, SEE DRAWING S1.0.
- 2. ▶ SYMBOL ON PLAN INDICATES MOMENT CONNECTION.



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**ELEVATIONS** 

PROJECT #: 1601 DRAFTER: JRN

CIVIL / STRUCTURAL ENGINEERING 3500 N. CAUSEWAY BLVD., #1100, METAIRIE, LA 70002 CHECKER: JBH SCALE: AS NOTED ISSUED: 04/20/2018

S3.11

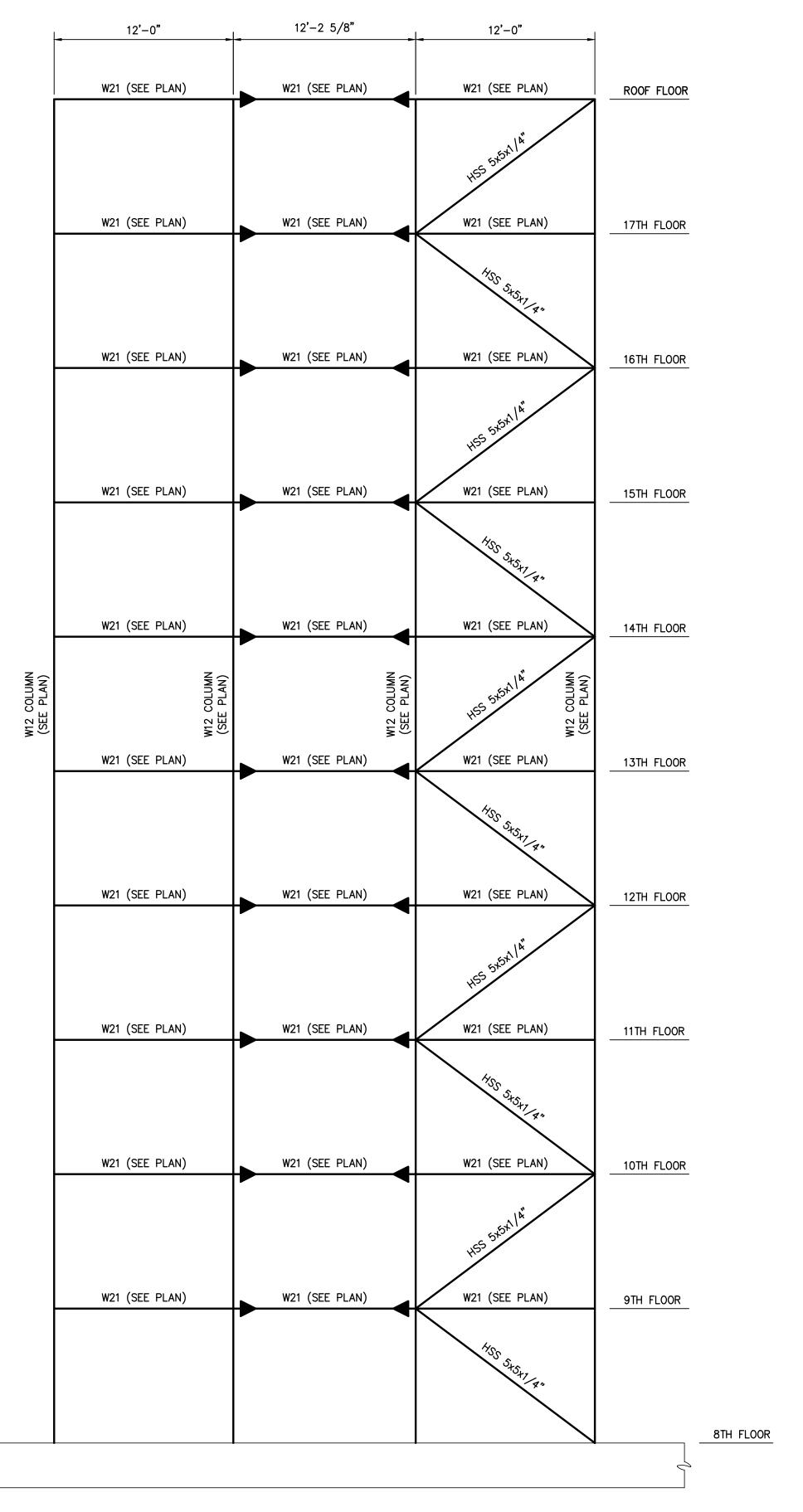
PERMIT SET DOCUMENTS

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ELEVATOR TOWER #3 (EAST WALL -LOOKING WEST) STEEL FRAMING ELEVATION

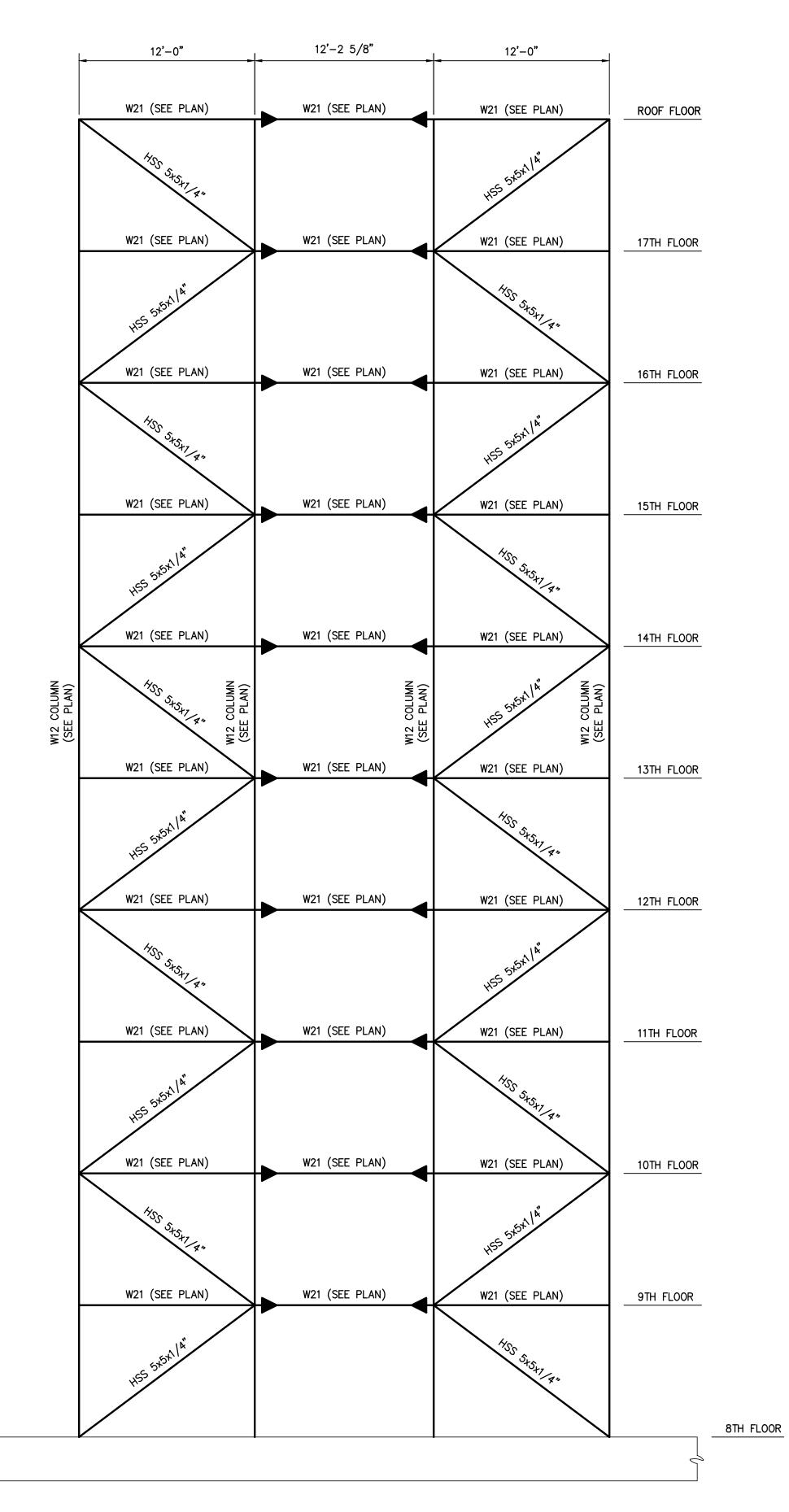
SCALE: 3/16" = 1'-0"

SCALE: 3/16" = 1'-0"

PLAN NOTES:

1. FOR ALL NOTES, SEE DRAWING S1.0.

2. - SYMBOL ON PLAN INDICATES MOMENT CONNECTION.



ELEVATOR TOWER #3 (WEST WALL -LOOKING EAST) STEEL FRAMING ELEVATION

W21 (SEE PLAN) W21 (SEE PLAN) ROOF FLOOR W21 (SEE PLAN) W21 (SEE PLAN) 17TH FLOOR W21 (SEE PLAN) W21 (SEE PLAN) 16TH FLOOR W21 (SEE PLAN) W21 (SEE PLAN) 15TH FLOOR W21 (SEE PLAN) W21 (SEE PLAN) W21 (SEE PLAN) W21 (SEE PLAN) 13TH FLOOR W21 (SEE PLAN) W21 (SEE PLAN) W21 (SEE PLAN) W21 (SEE PLAN) 11TH FLOOR W21 (SEE PLAN) W21 (SEE PLAN) 10TH FLOOR W21 (SEE PLAN) W21 (SEE PLAN) 9TH FLOOR 8TH FLOOR

9'-2 3/4"

10'-0"

ELEVATOR TOWER #3 (NORTH AND SOUTH WALL - LOOKING NORTH) STEEL FRAMING ELEVATION

SCALE: 3/16" = 1'-0"





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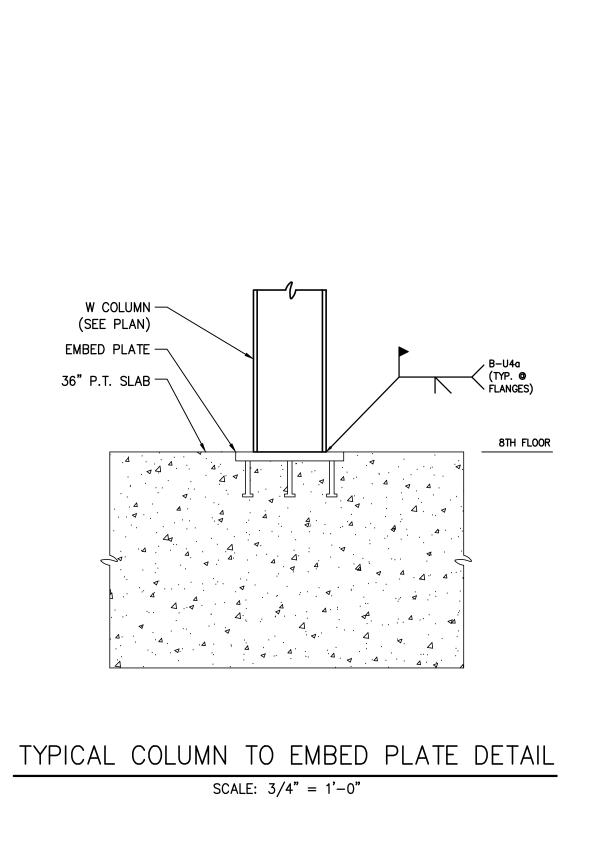
| F | REVIS | SION | HIS | STOR | Υ |  |
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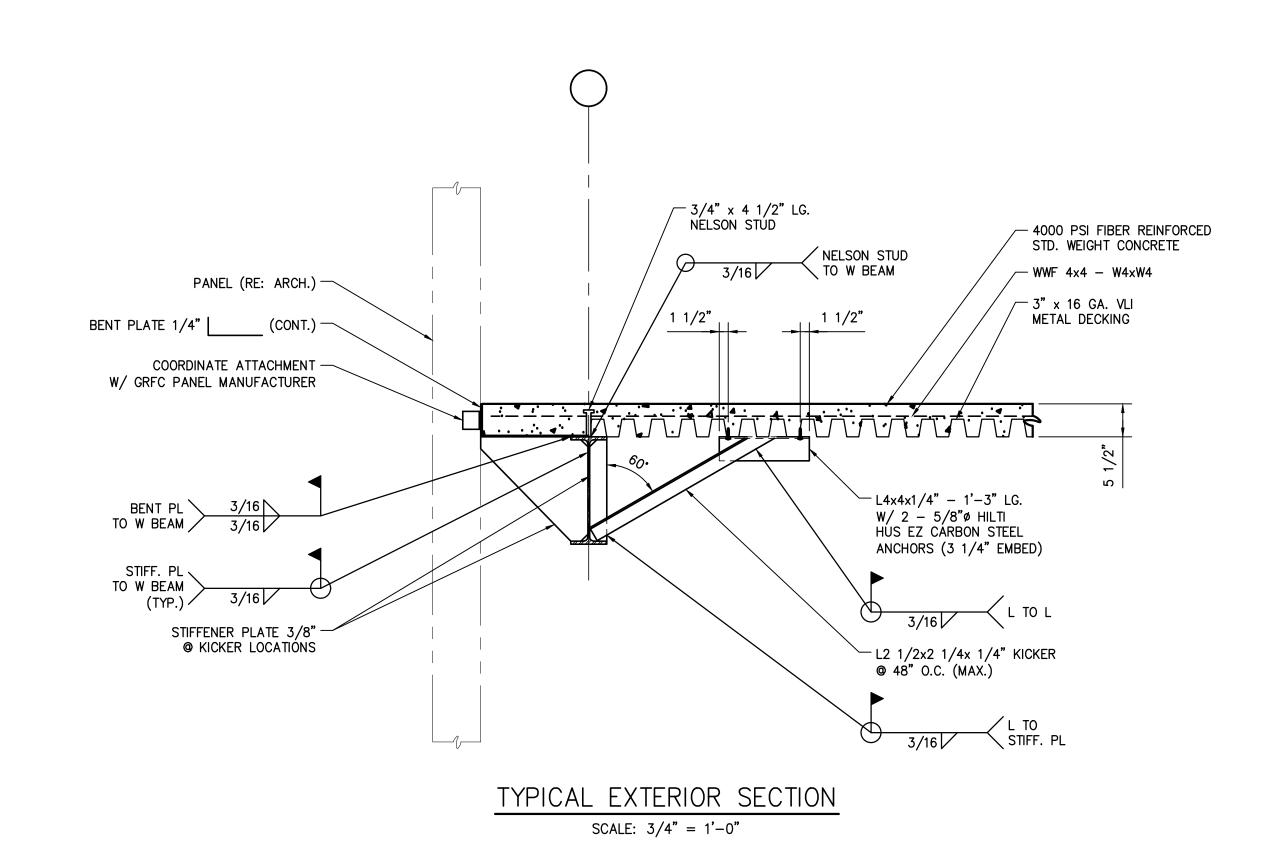
**ELEVATIONS** 

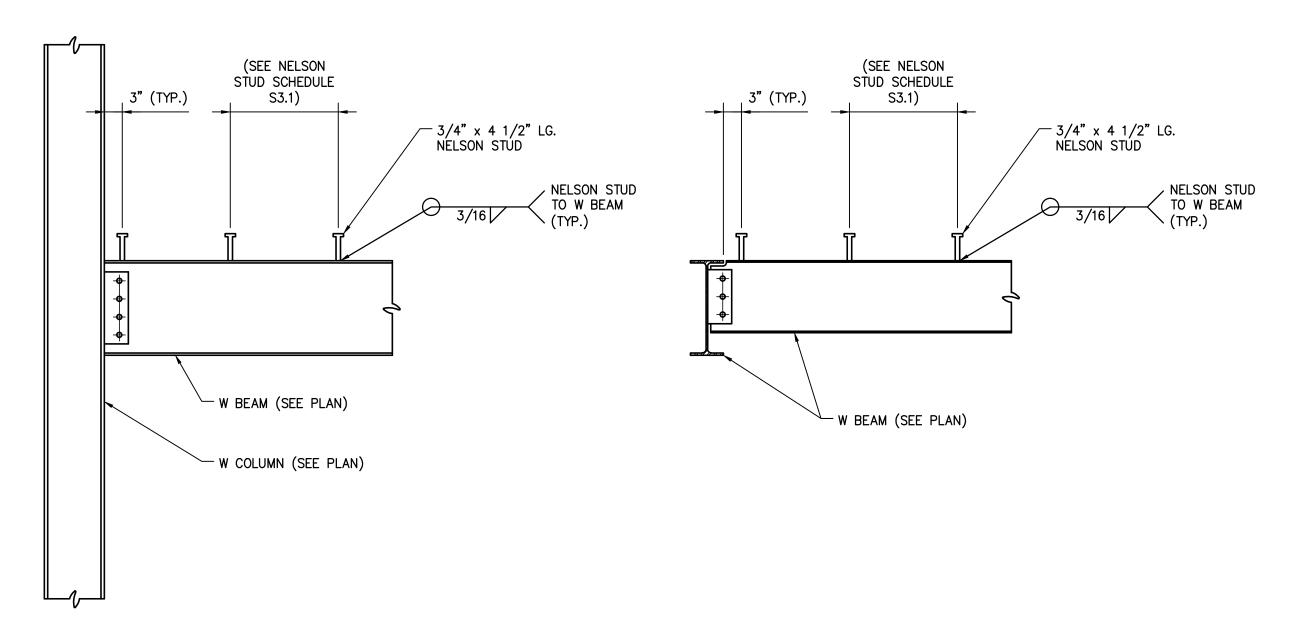
PROJECT #: 1601 PHASE: PERMIT

DRAFTER: JRN CHECKER: JBH SCALE: AS NOTED

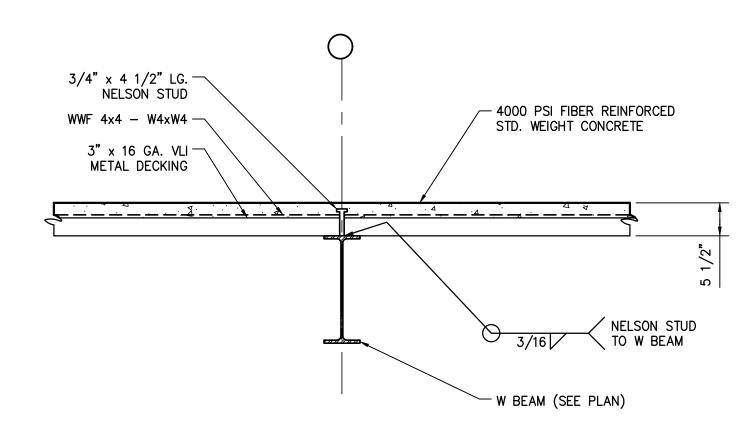
ISSUED: 04/20/2018 S3.12





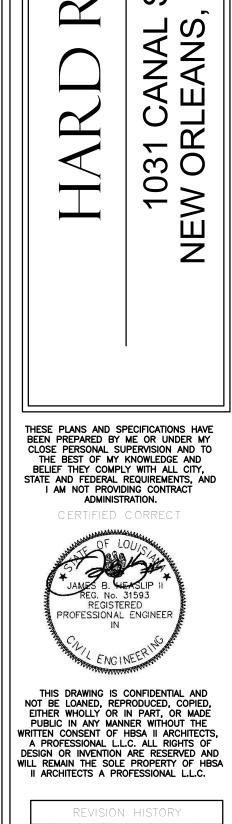


TYPICAL NELSON STUD LAYOUT DETAIL SCALE: 3/4" = 1'-0"



TYPICAL INTERIOR SECTION SCALE: 3/4" = 1'-0"

PERMIT SET DOCUMENTS

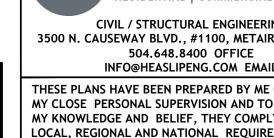


P L L C

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593 ICENSE NUMBER:

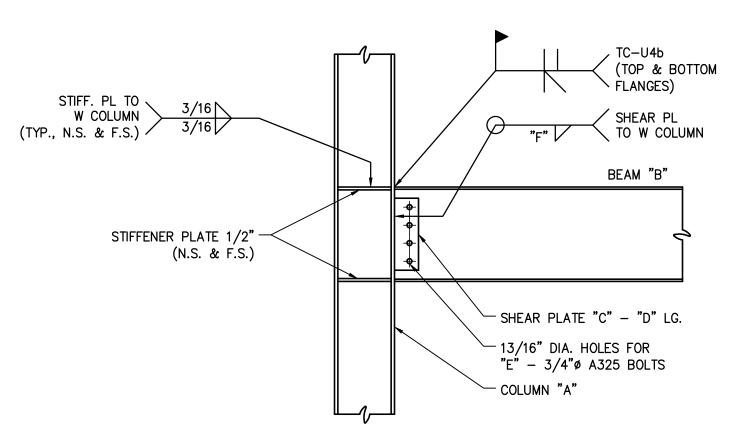
HEASLIP ENGINEERING CIVIL / STRUCTURAL ENGINEERING 3500 N. CAUSEWAY BLVD., #1100, METAIRIE, LA 70002 INFO@HEASLIPENG.COM EMAIL

DRAFTER: JRN CHECKER: JBH SCALE: AS NOTED ISSUED: 04/20/2018

**DETAILS** 

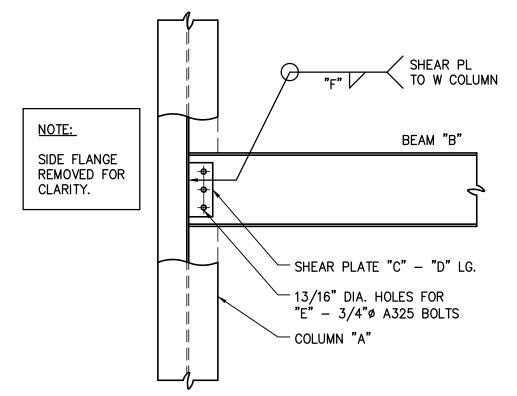
PROJECT #: 1601 PHASE: PERMIT

S3.13



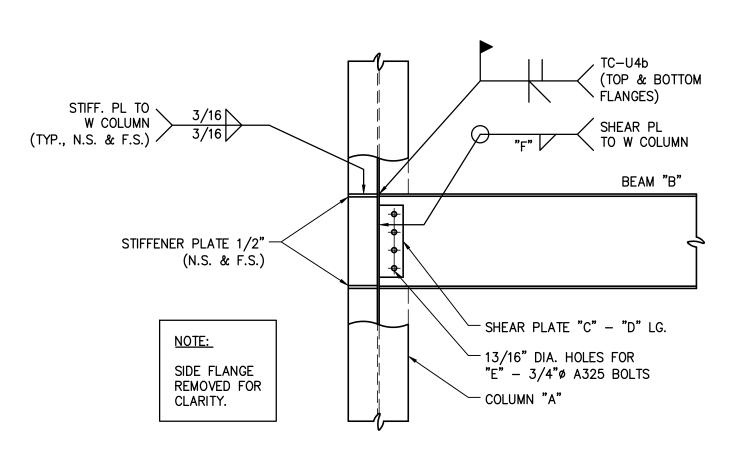
| COLUMN<br>"A" | BEAM<br>"B" | SHEAR PL<br>"C" | SHEAR PL<br>LENGTH<br>"D" | NUMBER<br>OF BOLTS<br>"E" | WELD<br>"F" |
|---------------|-------------|-----------------|---------------------------|---------------------------|-------------|
| W10x49        | W12x26      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W10x49        | W14x22      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W10x49        | W16x26      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W10x49        | W18x35      | 1/2 × 4         | 1'-0"                     | 4                         | 3/16        |
| W10x49        | W21x44      | 1/2 x 4         | 1'-3"                     | 5                         | 3/16        |
| W10x49        | W24x55      | 1/2 x 4         | 1'-6"                     | 6                         | 3/16        |
| W12x65        | W12x26      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W12x65        | W14x22      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W12x65        | W16x26      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x65        | W18x35      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x65        | W21x44      | 1/2 x 4         | 1'-3"                     | 5                         | 3/16        |
| W12x65        | W24x55      | 1/2 x 4         | 1'-6"                     | 6                         | 3/16        |
| W12x72        | W12x26      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W12x72        | W14x22      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W12x72        | W16x26      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x72        | W18x35      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x72        | W21x44      | 1/2 x 4         | 1'-3"                     | 5                         | 3/16        |
| W12x72        | W24x55      | 1/2 x 4         | 1'-6"                     | 6                         | 3/16        |

# TYPICAL BEAM TO COLUMN FLANGE MOMENT CONNECTION DETAIL SCALE: 3/4" = 1'-0"



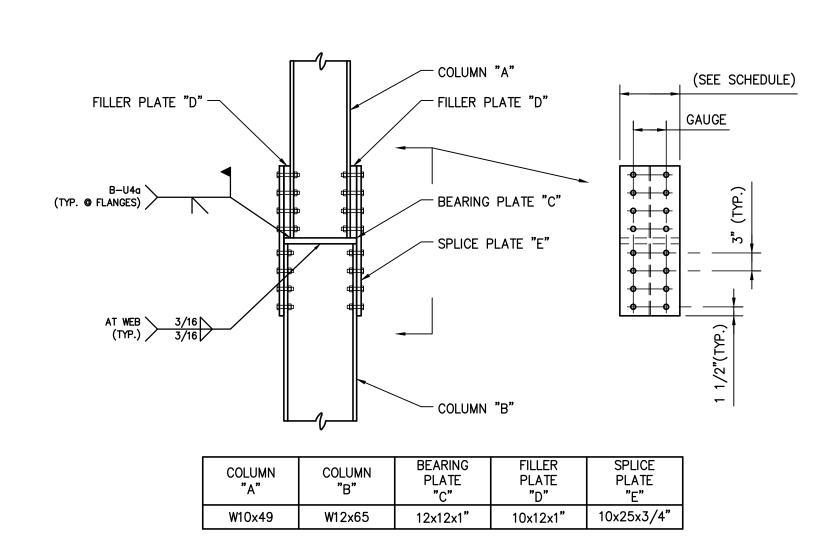
| COLUMN<br>"A" | BEAM<br>"B" | SHEAR PL<br>"C" | SHEAR PL<br>LENGTH<br>"D" | NUMBER<br>OF BOLTS<br>"E" | WELD<br>"F" |
|---------------|-------------|-----------------|---------------------------|---------------------------|-------------|
| W10x49        | W12x26      | 1/2 × 4         | 0'-9"                     | 3                         | 3/16        |
| W10x49        | W14x22      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W10x49        | W16x26      | 1/2 × 4         | 1'-0"                     | 4                         | 3/16        |
| W10x49        | W18x35      | 1/2 × 4         | 1'-0"                     | 4                         | 3/16        |
| W10x49        | W21x44      | 1/2 x 4         | 1'-3"                     | 5                         | 3/16        |
| W10x49        | W24x55      | 1/2 × 4         | 1'-6"                     | 6                         | 3/16        |
| W12x65        | W12x26      | 1/2 × 4         | 0'-9"                     | 3                         | 3/16        |
| W12x65        | W14x22      | 1/2 × 4         | 0'-9"                     | 3                         | 3/16        |
| W12x65        | W16x26      | 1/2 × 4         | 1'-0"                     | 4                         | 3/16        |
| W12x65        | W18x35      | 1/2 × 4         | 1'-0"                     | 4                         | 3/16        |
| W12x65        | W21x44      | 1/2 × 4         | 1'-3"                     | 5                         | 3/16        |
| W12x65        | W24x55      | 1/2 × 4         | 1'-6"                     | 6                         | 3/16        |
| W12x72        | W12x26      | 1/2 × 4         | 0'-9"                     | 3                         | 3/16        |
| W12x72        | W14x22      | 1/2 × 4         | 0'-9"                     | 3                         | 3/16        |
| W12x72        | W16x26      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x72        | W18x35      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x72        | W21x44      | 1/2 x 4         | 1'-3"                     | 5                         | 3/16        |
| W12x72        | W24x55      | 1/2 x 4         | 1'-6"                     | 6                         | 3/16        |

TYPICAL BEAM TO COLUMN (WEB) CONNECTION DETAIL SCALE: 3/4" = 1'-0"

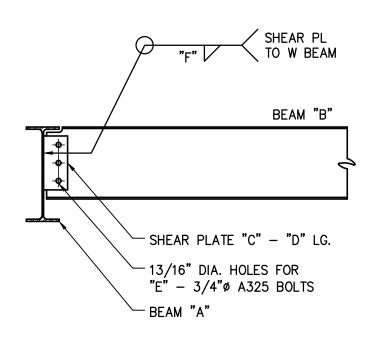


| COLUMN<br>"A" | BEAM<br>"B" | SHEAR PL<br>"C" | SHEAR PL<br>LENGTH<br>"D" | NUMBER<br>OF BOLTS<br>"E" | WELD<br>"F" |
|---------------|-------------|-----------------|---------------------------|---------------------------|-------------|
| W10×49        | W12x26      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W10x49        | W14x22      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W10x49        | W16x26      | 1/2 × 4         | 1'-0"                     | 4                         | 3/16        |
| W10x49        | W18x35      | 1/2 × 4         | 1'-0"                     | 4                         | 3/16        |
| W10x49        | W21x44      | 1/2 × 4         | 1'-3"                     | 5                         | 3/16        |
| W10x49        | W24x55      | 1/2 x 4         | 1'-6"                     | 6                         | 3/16        |
| W12x65        | W12x26      | 1/2 × 4         | 0'-9"                     | 3                         | 3/16        |
| W12x65        | W14x22      | 1/2 × 4         | 0'-9"                     | 3                         | 3/16        |
| W12x65        | W16x26      | 1/2 × 4         | 1'-0"                     | 4                         | 3/16        |
| W12x65        | W18x35      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x65        | W21x44      | 1/2 x 4         | 1'-3"                     | 5                         | 3/16        |
| W12x65        | W24x55      | 1/2 x 4         | 1'-6"                     | 6                         | 3/16        |
| W12x72        | W12x26      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W12x72        | W14x22      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W12x72        | W16x26      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x72        | W18x35      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x72        | W21x44      | 1/2 x 4         | 1'-3"                     | 5                         | 3/16        |
| W12x72        | W24x55      | 1/2 x 4         | 1'-6"                     | 6                         | 3/16        |

# TYPICAL BEAM TO COLUMN WEB MOMENT CONNECTION DETAIL SCALE: 3/4" = 1'-0"

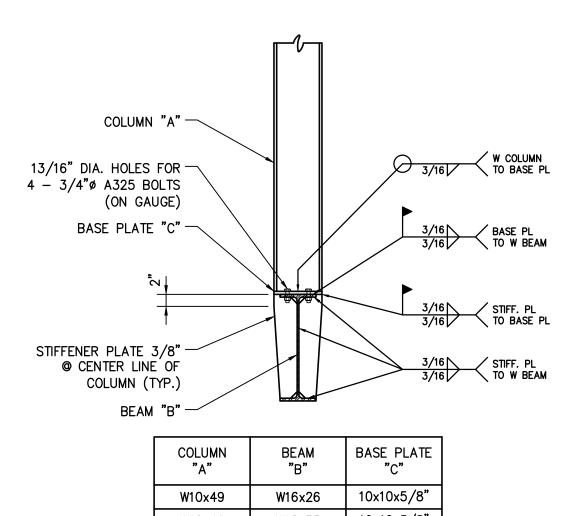


TYPICAL COLUMN SPLICE PLATE CONNECTION DETAIL SCALE: 3/4" = 1'-0"



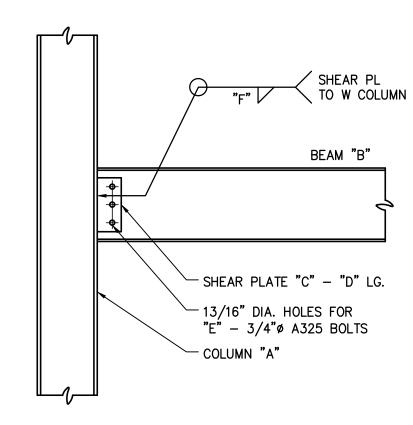
| BEAM<br>"A" | BEAM<br>"B" | SHEAR PL<br>"C" | L LENGTH<br>"C" | NUMBER<br>OF BOLTS<br>"E" | WELD<br>"F" |
|-------------|-------------|-----------------|-----------------|---------------------------|-------------|
| W12x26      | W12x26      | 1/2 x 4         | 0'-9"           | 3                         | 3/16        |
| W14×22      | W12x26      | 1/2 x 4         | 0'-9"           | 3                         | 3/16        |
| W14x22      | W14x22      | 1/2 x 4         | 0'-9"           | 3                         | 3/16        |
| W16x26      | W12x26      | 1/2 x 4         | 0'-9"           | 3                         | 3/16        |
| W16x26      | W14x22      | 1/2 x 4         | 0'-9"           | 3                         | 3/16        |
| W16x26      | W16x26      | 1/2 x 4         | 1'-0"           | 4                         | 3/16        |
| W18x35      | W12x26      | 1/2 x 4         | 0'-9"           | 3                         | 3/16        |
| W18x35      | W14x22      | 1/2 x 4         | 0'-9"           | 3                         | 3/16        |
| W18x35      | W16x26      | 1/2 x 4         | 1'-0"           | 4                         | 3/16        |
| W18x35      | W18x35      | 1/2 x 4         | 1'-0"           | 4                         | 3/16        |
| W21x44      | W12x26      | 1/2 x 4         | 0'-9"           | 3                         | 3/16        |
| W21x44      | W14x22      | 1/2 x 4         | 0'-9"           | 3                         | 3/16        |
| W21x44      | W16x26      | 1/2 x 4         | 1'-0"           | 4                         | 3/16        |
| W21x44      | W18x35      | 1/2 x 4         | 1'-0"           | 4                         | 3/16        |
| W21x44      | W21x44      | 1/2 x 4         | 1'-3"           | 5                         | 3/16        |
| W24x55      | W21x44      | 1/2 x 4         | 1'-3"           | 5                         | 3/16        |
| W24x55      | W24x55      | 1/2 x 4         | 1'-6"           | 6                         | 3/16        |

### TYPICAL BEAM TO BEAM CONNECTION DETAIL SCALE: 3/4" = 1'-0"



| COLUMN<br>"A" | BEAM<br>"B" | BASE PLATE<br>"C" |  |
|---------------|-------------|-------------------|--|
| W10x49        | W16x26      | 10x10x5/8"        |  |
| W10x49        | W18x35      | 10x10x5/8"        |  |
| W10x49        | W21x44      | 10x10x5/8"        |  |
| W10x49        | W24x55      | 10x10x5/8"        |  |

TYPICAL COLUMN TO BEAM CONNECTION DETAIL SCALE: 3/4" = 1'-0"



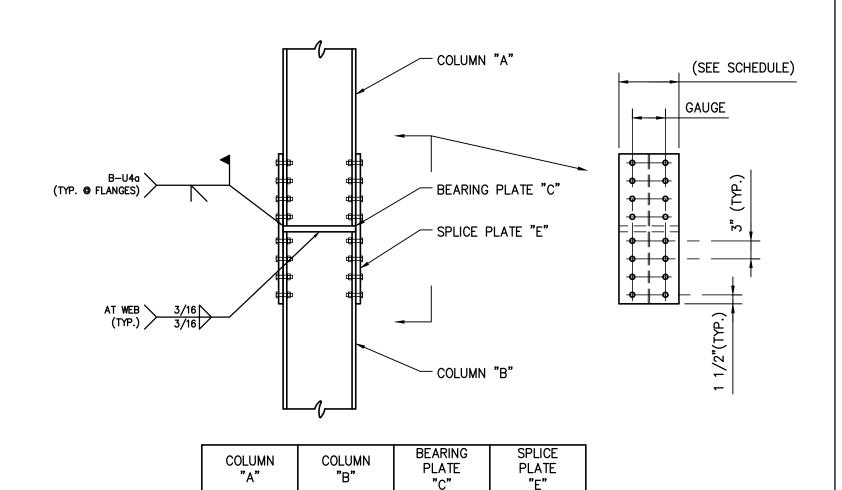
| COLUMN<br>"A" | BEAM<br>"B" | SHEAR PL<br>"C" | SHEAR PL<br>LENGTH<br>"D" | NUMBER<br>OF BOLTS<br>"E" | WELD<br>"F" |
|---------------|-------------|-----------------|---------------------------|---------------------------|-------------|
| W10x49        | W12x26      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W10x49        | W14x22      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W10x49        | W16x26      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W10x49        | W18x35      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W10x49        | W21×44      | 1/2 x 4         | 1'-3"                     | 5                         | 3/16        |
| W10x49        | W24x55      | 1/2 x 4         | 1'-6"                     | 6                         | 3/16        |
| W12x65        | W12x26      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W12x65        | W14x22      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W12x65        | W16x26      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x65        | W18x35      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x65        | W21x44      | 1/2 x 4         | 1'-3"                     | 5                         | 3/16        |
| W12x65        | W24x55      | 1/2 x 4         | 1'-6"                     | 6                         | 3/16        |
| W12x72        | W12x26      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W12x72        | W14x22      | 1/2 x 4         | 0'-9"                     | 3                         | 3/16        |
| W12x72        | W16x26      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x72        | W18x35      | 1/2 x 4         | 1'-0"                     | 4                         | 3/16        |
| W12x72        | W21x44      | 1/2 x 4         | 1'-3"                     | 5                         | 3/16        |
| W12x72        | W24x55      | 1/2 x 4         | 1'-6"                     | 6                         | 3/16        |

### TYPICAL BEAM TO COLUMN CONNECTION DETAIL SCALE: 3/4" = 1'-0"

10x25x3/4"

12x25x3/4"

12x25x3/4"



10x10x1"

12x12x1"

12x12x1"

TYPICAL COLUMN SPLICE PLATE CONNECTION DETAIL SCALE: 3/4" = 1'-0"

W10x49

W12x65

W12x72

PERMIT SET DOCUMENTS

W10x49

W12x65

W12x72



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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593

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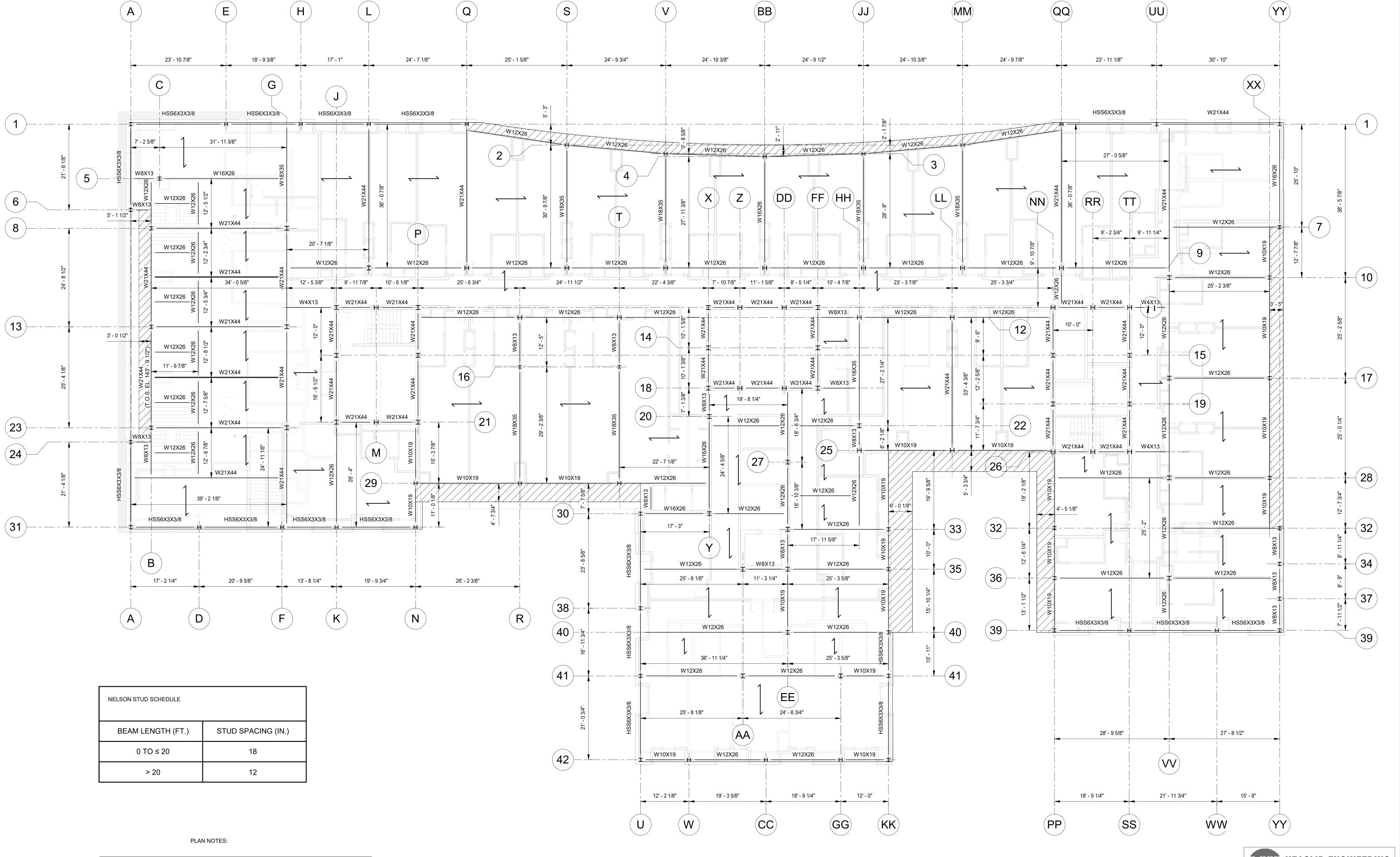
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**DETAILS** 

PROJECT #: 1601 PHASE: PERMIT DRAFTER: JRN CHECKER: JBH

SCALE: AS NOTED ISSUED: 04/20/2018

S3.14



1. SEE DRAWING S1.0 FOR GENERAL NOTES.

SYMBOL ON PLAN INDICATES 1/4" BENT PLATE.

SYMBOL ON PLAN INDICATES DECK DIRECTION.

14TH FLOOR FRAMING PLAN (T.O.S. EL. 143'-9 1/2" / T.O.S. EL. @ POOL 135'-9 1/2" / 3/32" = 1'-0"





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LICENSE NUMBER:

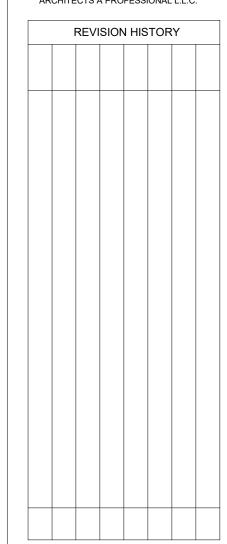
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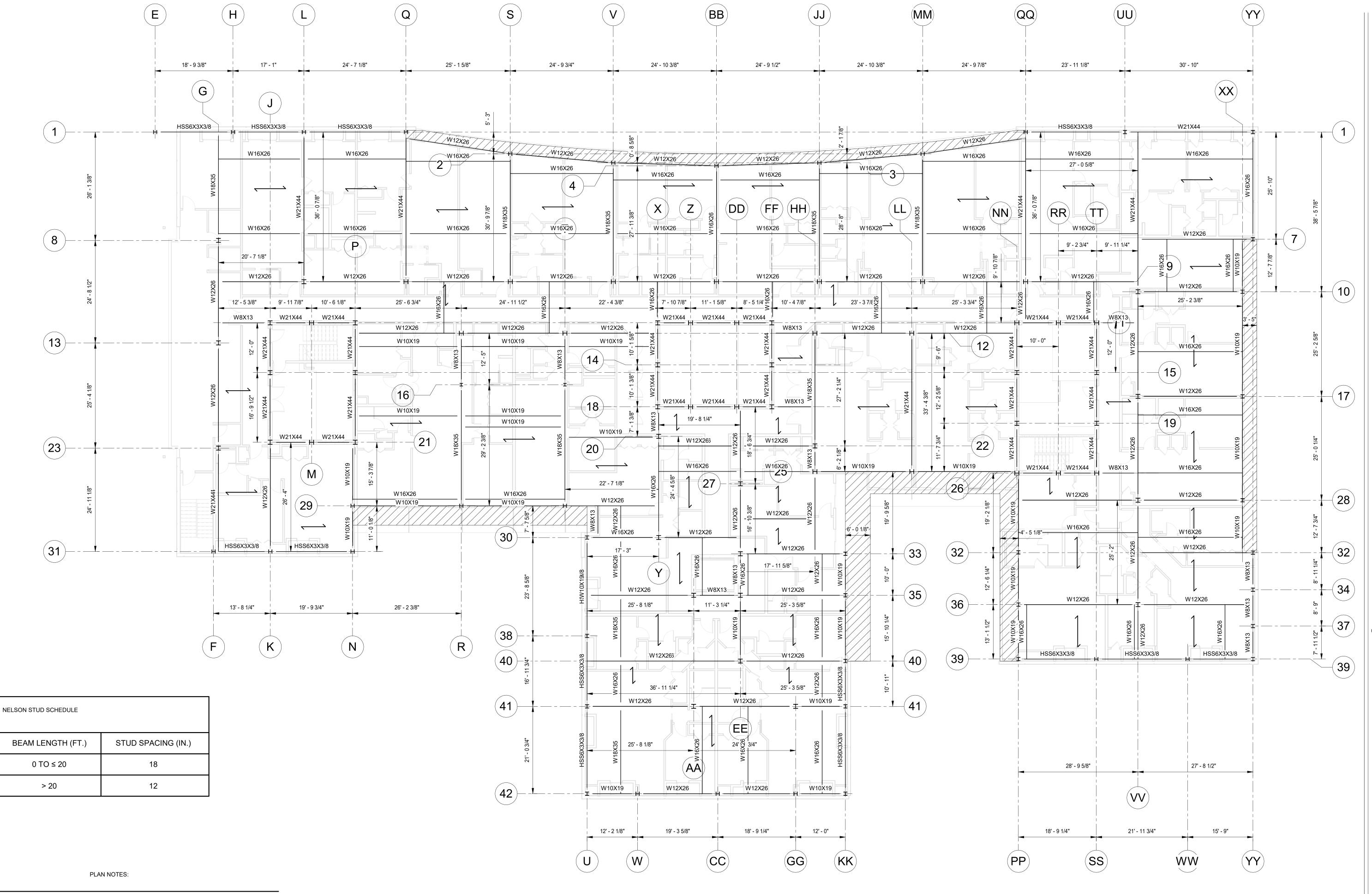


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14TH FLOOR FRAMING PLAN

PROJECT#: 1601 PHASE: PERMIT DRAFTER: BJD CHECKER: JBH SCALE: AS NOTED ISSUED: 06/08/18



1. SEE DRAWING S1.0 FOR GENERAL NOTES.

2. SYMBOL ON PLAN INDICATES 1/4" BENT PLATE.

3. - SYMBOL ON PLAN INDICATES DECK DIRECTION.

15TH FLOOR FRAMING PLAN
3/32" = 1'-0"





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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

S3.3

PLAN

PROJECT#: 1601

PHASE: PERMIT

CHECKER: JBH

SCALE: AS NOTED

ISSUED: 06/08/18

SHEET#:

DRAFTER: BJD

SMIH

ARCHITECTS II

P L C

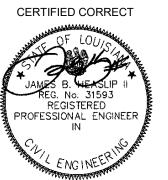
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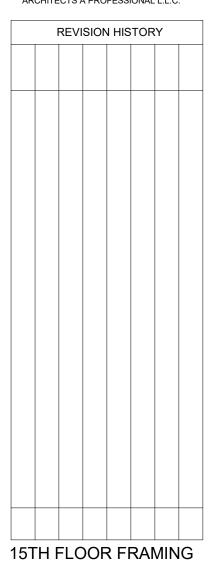
HARD ROCK 1031 CANAL STREET

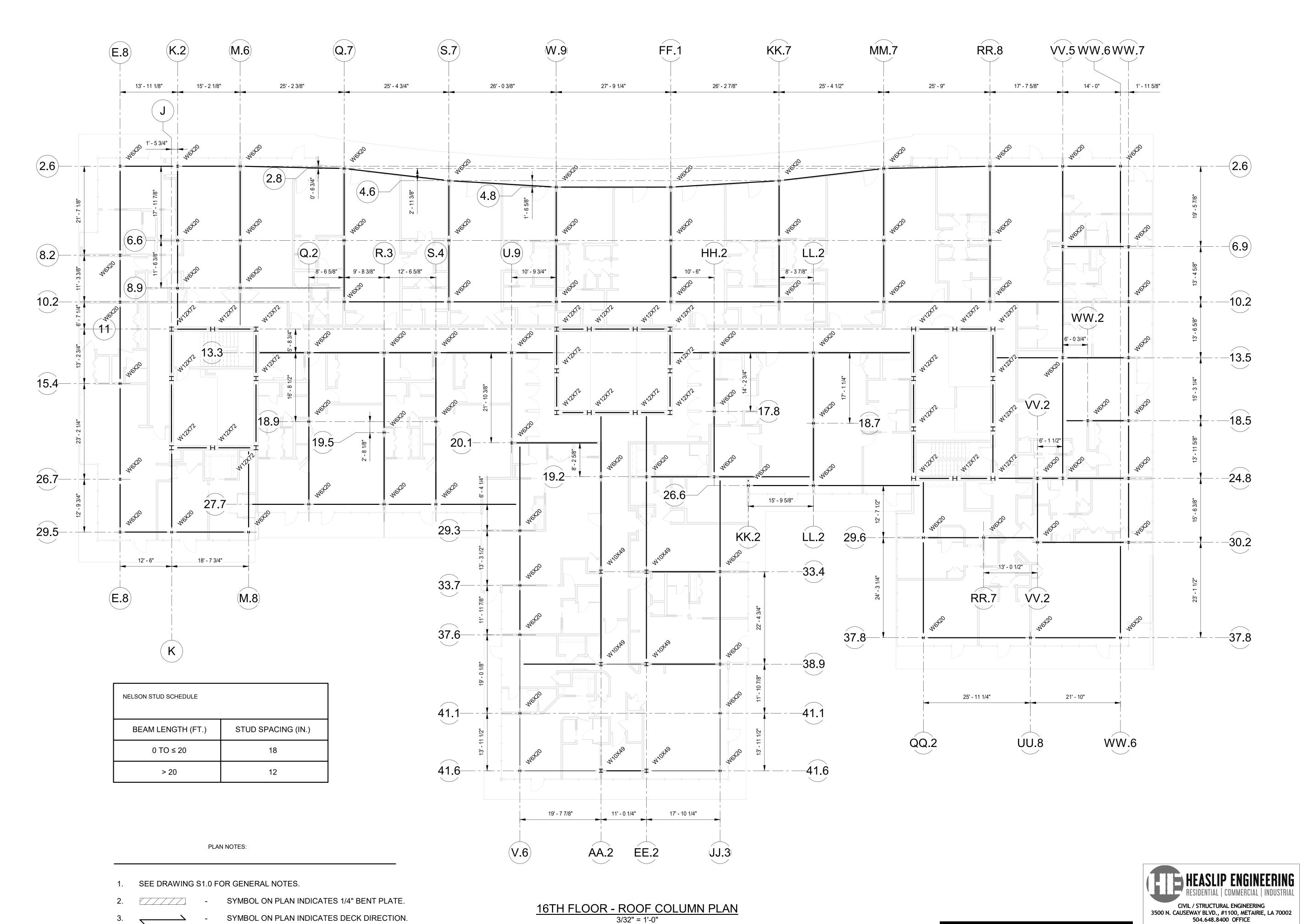
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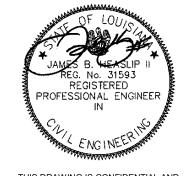




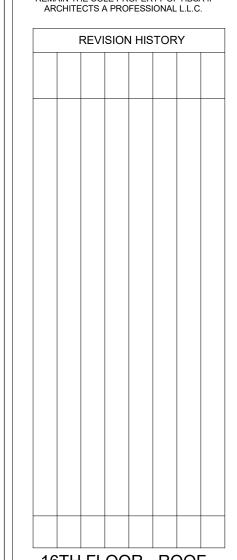
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16TH FLOOR - ROOF **COLUMN PLAN** 

PROJECT#: 1601 PHASE: PERMIT DRAFTER: BJD CHECKER: JBH SCALE: AS NOTED ISSUED: 06/08/18

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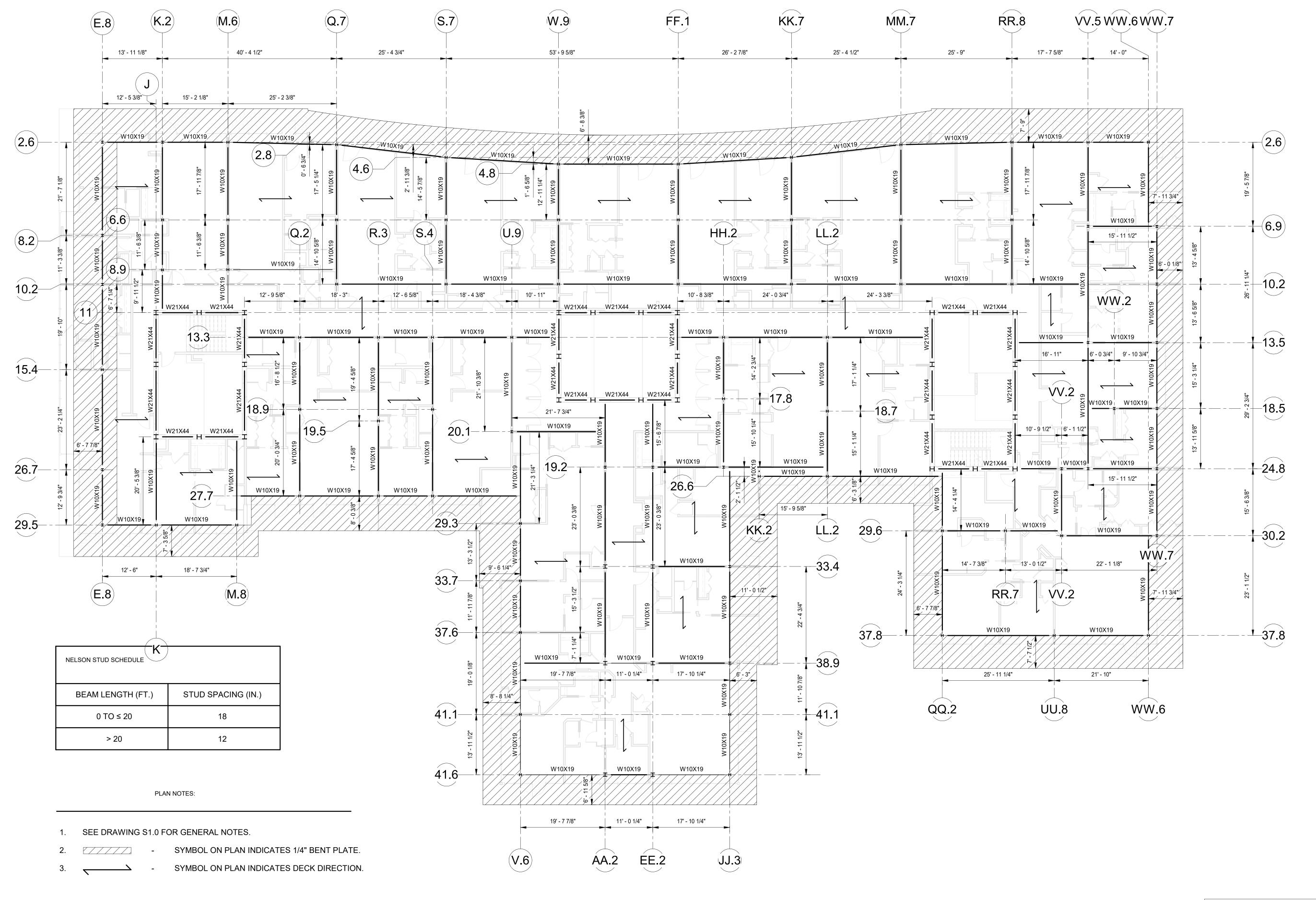
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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

I AM NOT OBSERVING THE WORK.

LICENSE NUMBER:

SHEET#:



16TH & 17TH FLOOR FRAMING PLAN 3/32" = 1'-0"





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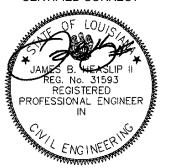
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LICENSE NUMBER:

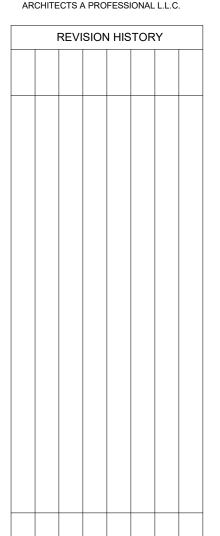
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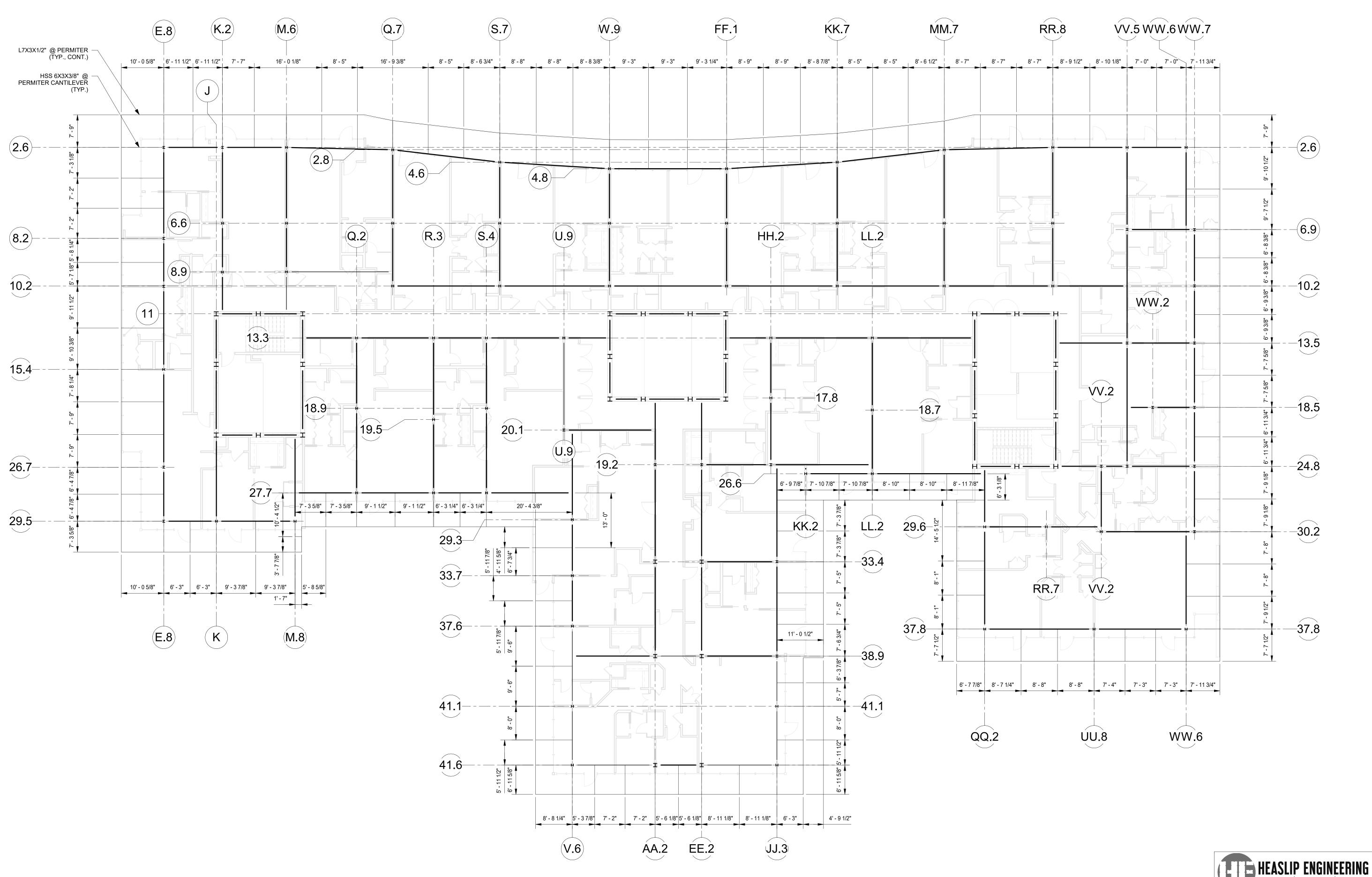


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 $^{ackslash}$ 16TH & 17TH FLOOR $^{-}$ FRAMING PLAN OPTION PROJECT#: 1601

PHASE: PERMIT DRAFTER: BJD CHECKER: JBH SCALE: AS NOTED ISSUED: 06/08/18 SHEET#:



16TH & 17TH FLOOR FRAMING PLAN OPTION B
3/32" = 1'-0"





LICENSE NUMBER:

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

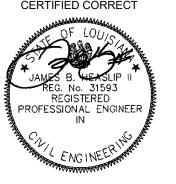
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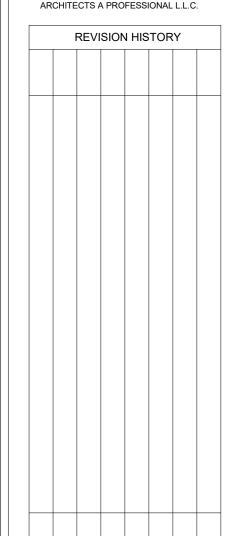
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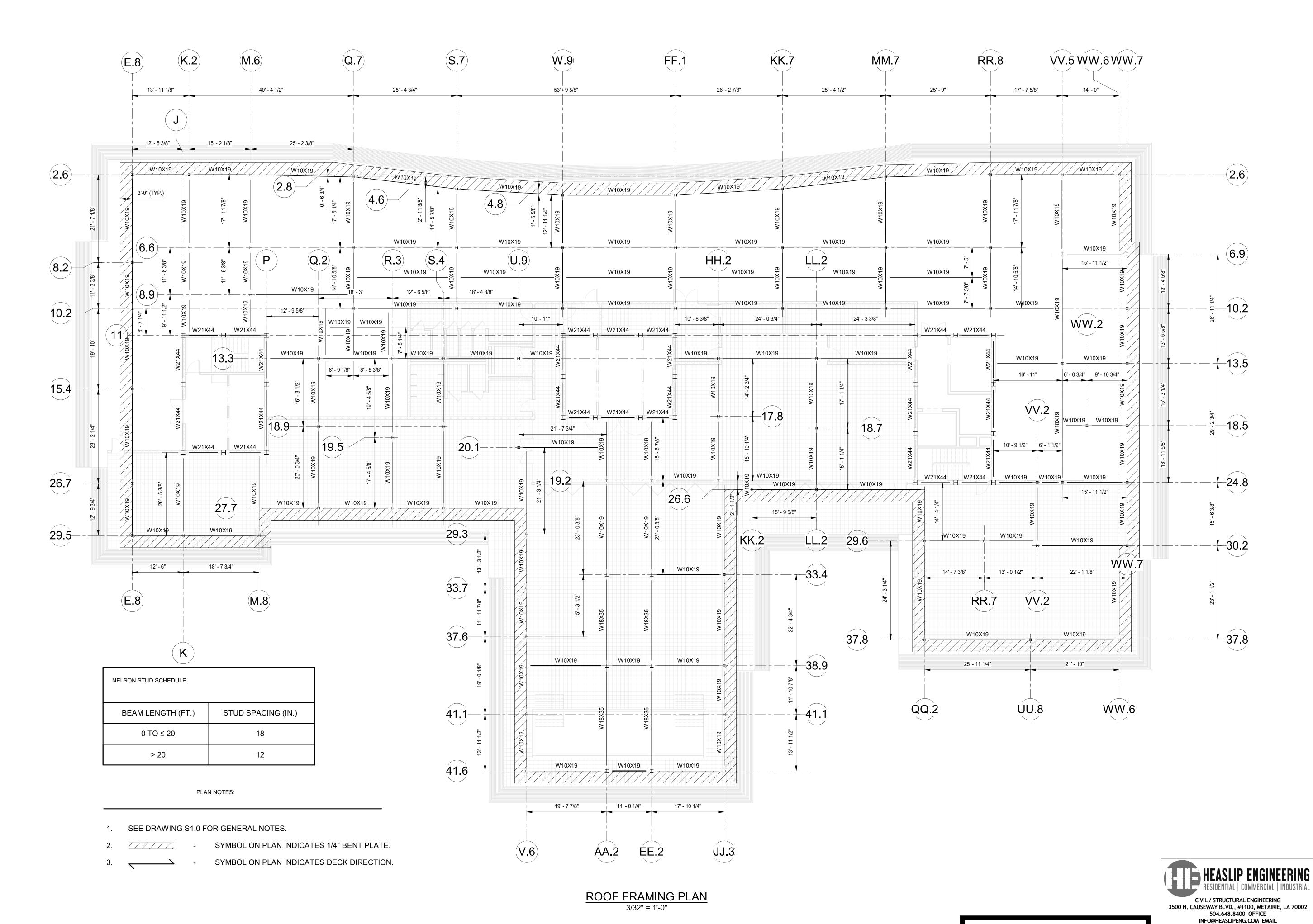
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16TH & 17TH FLOOR
FRAMING PLAN OPTION

PROJECT#: 1601
PHASE: PERMIT
DRAFTER: BJD
CHECKER: JBH
SCALE: AS NOTED
ISSUED: 06/08/18

SHEET#:



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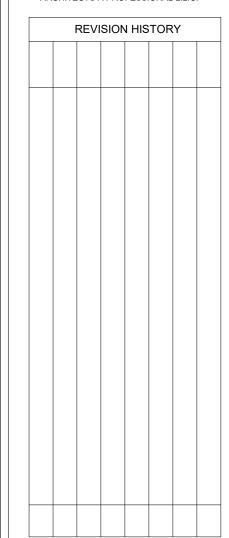
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ROOF FRAMING PLAN

PROJECT#: 1601
PHASE: PERMIT
DRAFTER: BJD
CHECKER: JBH
SCALE: AS NOTED
ISSUED: 06/08/18

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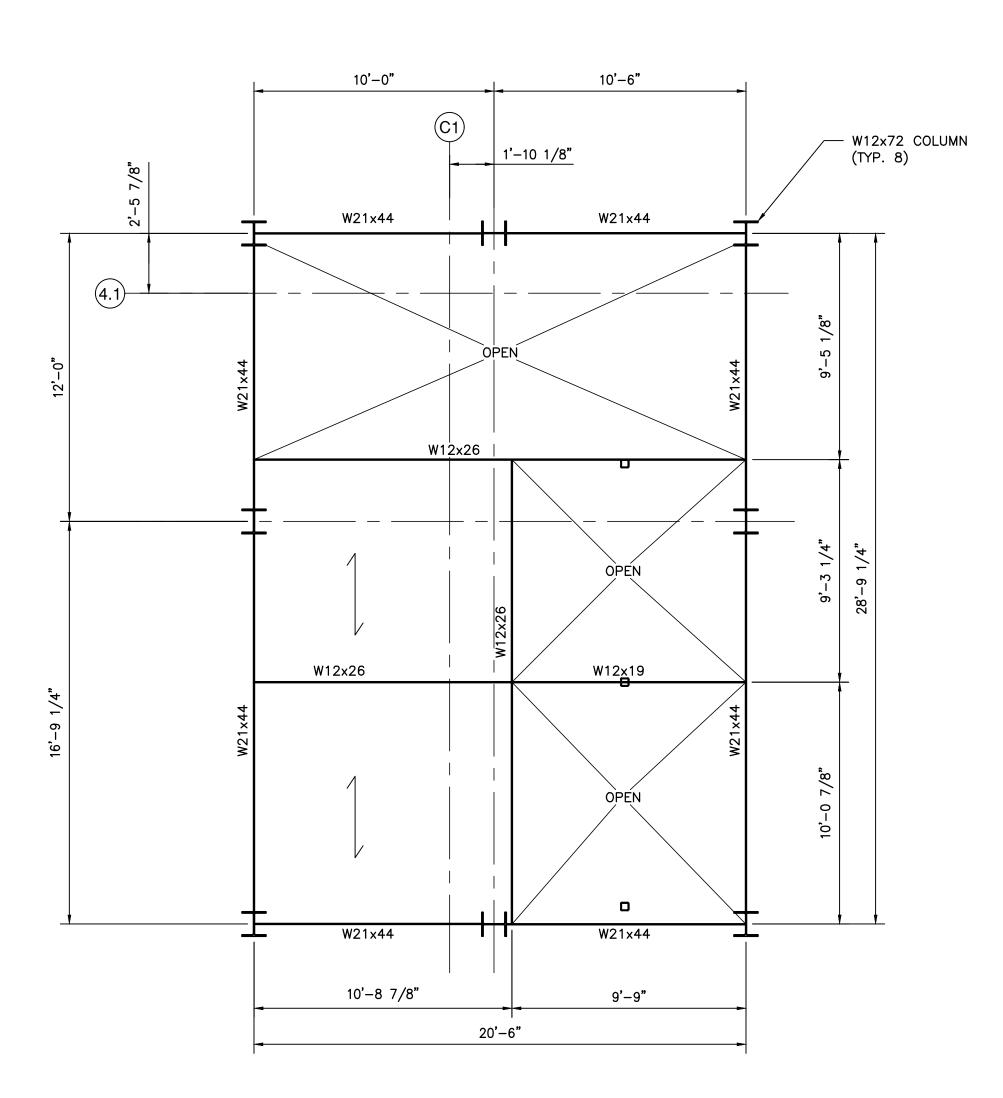
ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128

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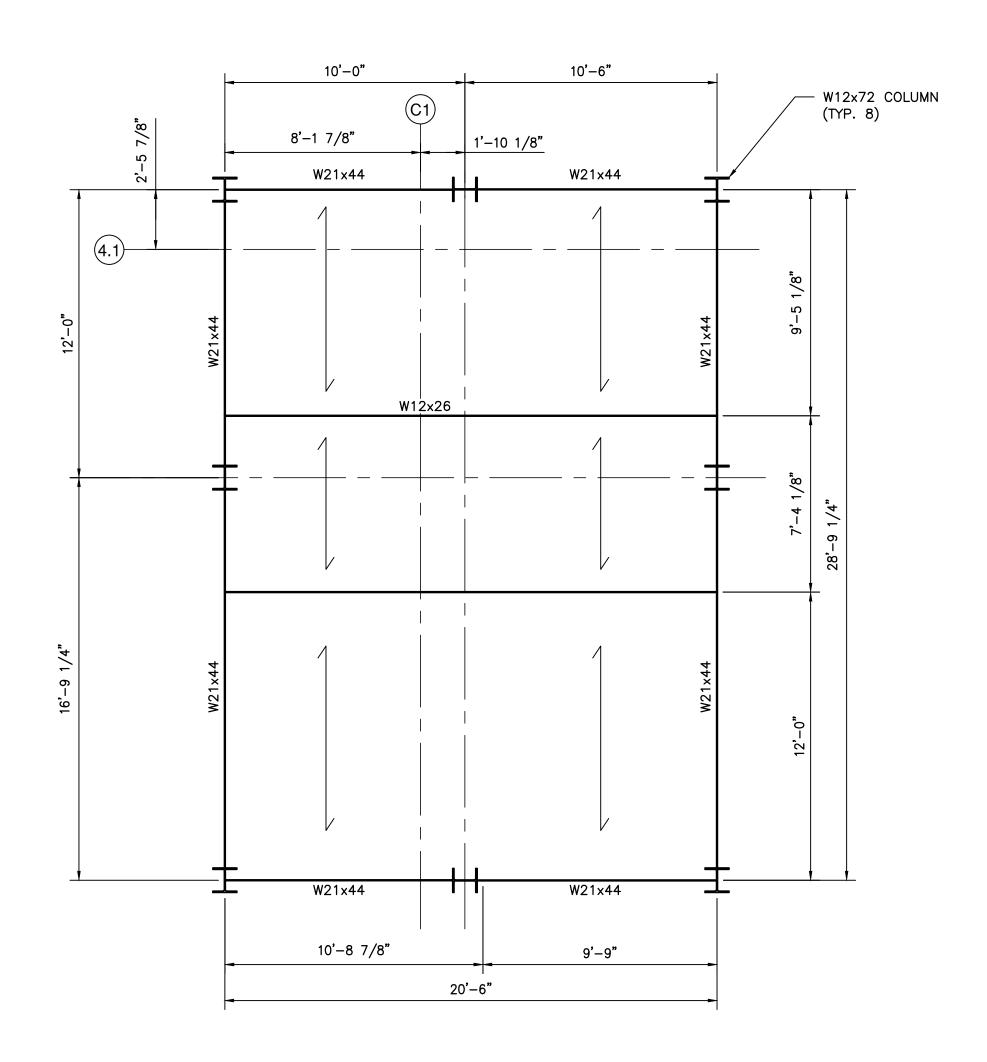
LICENSE NUMBER:

SHEET#:

PERMIT SET DOCUMENTS



9th-17th FLOOR ELEVATOR TOWER 1 PLAN DETAIL SCALE: 1/4" = 1'-0"



ROOF ELEVATOR TOWER 1 PLAN DETAIL SCALE: 1/4" = 1'-0"

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593

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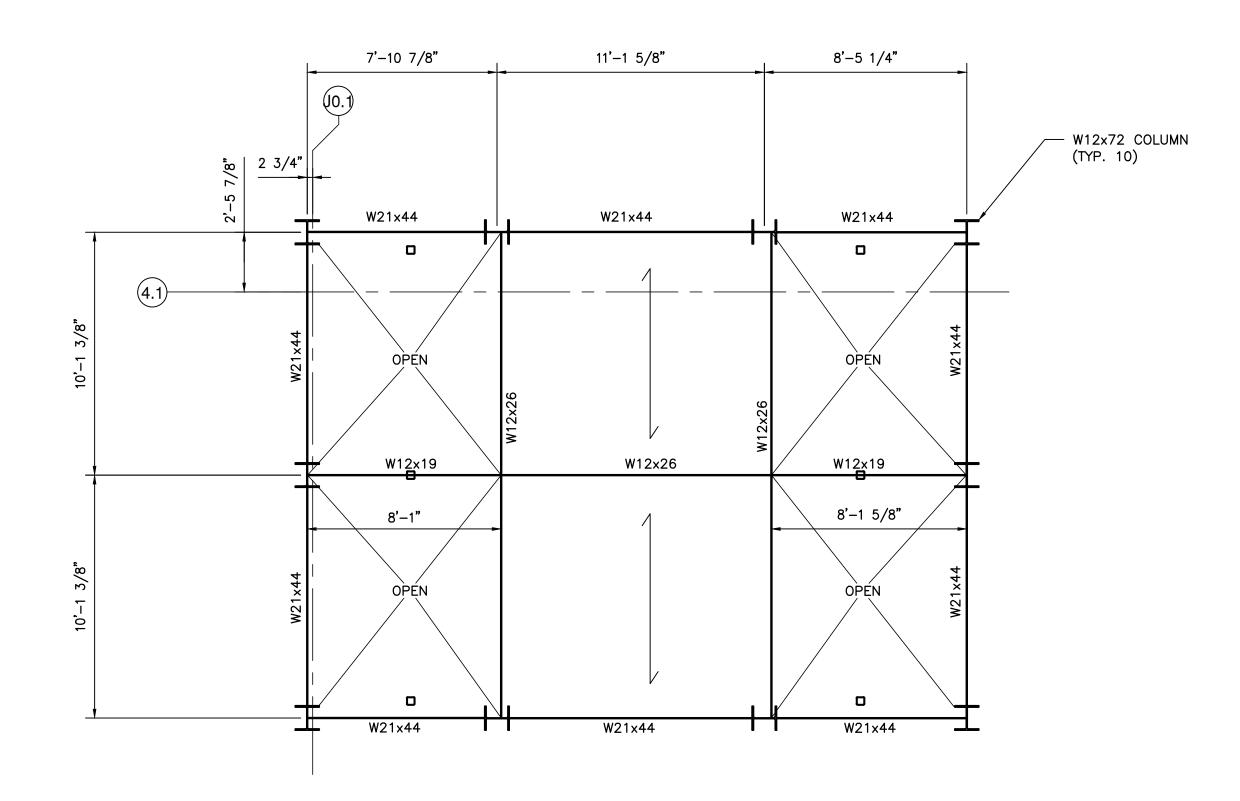
REVISION HISTORY

ENLARGED PLANS -**ELEVATOR TOWER #1** 

PROJECT #: 1601 PHASE: PERMIT DRAFTER: JRN

CHECKER: JBH SCALE: AS NOTED ISSUED: 04/20/2018

S3.7



7'-10 7/8" 11'–1 5/8" 8'-5 1/4" W12x72 COLUMN (TYP. 10) W21x44 W21x44 W21x44 4.1)— W21x44 W21x44 W21x44

9th-17th FLOOR ELEVATOR TOWER 2 PLAN DETAIL SCALE: 1/4" = 1'-0"

9th-17th FLOOR ELEVATOR TOWER 2 PLAN DETAIL SCALE: 1/4" = 1'-0"

PERMIT SET DOCUMENTS



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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593

DRAFTER: JRN CHECKER: JBH SCALE: AS NOTED ISSUED: 04/20/2018

S3.8

ENLARGED PLANS -**ELEVATOR TOWER #2** 

PROJECT #: 1601

PHASE: PERMIT

PLLC

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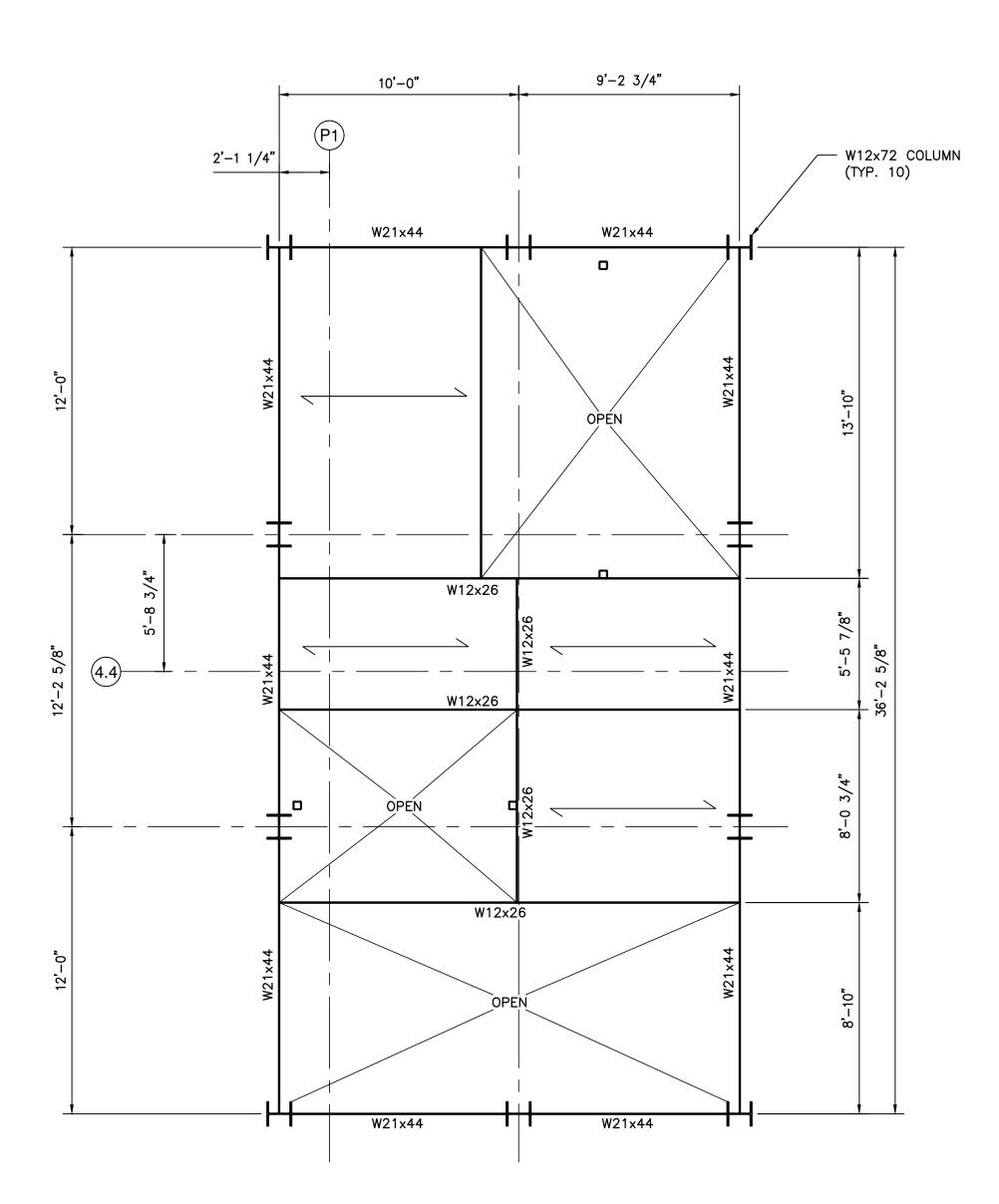
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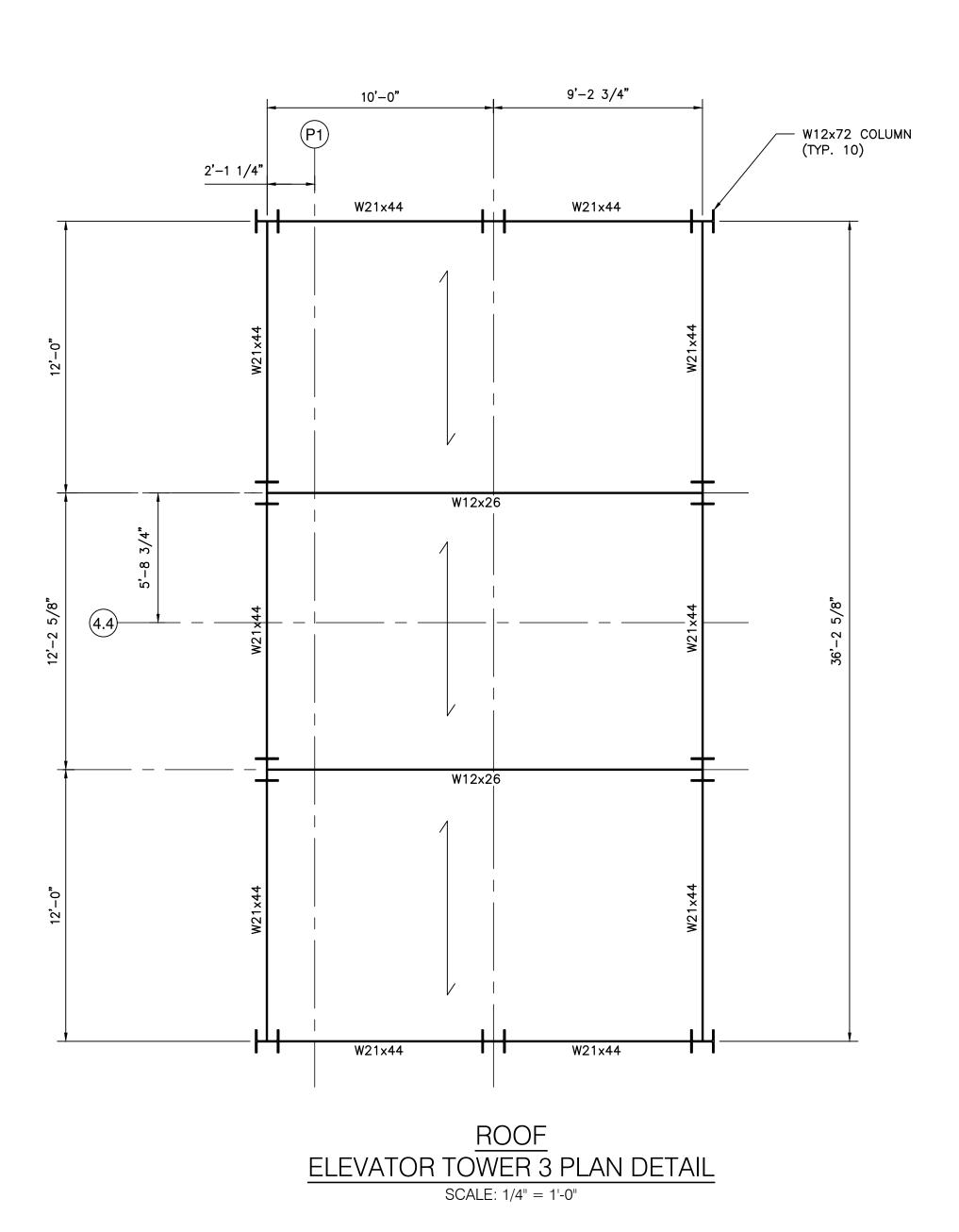
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REVISION HISTORY



9th-17th FLOOR ELEVATOR TOWER 3 PLAN DETAIL SCALE: 1/4" = 1'-0"



HEASLIP ENGINEERING
RESIDENTIAL | COMMERCIAL | INDUSTRIAL ELEVATOR TOWER #3 PROJECT #: 1601 CIVIL / STRUCTURAL ENGINEERING 3500 N. CAUSEWAY BLVD., #1100, METAIRIE, LA 70002 504.648.8400 OFFICE INFO@HEASLIPENG.COM EMAIL

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ENGINEER: JAMES B HEASLIP HE PROJECT #: 15128 31593 S3.9

SCALE: AS NOTED ISSUED: 04/20/2018

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